GENERAL:
UNLESS NOTED OTHERWISE, ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE LATEST BUILDING CODE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL EXISTING CONDITIONS AT THE JOB SITE, AND TO FULLY COORDINATE ALL DIMENSIONS AND CONDITIONS OF DETAILS WITH OTHER DISCIPLINES. ANY FIELD CONDITIONS REQUIRING CONSTRUCTION THAT IS DIFFERENT FROM THAT SHOWN ON THE PLANS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT. ANY CONFLICTING DETAILS SHOWN IN THE DRAWINGS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO THE CONSTRUCTION OF SAID DETAIL. DO NOT SCALE DRAWINGS. ANY QUESTIONS REGARDING THE CONSTRUCTION DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT IN THE FORM OF A WRITTEN REQUEST FOR INFORMATION (RFI).

ALL SUPPORT OF CONSTRUCTION LOADS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL SHORING AND BRACING REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING THE CONSTRUCTION PROCESS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALL WORK SHALL BE DONE IN ACCORDANCE WITH OSHA REQUIREMENTS. POTENTIAL CONFLICTS BETWEEN THESE DOCUMENTS AND OSHA REQUIREMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE WORK. ALL PROCEDURES OF SOIL EXCAVATION, BACK FILL, AND SUPPORT OF ADJACENT PROPERTY DURING EARTHWORK SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. ALL DIMENSIONS INDICATED ON PLANS SHALL BE TO FACE OF STUDS, FACE OF CONCRETE BLOCK, FACE OF ROUGH CONCRETE, CENTERLINE OF COLUMNS, BOTTOM OF METAL DECK, AND TOP OF SLAB, UNLESS NOTED OTHERWISE. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS NOT INDICATED ON STRUCTURAL DRAWINGS. THE FOLLOWING DESIGN CRITERIA SHALL BE ENFORCED.

GOVERNING BUILDING CODE: IBC 2015
RISK CATEGORY: II (IBC TABLE 1604.5)

LOADING:

ROOF LOAD
1. ROOF DEAD LOAD: 25 PSF
2. ROOF LIVE LOAD: 20 PSF (NON-CONCURRENT WITH ROOF SNOW LOAD)
3. ROOF SNOW LOAD:
   A. GROUND SNOW LOAD Pg = 21 PSF (ELEVATION= 2,750 FT)
   B. FLAT ROOF SNOW LOAD Pf = 15 PSF (SNOW DRIFT PER ASCE 7-10)
   C. SNOW EXPOSURE FACTOR Ce = 1.0
   D. SNOW LOAD IMPORTANCE FACTOR Is = 1.0
   E. THERMAL FACTOR Ct = 1.0

FLOOR LOAD
1. DEAD: 60 PSF (4" CONCRETE SLAB OVER METAL DECK)
2. LIVE: 100 PSF (RESTAURANT)
3. LIVE: 125 PSF (WALKIN COOLER/FREEZER/ KITCHEN AREA/ STORAGE)

WIND LOAD
1. BASIC WIND SPEED: 115 MPH (ULTIMATE)
2. WIND EXPOSURE TYPE: C
3. WIND IMPORTANCE FACTOR, Iw= 1.0
4. INTERNAL PRESSURE COEFFICIENT= ±0.18

SEISMIC LOAD
1. SEISMIC USE GROUP II
2. SEISMIC IMPORTANCE FACTOR Ie= 1.0
3. SITE COEFFICIENTS
   A. Ss = 0.510g, Fa=1.2, SMS=0.612g, SDS = 0.408g
   B. S1 = 0.160g, Fv=1.65, SM1=0.264g, SD1 = 0.176g
   C. Ct = 0.02
   D. SOIL SITE CLASS= C
   E. SEISMIC DESIGN CATEGORY= C

4. BASIC LFRS = LIGHT FRAMED WALL WITH SHEAR WALLS
   A. RESPONSE MODIFICATION COEFFICIENT R=6.5
   B. SYSTEM OVERSTRENGTH FACTOR g=3
   C. DEFLECTION AMPLIFICATION FACTOR Cd=4
   D. W WEIGHT OF STRUCTURE:
   E. DESIGN BASE SHEAR = 0.063W (ULTIMATE), 0.045W (SERVICE)
   F. DESIGN PROCEDURE: EQUIVALENT LATERAL FORCE

DEFLECTION LIMITS:
4. FLOOR L/360
5. ROOF L/240
6. WALL WITH EFIC OR STUCCO L/360
7. WALL WITH STONE CLADDING L/600

ALTERNATES:
ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.

DISCREPANCIES:
IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS PLAN/DETAILS OR REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.

SITE VERIFICATION:
THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.

PRE-CONSTRUCTION MEETINGS:
THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING PRE-CONSTRUCTION MEETINGS PRIOR TO COMMENCING WORK. PRE-CON MEETINGS, SCHEDULED APPROXIMATELY TWO WEEKS PRIOR TO THE START OF THE RELEVANT WORK, ARE REQUIRED FOR THE FOLLOWING PHASES OF CONSTRUCTION: POST-TENSIONED SLABS, SHOTCRETE, CONCRETE, MASONRY. ATTENDEES FOR THE PRE-CONSTRUCTION MEETING ARE TO INCLUDE CONTRACTOR, RELEVANT SUBCONTRACTORS, FABRICATORS, INSPECTORS, ARCHITECT/ENGINEER, AND REPRESENTATIVE OF THE AUTHORITY HAVING JURISDICTION WHERE REQUIRED. MEETING AGENDAS ARE TO INCLUDE REVIEW OF THE WORK SCOPE, PROJECT SCHEDULE RELEVANT TO THE WORK, CONTACT INFORMATION OF RESPONSIBLE PARTIES, INSPECTION POINTS, REVIEW OF MATERIALS AND ANY SPECIAL CASES OR ISSUES, PROCEDURES FOR CLARIFICATIONS IF REQUIRED, TESTING AND ACCEPTANCE, ETC.

MEANS, METHODS AND SAFETY REQUIREMENTS:
THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH). CONTRACTOR IS RESPONSIBLE TO ADHERE TO OSHA REGULATIONS REGARDING STEEL ERECTION ITEMS SPECIFICALLY ADDRESSED ON THE LATEST OSHA REGULATIONS. BOLTING AND FIELD WELDING AT ALL MEMBER CONNECTIONS IS TO BE COMPLETED PRIOR TO THE RELEASE OF THE MEMBER FROM THE HOISTING MECHANISM UNLESS REVIEWED AND APPROVED BY THE GENERAL CONTRACTOR'S TEMPORARY BRACING AND SHORING DESIGN ENGINEER.

BRACING/SHORING DESIGN ENGINEER:
THE CONTRACTOR SHALL AT HIS DISCRETION EMPLOY AN SSE, A REGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING.

TEMPORARY SHORING, BRACING:
THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

CONSTRUCTION LOADS:
LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA & LOADS BELOW OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

CHANGES IN LOADING:
THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE SER OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL / STRUCTURAL / MECHANICAL / ELECTRICAL OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF LOCATION, LOAD, SIZE AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 POUNDS. PROVIDE MARKED-UP STRUCTURAL PLAN INDICATING LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

NOTE PRIORITIES:
PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON THE INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THE STRUCTURAL GENERAL NOTES.

PLAN INFORMATION:
DIMENSIONS ARE FOR REFERENCE, CONTRACTOR TO VERIFY ALL DIMENSIONS. DIMENSIONS ARE PROVIDED BY THE ARCHITECT'S ELECTRONIC FILE. ALWAYS VERIFY THESE PLANS AND DIMENSIONS WITH THE ARCHITECT PLANS UNDER NO CIRCUMSTANCES WILL McNEIL ENGINEERING, ITS EMPLOYEES OR AGENTS BE LIABLE FOR ANY DIRECT, INDIRECT PUNITIVE OR CONSEQUENTIAL DAMAGES THAT MAY RESULT IN ANY WAY FROM YOUR USE, MISUSE, REFERENCE TO OR RELIANCE ON ANY OF THE INFORMATION PROVIDED OR THAT RESULT FROM MISTAKES, ERRORS, OMISSIONS, INTERPRETATIONS OR DEFECTS. MCNEIL ENGINEERING EXPRESSLY DISCLAIMS ALL WARRANTIES, INCLUDING ANY EXPRESS OR

IMPLIED WARRANTIES OF MERCHANTABILITY OR FITNESS FOR A PARTICULAR PURPOSE WITH RESPECT TO THE INFORMATION HEREIN.

WHILE MCNEIL ENGINEERING MAKES EVERY EFFORT TO PRESENT ACCURATE AND RELIABLE INFORMATION, MCNEIL ENGINEERING DOES NOT ENDORSE, APPROVE OR CERTIFY THE INFORMATION PROVIDED BY OTHERS, NOR DOES MCNEIL ENGINEERING GUARANTEE ITS ACCURACY, COMPLETENESS OR TIMELINESS. USE OF THIS INFORMATION IS VOLUNTARY AND RELIANCE ON IT SHOULD ONLY BE UNDERTAKEN AFTER YOU HAVE INDEPENDENTLY VERIFIED ITS ACCURACY, COMPLETENESS AND TIMELINESS.

CONTRACTOR SHALL BE RESPONSIBLE TO FIELD VERIFY DIMENSIONS AGAINST THE CORRESPONDING OFFICIAL CONSTRUCTION DRAWINGS. DIMENSIONS SHOWN ON THE CONSTRUCTION DOCUMENTS MUST BE VERIFIED WITH ARCHITECTURAL PLANS. IF ANY DISCREPANCIES ARE FOUND THE CONTRACTOR SHALL NOTIFY THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. MCNEIL ENGINEERING DOES NOT GUARANTEE THAT THIS ELECTRONIC MEDIA HAS NOT BEEN DAMAGED, ALTERED OR MODIFIED DURING TRANSMISSION AND/OR STORAGE. MCNEIL ENGINEERING DOES NOT GUARANTEE CHANGES ON THE ARCHITECTURAL PLANS HAVE BEEN FULLY CONVEYED AND THE THE CONSTRUCTION DOCUMENT UPDATED. ANYONE RECEIVING ELECTRONIC MEDIA MUST VERIFY ALL INFORMATION WITH THE CORRESPONDING OFFICIAL CONSTRUCTION DRAWINGS. ANY USE OR REUSE OF THIS INFORMATION SHALL BE THE FULL RESPONSIBILITY OF THE USER.

FOUNDATIONS:
SOILS REPORT: AGEC APPLIED GEOTECH DATED JUNE 6, 2018 PROJECT NO. 2180293 AND UPDATE LETTER DATED SEPTEMBER 18, 2018 AND UPDATE LETTER DATED SEPTEMBER 18, 2018.

SOIL TO BE OBSERVED PRIOR TO PLACEMENT OF FOOTINGS. ALL FOOTING DEPTHS INDICATED ON PLANS ARE MINIMUM DEPTHS. FOOTINGS MAY BE PLACED IN NEAT EXCAVATED TRENCHES. TRENCH SHALL BE APPROVED BY INSPECTOR PRIOR TO PLACEMENT OF CONCRETE. AT LOCATIONS WHERE STRUCTURAL FILL IS REQUIRED, FILL SHALL BE PLACED IN 6" LIFTS & COMPACTED AT OPTIMUM MOISTURE CONTENT. REFER TO SOILS INVESTIGATION FOR DEPTH AND EXTENT OF STRUCTURAL FILL.

1. MAXIMUM NET BEARING PRESSURE = 2,000 PSF TO 2,500 PSF (NET ALLOWABLE) DEPENDING ON STRUCTURAL FILL THICKNESS AND SIZE OF FOOTING SEE SOILS REPORT AND UPDATE LETTER
2. MIN DEPTH= 12"
3. INTERIOR FOOTINGS = 12" (BOTTOM OF FOOTING FROM TOP OF GRADE)
4. LATERAL SOIL PRESSURE : ACTIVE = 45 PCF; AT REST = 65 PCF; PASSIVE = 220 PCF
5. SEISMIC LATERAL EARTH PRESSURE: ACTIVE = +9 PCF, AT REST = 0 PCF, PASSIVE=-14 PCF
6. FRICTION COEFFICIENT : 0.45
7. CONVENTIONAL SPREAD AND SPOT FOOTINGS WITH SLAB ON GRADE FLOORS TO BEAR DIRECTLY ON THE UDERLYING UNDISTURBED BEDROCK OR ON A ZONE OF PROPERLY COMPACTED, IMPORTED STRUCTURAL FILL PER SOILS REPORT.
8. LIQUEFACTION POTENTIAL= "LOW"

CONCRETE:
ALL CONCRETE MATERIALS SHALL COMPLY WITH THE STANDARDS SPECIFIED IN THE LATEST EDITION OF THE ACI 318 BUILDING CODE. EACH MIX DESIGN SHALL BE REVIEWED BY AN APPROVED INDEPENDENT LABORATORY, AND SHALL BE SUBMITTED TO THE ENGINEER AT LEAST 2 WEEKS PRIOR TO THE PLACEMENT OF CONCRETE. CONTRACTOR SHALL INFORM THE ENGINEER AT LEAST 2 DAYS PRIOR TO PLACING ANY CONCRETE SO THAT THE ENGINEER MAY HAVE THE OPPORTUNITY TO REVIEW THE WORK. CONCRETE TESTING SHALL BE PERFORMED BY AN APPROVED INDEPENDENT TESTING LABORATORY. THE TESTING AGENCY SHALL TEST (4) CYLINDERS FROM EACH CLASS OF CONCRETE USED EACH DAY. A MINIMUM OF (1) SAMPLE MUST BE TAKEN FROM EACH 150 CUBIC YARDS OF CONCRETE.

LOCATION	SPECIAL INSPECTION	EXPOSURE CLASS	SLUMP (MAX)	AGGREGATE (MAX SIZE)	AIR CONTENT	COMPRESSIVE STRENGTH (PSI)
FOOTINGS (INTERIOR)	NO	F0, S2, P0, C0	5"	1" DIA.	1.5%	4,500 PSI
FOOTINGS (EXTERIOR)	NO	F1, S2, P0, C0	5"	1" DIA.	1.5%	4,500 PSI
CONCRETE WALLS	YES	F1, S2, P0, C1	4"	3/4" DIA.	5%	4,500 PSI
CONCRETE COLUMNS	YES	F1, S2, P0, C1	4"	3/4" DIA.	5%	4,500 PSI
INTERIOR SLAB ON GRADE	NO	F1, S2, P0, C0	5"	3/4" DIA.	1.5%	4,500 PSI
SLAB ON DECK	NO	F0, S0, P0, C0	4"	3/4" DIA.	6%	4,000 PSI

CONCRETE TYPE: TYPE V
MAX WATER TO CEMENT RATIO 0.45
MAX FLYASH: 25%

AIR CONTENT +- 1.5% MEASURED AT POINT OF FINAL PLACEMENT. AIR-ENTRAINING ADMIXTURES SHALL COMPLY WITH ASTM C260 (WHEN USED). CALCIUM CHLORIDE SHALL NOT BE ADDED TO THE CONCRETE MIX. UNREINFORCED CONCRETE SLABS ON GRADE MAY HAVE CALCIUM CHLORIDE NOT EXCEEDING ONE PERCENT. AIR ENTRAINMENT SHALL BE ADJUSTED FOR THE USE OF ADMIXTURES AND FLY ASH.

ANY CONCRETE THAT FAILS TO MEET SPECIFICATIONS SHALL BE REMOVED AND REPLACED AT THE EXPENSE OF THE CONTRACTOR.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CONSTRUCTION, DESIGN, PLACEMENT AND REMOVAL OF ALL FORMWORK. ALL SHORING DURING PLACEMENT OF CONCRETE IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

SEE CIVIL DRAWINGS FOR SITE CONCRETE REQUIREMENTS. IN ABSENCE OF INFORMATION, USE VALUES LISTED.

CONCRETE SHALL BE SPECIALLY INSPECTED PER, TABLE 1704.4

CAST-IN-PLACE CONCRETE COVER:
CONFORM TO THE FOLLOWING COVER AND CORROSION PROTECTION REQUIREMENTS UNLESS NOTED OTHERWISE IN THE DRAWINGS:

REINFORCEMENT LOCATION	MIN. COVER
FOOTING BOTTOM REINFORCEMENT	3"
FOOTING TOP REINFORCEMENT	2"
SLAB ON GRADE REINFORCEMENT	2" FROM TOP
WALLS IN CONTACT WITH EARTH	2"
WALLS NOT IN CONTACT WITH EARTH	3/4"

CONCRETE REINFORCING:
ALL REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60, Fy=60,000 PSI MIN., UNLESS NOTED OTHERWISE. BARS SHALL BE TIED SECURE PRIOR TO PLACEMENT OF CONCRETE TO MAINTAIN PROPER PLACEMENT AFTER CONCRETE IS IN PLACE. LAP ALL BARS 40 DIAMETERS UNLESS NOTED OTHERWISE. SPLICE BARS ONLY WHERE SHOWN ON PLANS.

SHOP DRAWINGS OF ALL BARS AND LOCATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. NORMAL WEIGHT CONCRETE SHALL HAVE A UNIT WEIGHT OF POUNDS PER CUBIC FOOT. USE OF CALCIUM CHLORIDE IS NOT PERMITTED IN ANY CONCRETE MIXES. ALL OTHER ADDITIVES AND ADMIXTURES MUST HAVE THE WRITTEN APPROVAL OF THE ENGINEER. THE ENGINEER SHALL HAVE 10 BUSINESS DAYS TO REVIEW SHOP DRAWINGS.

STRUCTURAL STEEL:
ALL STRUCTURAL STEEL COMPONENTS SHALL BE FABRICATED AND ERECTED ACCORDING TO THE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "SPECIFICATIONS FOR DESIGN FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS", WITH " COMMENTARY", AND THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AS WELL AS THE FOLLOWING STANDARDS:

AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS" APPROVED BY THE RESEARCH COUNCIL ON RIVETED AND BOLTED STRUCTURAL JOINTS OF THE ENGINEERING FOUNDATION.
AWS D1.1 "STRUCTURAL WELDING CODE".
ASTM A-6 "GENERAL REQUIREMENTS FOR DELIVERY OF ROLLED STEEL PLATES, SHAPES, SHEET PILING AND BARS FOR STRUCTURAL USE".

ALL STEEL SECTIONS SHALL CONFORM TO THE FOLLOWING:
WIDE FLANGE SHAPES: ASTM A572 GRADE 50 OR ASTM A992 GRADE 50.
HOLLOW STRUCTURAL SECTIONS: ASTM A500 GRADE B
FY MIN. = 46 KSI
ANGLES, CHANNELS, PLATES & BARS: ASTM A36.

SHOP PAINTING:

PAINTING REQUIREMENTS: NO SHOP PAINTING WILL BE REQUIRED EXCEPT WHERE PLANS INDICATE THAT STRUCTURAL STEEL WILL BE EXPOSED TO WEATHER. (I.E. COOLING TOWER FRAMING, DOOR LINTELS, SCREEN WALLS, ETC.)

SURFACE PREPARATION: REMOVE LOOSE RUST, LOOSE MILL SCALE, AND SPATTER, SLAG OR FLUX DEPOSITS. CLEAN STEEL IN ACCORDANCE WITH STEEL STRUCTURES PAINTING COUNCIL (SSPC) SP-3 "POWER TOOL CLEANING".

PAINTING: ONE COAT OF APPROVED PRIMER APPLIED BY BRUSH, SPRAY, ROLLER OR DIP TO PRODUCE A DRY FILM THICKNESS OF NOT LESS THAN 2 MILS.

NOTE: DO NOT SHOP PAINT WHEN TEMPERATURE IS BELOW 45 DEGREES F., OR WHEN STEEL TEMPERATURE IS BELOW DEW POINT OF ATMOSPHERE OR ON WET SURFACES.

ALL WELDING SHALL BE DONE BY CERTIFIED AWS WELDERS, AND COMPLY WITH THE LATEST EDITION OF THE AWS D1.1 CODE FOR WELDING IN BUILDING CONSTRUCTION. ALL FILLET WELDS TO BE 3/16" UNLESS NOTED OTHERWISE. ALL STEEL TO STEEL BOLTED CONNECTIONS SHALL BE WITH ASTM A307, UNLESS NOTED OTHERWISE. PROVIDE EDGE DISTANCE IN ACCORDANCE TO AISC TABLE J3.7 UNLESS NOTED OTHERWISE. ALL FIELD WELDS SHALL BE SPECIALLY INSPECTED PER IBC TABLE. ALL TESTING AND INSPECTION REPORTS SHALL BE SENT TO THE ENGINEER FOR REVIEW.

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. THE ENGINEER SHALL HAVE 10 BUSINESS DAYS TO REVIEW SHOP DRAWINGS.

STRUCTURAL STEEL SHALL BE SPECIALLY INSPECTED AS PER AISC 360.

STEEL DECK:
ALL STEEL DECKS SHALL COMPLY WITH THE LATEST REQUIREMENTS OF THE STEEL DECK INSTITUTE. CONTRACTOR SHALL SUBMIT CURRENT ICBO REPORTS FOR EACH DECK USED TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO ATTACHMENT OF STEEL DECK. DECKING LAYOUT SHALL BE SUCH THAT DECKING SPANS 3 OR MORE SPANS MINIMUM. WHERE SUCH LAYOUT IS IMPOSSIBLE, DECKING MUST MEET DESIGN CRITERIA FOR SIMPLE SPAN CONDITIONS. PROVIDE 2" MINIMUM BEARING. THE ENGINEER SHALL HAVE 10 BUSINESS DAYS TO REVIEW SHOP DRAWINGS.

OPEN WEB STEEL JOISTS AND TRUSSES:
ALL STEEL JOIST FABRICATION AND DESIGN SHALL COMPLY WITH THE LATEST EDITION OF THE STEEL JOIST INSTITUTE "STANDARD SPECIFICATIONS AND CODE OF PRACTICE". ALL ROOF JOISTS SHALL BE CAMBERED AS SPECIFIED IN THE STEEL JOIST INSTITUTE SPECIFICATIONS, UNLESS NOTED OTHERWISE. NO JOIST OR GIRDER MAY BE ALTERED OR MODIFIED IN THE FIELD WITHOUT THE WRITTEN APPROVAL FROM THE ENGINEER. A COMPLETE SET OF SHOP DRAWINGS, ICC REPORTS AND CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO THE FABRICATION OF ANY ANY MEMBERS. SUCH CALCULATIONS SHALL BE PREPARED UNDER THE DIRECTION OF A LICENSED ENGINEER IN THE STATE IN WHICH THE BUILDING IS TO BE BUILT.

LAMINATED VENEER LUMBER:
ALL LAMINATED VENEER LUMBER SHALL CONFORM TO THE SPECIFICATIONS OF WEYERHAEUSER CORPORATION FOR VENEER LUMBER, OR ENGINEER APPROVED EQUIVALENT. DESIGN VALUES SHALL MEET OR EXCEED THOSE PUBLISHED VALUES IN THE WEYERHAEUSER PRODUCT GUIDE, LATEST EDITION. A COMPLETE SET OF STRUCTURAL SHOP DRAWINGS, INDICATING MEMBERS AND PLACEMENT SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO THE FABRICATION OF THE MEMBERS. THE ENGINEER SHALL HAVE 10 BUSINESS DAYS TO REVIEW SHOP DRAWINGS.

SHEATHING:
SHEATHING SHALL BE A.P.A. RATED, SEE PLAN FOR SPAN RATING AND THICKNESS.
SHEATHING INSTALLATION:

ROOF AND FLOOR SHEATHING SHALL BE LAID WITH THE FACE GRAIN PERPENDICULAR TO THE FRAMING MEMBERS U.N.O. AND END JOINTS SHALL BE STAGGERED. WALL SHEATHING MAY BE APPLIED HORIZONTALLY OR VERTICALLY.

ALL NAILS SHALL BE COMMON WIRE NAILS U.N.O. EQUIVALENT PNEUMATIC DRIVEN NAILS MAY BE USED IF FASTENER MANUFACTURER HAS CURRENT I.C.C. APPROVAL. FASTENERS TO BE USED SHALL BE EQUIVALENT IN LATERAL AND WITHDRAWAL STRENGTH TO THE SIZE COMMON NAIL SPECIFIED.

USE EXTERIOR GRADE SHEATHING AT DECKS AND CORRIDORS.

ROOF SHEATHING:
EDGE BLOCKING OF UNSUPPORTED EDGES OF SHEATHING AS NOTED ON PLANS. PLY CLIPS OR APPROVED EQUAL CONNECTOR SHALL BE INSTALLED AT MID SPAN BETWEEN EACH SUPPORT WHEN RAFTER SPACING EXCEEDS 16" AND EDGE BLOCKING IS NOT SPECIFIED.

TYPICAL NAILING SHALL BE 8d @ 6" O.C. AT SUPPORTED EDGES AND OVER SHEAR WALLS AND 8d AT 12" O.C. AT INTERMEDIATE SUPPORTS, U.N.O.

FLOOR SHEATHING:
EDGE BLOCKING OF UNSUPPORTED EDGES OF SHEATHING AS NOTED ON PLANS.

TYPICAL NAILING SHALL BE 10d @ 6" O.C. ALL SUPPORTED EDGES AND OVER SHEAR WALLS, AND 10d @ 12" O.C. ALL INTERMEDIATE SUPPORTS U.N.O. USE RING SHANK NAILS.

ALL FLOOR SHEATHING SHALL BE GLUED TO JOISTS. THE FIELD-GLUED FLOOR SYSTEM SHALL BE INSTALLED ACCORDING TO THE RECOMMENDATION OF THE AMERICAN PLYWOOD ASSOCIATION. GLUE SHALL BE APPLIED TO THE JOISTS AND TO THE GROOVE IN THE EDGE OF THE T & G PANELS. GLUE SHALL MEET THE REQUIREMENTS OF THE AMERICAN PLYWOOD ASSOCIATION ADHESIVE SPEC. AFG-D1 AND SHALL BE APPLIED AS DIRECTED BY THE GLUE MANUFACTURER. GLUE MAY BE APPLIED MANUALLY OR WITH PNEUMATIC OF ELECTRIC EQUIPMENT.

ROUGH CARPENTRY:
FRAMING LUMBER SHALL BE KILN DRIED AND SHALL MEET THE FOLLOWING MINIMUM STANDARD U.N.O.

USE:	SPECIES	GRADE
SILL PLATES 2 x 4	D.F.	STANDARD OR BETTER.
2 x 6, 2 x 8	D.F	NO. 2 OR BETTER.

ALL SILL PLATES IN CONTACT WITH CONCRETE OR MASONRY, SHALL BE PRESSURE TREATED OR CALIFORNIA REDWOOD.

HORIZONTAL FRAMING LUMBER: (UNO)		
4x4 AND SMALLER	D.F.	NO. 2
2x ROOF JOISTS & RAFTERS	D.F.	NO. 2
2x FLOOR JOISTS	D.F.	NO. 2
3x LEDGERS	D.F.	NO. 1
4x HEADERS & BEAMS	D.F.	NO. 1
6x6 & LARGER BEAMS	D.F.	NO. 1

VERTICAL FRAMING LUMBER: (U.N.O.)		
ALL STUDS	D.F.	STUD GRADE OR #2 (SEE PLAN)
ALL POSTS	D.F.	NO. 1
ALL OTHER LUMBER U.N.O	D.F.	STANDARD OR BETTER.

FINGER-JOINTED LUMBER MAY BE USED EXCEPT AT SHEARWALL HOLDOWNS LOCATIONS.

AT EXTERIOR LOCATIONS, DECKS EXPOSED CORRIDORS, USE APA RATED SHEATHING EXTERIOR. WHERE CONSTRUCTION DELAYS ARE EXPECTED PRIOR TO PROVIDING PROTECTION USE APA RATED SHEATHING EXPOSURE 1 COMMONLY KNOWN AS "CDX".

PROVIDE A MINIMUM OF (2) STUDS UNDER ALL BEAM BEARING LOCATIONS UNO. PROVIDE A MINIMUM OF (3) STUDS UNDER ALL GIRDER TRUSS BEARING LOCATIONS UNO. WHERE POSTS OR MULTIPLE STUDS UNDER BEAMS OR HEADERS ARE IDENTIFIED ON DRAWINGS, THOSE POSTS OR MULTIPLE STUDS SHALL BE CARRIED TO THE FOUNDATION. BLOCK JOISTS AT ALL SUPPORTS. DOUBLE JOISTS UNDER PARALLEL PARTITIONS. BLOCK UNDER PERPENDICULAR PARTITIONS AT 32" O.C.

PREFABRICATED ROOF WOOD TRUSSES:
TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE FOLLOWING STANDARDS:
NATIONAL DESIGN SPECS. AMERICAN INSTITUTE OF TIMBER CONSTRUCTION RULE NO. 16. DESIGN SPECS FOR LT. MTL. PLATE CONNECTED WOOD TRUSSES BY THE TRUSS PLATE INSTITUTE.
CAMBER TRUSSES FOR 1 1/2 TIMES THE DEAD LOAD DEFLECTION.

TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MIN. SUPERIMPOSED LOADING: ROOF TRUSSES, ASSUMED FOR TOTAL LOADING OF 20 PSF NON-CONCURRNET WITH THE FOLLOWING:

TOP CHORD DL = 10 psf
LL = 20 psf, (PLUS DRIFTING PER ASCE 7)
Cd= 1.0
BTM. CHORD DL= 15 psf
LL = 10 psf (NOT TO BE APPLIED AT THE SAME TIME AS TOP CHORD LL)

MAXIMUM DEFLECTION OF TRUSSES SHALL BE (DEAD + LIVE).
ROOF - L/240
WEB MEMBERS MAY NOT BE SHOWN ON DRAWINGS. MANUFACTURER TO DETERMINE WEB MEMBER CONFIGURATION AND TRUSS CONNECTION. NO TRUSS MAY BE ALTERED OR MODIFIED IN THE FIELD WITHOUT THE WRITTEN APPROVAL FROM THE ENGINEER. A COMPLETE SET OF SHOP DRAWINGS, ICC REPORTS AND CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO THE FABRICATION OF ANY MEMBERS. SUCH CALCULATIONS SHALL BE PREPARED UNDER THE DIRECTION OF A LICENSED ENGINEER IN THE STATE IN WHICH THE BUILDING IS TO BE BUILT.

QUALITY ASSURANCE PLAN:

SPECIAL INSPECTION AND TESTING AS REQUIRED BY THE IBC SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER UNLESS WAIVED BY THE BUILDING OFFICIAL. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH REQUIRED INSPECTIONS/TESTS AS INDICATED BELOW, REFERRING TO THE IBC SECTION INDICATED AS APPROPRIATE, REF IBC SECTION 1704.

CONCRETE WALLS/COLUMNS
STRUCTURAL STEEL

WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY:
AS PART OF THE QUALITY ASSURANCE AND SPECIAL INSPECTION PLAN, THE CONTRACTOR SHALL SUBMIT TO THE BUILDING OFFICIAL AND THE OWNER A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY CONTAINING THE FOLLOWING ITEMS:

1. ACKNOWLEDGMENT AND AWARENESS OF THE SPECIAL INSPECTION REQUIREMENTS.
2. ACKNOWLEDGMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
3. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF REPORTS.
4. IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

SHOP DRAWING DEVIATIONS:
WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS THEY SHALL BE DESIGNED AND STAMPED BY THE RESPONSIBLE SSE.

GENERAL CONTRACTOR'S PRIOR REVIEW:
ONCE THE CONTRACTOR HAS COMPLETED HIS REVIEW OF THE SSE COMPONENT DRAWINGS, THE SER WILL REVIEW THE SUBMITTAL ACCORDINGLY. REVIEW OF THE SPECIALTY STRUCTURAL ENGINEER'S (SSE) SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE SSE OF RESPONSIBILITY FOR THAT DESIGN. ALL NECESSARY BRACING, TIES, ANCHORAGE, PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE SSE'S DESIGN DRAWINGS AND CALCULATIONS. THESE ELEMENTS INCLUDE BUT ARE NOT LIMITED TO THE ITEMS LISTED BELOW.

DEFERRED SUBMITTAL ITEMS:
ITEMS REQUIRING DEFERRED SUBMITTALS THAT ARE LISTED BELOW ARE TO BE DESIGNED AND FABRICATED BY THE MANUFACTURER ACCORDING TO SPECIFICATIONS GIVEN IN THE CONSTRUCTION DOCUMENTS.

THESE DEFERRED SUBMITTALS SHALL FIRST BE SUBMITTED TO THE PROJECT ARCHITECT AND/OR ENGINEER FOR PREVIEW AND COORDINATION. UPON COMPLETION OF THE ARCHITECT/ENGINEER REVIEW, THE ARCHITECT/ENGINEER WILL SUBMIT THE DEFERRED SUBMITTALS TO THE BUILDING OFFICIAL FOR REVIEW AND APPROVAL. THE SUBMITTAL TO THE BUILDING OFFICIAL SHALL INCLUDE A LETTER STATING THAT THE ARCHITECT/ENGINEER REVIEW HAS BEEN PERFORMED AND THAT THE PLANS AND CALCULATIONS FOR THE DEFERRED SUBMITTAL ITEMS ARE FOUND TO BE IN GENERAL CONFORMANCE WITH THE DESIGN DRAWINGS WITH NO EXCEPTIONS.

THE FINAL SUBMITTAL SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE IN WHICH CONSTRUCTION WILL OCCUR AND SHALL BE AVAILABLE AT THE JOBSITE THROUGHOUT CONSTRUCTION.

CONSTRUCTION RELATED TO DEFERRED SUBMITTALS SHALL NOT COMMENCE UNTIL THE BUILDING OFFICIAL HAS APPROVED THE SUBMITTAL.

ALL MEMBERS SHALL CONFORM WITH AMERICAN AND IRON STEEL INSTITUTE (AISI) "SPECIFICATIONS FOR DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" CURRENT EDITION.

ALL MEMBERS SHALL BE FORMED FROM CORROSION-RESISTANT STEEL, CORRESPONDING TO THE REQUIREMENTS OF ASTM A1003-00.

ALL STRUCTURAL MEMBERS SHALL BE ZINC COATED MEETING ASTM A1003-00.

ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR MEMBERS OR, AS REQUIRED, FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS.

ALL FRAMING COMPONENTS SHALL BE PLUMBED, ALIGNED AND LEVELED.

ITEMS LISTED FOR DEFERRED SUBMITTAL SHALL BE DESIGNED BY SPECIALTY STRUCTURAL ENGINEERS LICENSED IN THE STATE OF UTAH. DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW (SEE NOTES THIS SHEET). THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE ARCHITECT, ENGINEER OF RECORD AND THE BUILDING OFFICIAL.

DEFERRED SUBMITTAL ITEMS:
1. METAL DECK SHOP DRAWINGS
2. OPEN WEB JOIST DESIGN AND SHOP DRAWINWG
3. AWNINGS
4. INTERIOR METAL STAIRS
5. CONCRETE DESIGN MIX

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PROFESSIONAL STRUCTURAL ENGINEER #5253654-2203 ANTHONY B. SCHMID STATE OF UTAH
02-13-19

NEW JAPANESE STEAKHOUSE & SUSHI
251 W. HILTON DR #202 P.O. BOX 2775
ST. GEORGE UTAH 84770

REVISIONS

REV	DATE	DESCRIPTION
1	02-13-19	REVIEW COMMENTS

PROJECT NO: 18448
DRAWN BY: GLO
CHECKED BY: ABS
DATE: 02/13/19
SHEET: 01 OF 14

GENERAL NOTES

S0.00

STRUCTURAL STEEL (IBC 1705.2.1, 1705.11.1 & 1705.12.2)

Item				Detailed Instructions and Frequencies
PRIOR TO WELDING (TABLE N5.4-1, AISC 360-10):				
Verify welding procedures (WPS) and consumable certificates	☒	Continuous	☐ Periodic	
Material identification	☐	Continuous	☒ Periodic	Verify type and grade of material.
Welder identification	☐	Continuous	☒ Periodic	A system shall be maintained by which a welder who has welded a joint or member can be identified.
Fit-up groove welds	☐	Continuous	☒ Periodic	Verify joint preparation, dimensions, cleanliness, tacking, and backing.
Access holes	☐	Continuous	☒ Periodic	Verify configuration and finish.
Fit-up fillet welds	☐	Continuous	☒ Periodic	Verify alignment, gaps at root, cleanliness of steel surfaces, and tack weld quality and location.
DURING WELDING (TABLE N5.4-2, AISC 360-10):				
Use of qualified welders	☐	Continuous	☒ Periodic	Verify that welders are appropriately qualified.
Control and handling of welding consumables	☐	Continuous	☒ Periodic	Verify packaging and exposure control.
Cracked tack welds	☐	Continuous	☒ Periodic	Verify that welding does not occur over cracked tack welds.
Environmental conditions	☐	Continuous	☒ Periodic	Verify win speed is within limits as well as precipitation and temperature.
WPS followed	☐	Continuous	☒ Periodic	Verify items such as settings on welding equipment, travel speed, welding materials, shielding gas type/flow rate, preheat applied, interpass temperature maintained, and proper position.
Welding techniques	☐	Continuous	☒ Periodic	Verify interpass and final cleaning, each pass is within profile limitations, and quality of each pass.
AFTER WELDING (TABLE N5.4-3, AISC 360-10):				
Welds cleaned	☐	Continuous	☒ Periodic	Verify that welds have been propyl cleaned.
Size, length, and location of welds	☒	Continuous	☐ Periodic	
Welds meet visual acceptance criteria	☒	Continuous	☐ Periodic	
Arc strikes	☒	Continuous	☐ Periodic	
k-area	☒	Continuous	☐ Periodic	
Backing & weld tabs removed	☒	Continuous	☐ Periodic	
Repair activities	☒	Continuous	☐ Periodic	
Document acceptance or rejection of welded joint/member	☒	Continuous	☐ Periodic	
NONDESTRUCTIVE TESTING (TABLE N5.5, AISC 360-10):				
CJP welds (Risk Cat. II)	☐	Continuous	☒ Periodic	Ultrasonic testing shall be performed on 10% of CJP groove welds in butt, T- and corner joints subject to transversely applied tension loading in materials 5/16-inch thick or greater. Testing rate must be increased if >5% of welds tested have unacceptable defects.
Size, length, and location of welds	☒	Continuous	☐ Periodic	**A reduction in the rate of ultrasonic testing is allowed per Section N5.5e.**
Welds meet visual acceptance criteria	☒	Continuous	☐ Periodic	
Arc strikes	☒	Continuous	☐ Periodic	
PRIOR TO BOLTING (TABLE N5.6-1, AISC 360-10):				
➢ Not required if only snug-tight joints are specified [per Section N5.6(1) of AISC 360-10]				
Certifications of fasteners	☒	Continuous	☐ Periodic	
Fasteners marked	☐	Continuous	☒ Periodic	Verify that fasteners have been marked in accordance with ASTM requirements.
Proper fasteners for joint	☐	Continuous	☒ Periodic	Verify grade, type, and bolt length if threads are excluded from the shear plane.
Proper bolting procedure	☐	Continuous	☒ Periodic	Verify proper procedure is used for the joint detail.
Connecting elements	☐	Continuous	☒ Periodic	Verify appropriate faying surface condition and hole preparation, if specified, meet requirements.
Pre-installation verification testing	☐	Continuous	☒ Periodic	Observe and document verification testing by installation personnel for fastener assemblies and methods used.
Proper storage	☐	Continuous	☒ Periodic	Verify proper storage of bolts, nuts, washers, and other fastener components.
DURING BOLTING (TABLE N5.6-2, AISC 360-10):				
➢ Not required if only snug-tight joints are specified [per Section N5.6(1) of AISC 360-10]				
➢ Not required for pretensioned joints using turn-of-th-nut method with match-marking, direct-tension-indicators, or twist-off type tension control method [per Section N5.6(2) of AISC 360-10]				
Fastener assemblies	☐	Continuous	☒ Periodic	Verify that fastener assemblies are of suitable condition, paced in all holes, and washers are positioned as required.
Snug-tight prior to pretensioning	☐	Continuous	☒ Periodic	Verify that joints are brought to snug-tight condition prior to pretensioning operation.
Fastener component	☐	Continuous	☒ Periodic	Verify that fastener component is not turned by wrench prevented from rotating.
Pretensioned fasteners	☐	Continuous	☒ Periodic	Verify that fasteners are Pretensioned in accordance with RCSC Specification, progressing systematically from the most rigid point toward the free edges.
AFTER BOLTING (TABLE N5.6-3, AISC 360-10):				
Document acceptance or rejection of bolted connections	☒	Continuous	☐ Periodic	
OTHER STEEL INSPECTIONS (SECTION N5.7, AISC 360-10; TABLES J8-1 & J10-1, AISC 341-10):				
Structural steel details	☐	Continuous	☒ Periodic	All fabricated steel or steel frames shall be inspected to verify compliance with the details shown in the construction documents, such as braces, stiffeners, member locations, and proper application of joint details at each connection.
Anchor rods and other embedments supporting structural steel	☐	Continuous	☒ Periodic	Shall be on the premises during the placement of anchor rods and other embedments supporting structural steel for compliance with construction documents. Verify the diameter, grade, type, and length of the anchor rod or embedded item, and the extent or depth of embedment prior to placement of concrete.
Reduced beam sections (RBS)	☐	Continuous	☒ Periodic	Verify contour and finish as well as dimensional tolerances (see Table J8-1 of AISC 341-10)
Protected zones	☐	Continuous	☒ Periodic	Verify that no holes or unapproved attachments are made within the protected zone (see Table J8-1 or AISC 341-10)
H-piles	☐	Continuous	☒ Periodic	Verify that no holes or unapproved attachments occur within the protected zones of piling (see Table J10-1 of AISC 341-10)
STEEL ELEMENTS OF COMPOSITE CONSTRUCTION (TABLES N6.1, AISC 360-10; TABLES J9-1 THRU J9-3, AISC 341-11)				
Placement and installation of steel deck	☒	Continuous	☐ Periodic	
Placement and installation of steel headed stud anchors	☒	Continuous	☐ Periodic	
Document acceptance or rejection of steel elements	☒	Continuous	☐ Periodic	
Reinforcing steel	☐	Continuous	☒ Periodic	Verify appropriate reinforcement size, spacing, and orientation; that it has not been re-bent in field; that it is correctly tied and supported; and that required steel clearances have been provided.
Composite member size	☐	Continuous	☒ Periodic	Verify that composite member is the required size.

CONCRETE CONSTRUCTION (IBC 1705.3 & 1705.12.1)

Item				Detailed Instructions and Frequencies
Reinforcing steel, including prestressing tendons	☐	Continuous	☒ Periodic	Verify prior to placing concrete that reinforcing is of specified type, grade and size; that it is free of oil, dirt and rust; that it is located and spaced properly; that hooks, bends, ties, stirrups and supplemental reinforcement are placed correctly; that lap lengths, stagger and offsets are provided; and that all mechanical connections are installed per the manufacturer's instructions and/or evaluation report.
Cast-in bolts & embeds	☐	Continuous	☒ Periodic	Inspection of anchors or embeds cast in concrete is required when allowable loads have been increased or where strength design is used.
Post-installed anchors or dowels	☐	Continuous	☒ Periodic	All post-installed anchors/dowels shall be specially inspected as required by the approved ICC-ES report.
Use of required mix design	☐	Continuous	☒ Periodic	Verify that all mixes used comply with the approved construction documents; ACI 318: Ch. 4, 5.2-5.4; and IBC 1904.3, 1913.2,1913.3.
Concrete sampling for strength tests, slump, air content, and temperature	☒	Continuous	☐ Periodic	
Concrete & shotcrete placement	☒	Continuous	☐ Periodic	
Curing temperature and techniques	☐	Continuous	☒ Periodic	Verify that the ambient temperature for concrete is kept at >50°F for at least 7 days after placement. High-early-strength concrete shall be kept at >50°F for at least 3 days. Accelerated curing methods may be used (see ACI 318:5.11.3). The ambient temperature for shotcrete shall be >40°F for the same period of time as noted for concrete. Shotcrete shall be kept continuously moist for at least 24 hours after shotcreting. All concrete materials, reinforcement, forms, fillers, and ground shall be free from frost. In hot weather conditions ensure that appropriate measures are taken to avoid plastic shrinkage cracking and that the specified water/cement ratio is not exceeded.
Pre-stressed concrete	☒	Continuous	☐ Periodic	
Erection of precast concrete	☐	Continuous	☒ Periodic	Verify that all precast elements are lifted, assembled and braced in accordance with the approved construction documents.
Strength verification	☐	Continuous	☒ Periodic	Verify that adequate strength has been achieved prior to the removal of shores and forms or the stressing of post-tensioned tendons.
Formwork	☐	Continuous	☒ Periodic	Verify that the forms are placed plumb and conform to the shapes, lines, and dimensions of the members as required by the approved construction documents.
Reinforcement complying with ASTM A 615 in special moment frames, special structural walls and coupling beams	☐	Continuous	☒ Periodic	Verify that ASTM A 615 reinforcing steel used in these areas complies with ACI 318: 21.1.5.2 by means of certified mill test reports. If this reinforcing steel is to be welded chemical tests shall be performed in accordance with ACI 318: 3.5.2.

SOILS CONSTRUCTION (IBC 1705.6)

Item				Detailed Instructions and Frequencies
Verify subgrade is adequate to achieve design bearing capacity	☐	Continuous	☒ Periodic	Prior to placement of concrete.
Verify excavations extend to proper depth and material	☐	Continuous	☒ Periodic	Prior to placement of compacted fill or concrete.
Verify that subgrade has been appropriately prepared prior to placing compacted fill	☐	Continuous	☒ Periodic	Prior to placement of compacted fill.
Perform classification and testing of compacted fill materials	☐	Continuous	☒ Periodic	All materials shall be checked at each lift for proper classifications and gradations not less than once for each 10,000 ft² of surface area.
Verify proper materials, densities and lift thicknesses during placement and compaction.	☒	Continuous	☐ Periodic	

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PROFESSIONAL ENGINEER #5253654-2203 ANTHONY B. SCHMID STATE OF UTAH

10-19-18

NEW JAPANESE STEAKHOUSE & SUSHI

251 W. HILTON DR #202 P.O. BOX 2775
ST. GEORGE UTAH 84770

REVISIONS

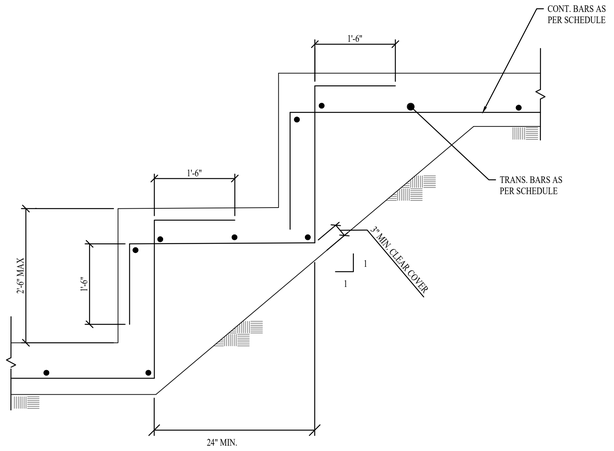
REV	DATE	DESCRIPTION

PROJECT NO:	18448
DRAWN BY:	GLO
CHECKED BY:	ABS
DATE:	10/19/18
SHEET:	02 OF 14

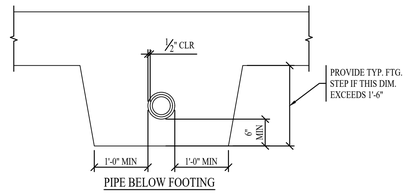
GENERAL NOTES

S0.01

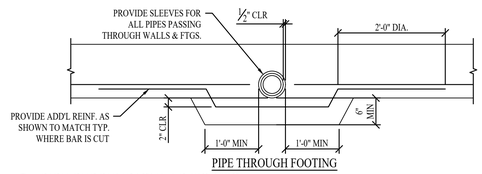
C:\Users\Anthony.MCNEIL-GROUP\Documents\Server\Current Projects\18448\18448_STRUCT (09-06-18).dwg Anthony Oct 19, 2018 - 9:35am



1-C S0.02 STEPPED FOOTING
N.T.S

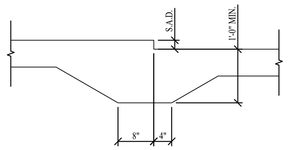


PIPE BELOW FOOTING

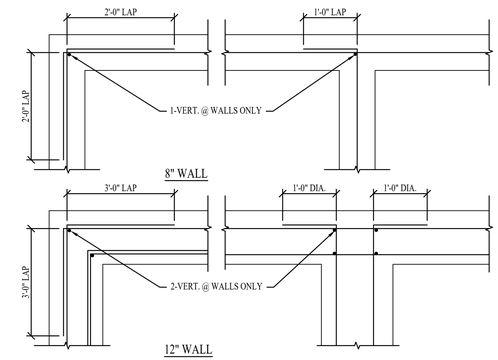


PIPE THROUGH FOOTING

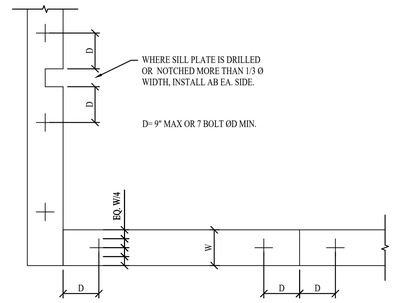
1-D S0.02 PIPE PERPENDICULAR TO FTG.
N.T.S



3-C S0.02 DEPRESSED SLAB ON GRADE
N.T.S



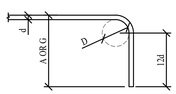
4-D S0.02 REINF. @ INTERSECTIONS
N.T.S



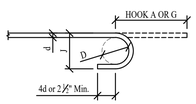
SEE PLANS & DETAILS FOR ANCHOR BOLT SIZE & SPACING

4-C S0.02 SILL PLATE BOLTING
N.T.S

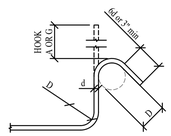
BAR SIZE		D		180°				90°		D		90°		135°			
				A OR G		J		A OR G				A OR G		A OR G		H	
U.S	SI	U.S	SI	U.S	SI	U.S	SI	U.S	SI	U.S	SI	U.S	SI	U.S	SI	U.S	SI
#3	#10	2½"	60	5	125	3	80	6	150	1½"	40	4"	105	4	105	2½"	65
#4	#13	3	80	6	160	4	105	8	200	2"	50	4½"	115	4½"	115	3"	80
#5	#16	3½"	96	7	175	5	130	10	250	2½"	65	6"	155	5½"	140	3¾"	95
#6	#19	4½"	115	8	200	6	155	1'-0"	300	4½"	115	1'-0"	305	8"	205	4½"	115
#7	#22	5½"	135	10	250	7	180	1'-2"	375	5½"	135	1'-2"	355	9"	230	5½"	135
#8	#25	6"	155	1'-1"	275	8	206	1'-4"	425	6"	155	1'-4"	410	10½"	270	6	155
#9	#29	9"	240	1'-3"	375	11½"	300	1'-7"	475	U.S. CUSTOMARY UNITS: in. or ft-in. SI UNITS: MM							
#10	#32	10½"	275	1'-7½"	425	1'-1½"	335	1'-10"	550								
#11	#36	11½"	305	1'-9½"	475	1'-2½"	375	2'-0"	600								
#14	#43	16 1½"	465	2'-3"	675	1'-9½"	550	2'-7"	7758								
#18	#57	22⅝"	610	3'-0"	925	2'-4½"	725	3'-5"	1050								



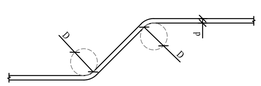
90° HOOK



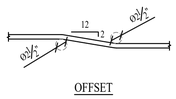
180° HOOK



135° C HOOK

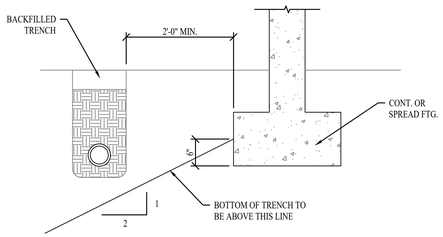


45° BEND

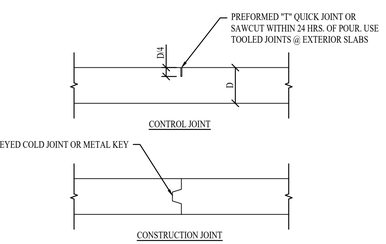


OFFSET

1-A S0.02 REINF. BENDS, HOOKS, AND OFFSET
N.T.S

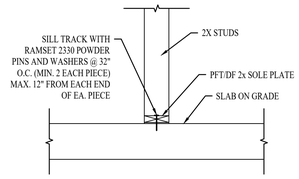


2-B S0.02 PIPE PARALLEL TO FTG.
N.T.S

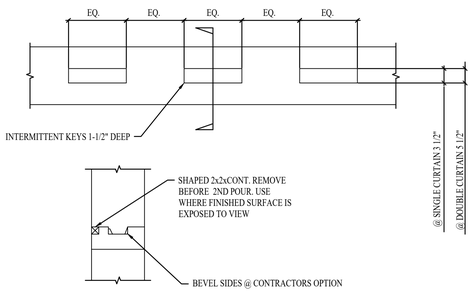


2-A S0.02 SLAB ON GRADE JOINT
N.T.S

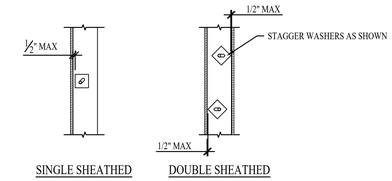
NOTE:
SPACE CONTROL AND/OR CONSTRUCTIONS JOINTS @ 16'-0" MAX. EACH WAY U.N.O. ON PLANS



SLAB ON GRADE
3-B S0.02 NON-BEARING WALL
N.T.S



3-A S0.02 CONST. JT. IN WALL OR FTG
N.T.S APPLIES TO BOTH VERT. & HORIZ. JOINTS



SINGLE SHEATHED DOUBLE SHEATHED

4-B S0.02 ANCHOR BOLT WASHER PLACEMENT
N.T.S

4-A S0.02 NOT USED
N.T.S

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PROFESSIONAL ENGINEER #5253654-2203 ANTHONY B. SCHMID STATE OF UTAH
10-19-18

NEW JAPANESE STEAKHOUSE & SUSHI
251 W. HILTON DR #202 P.O. BOX 2775
ST. GEORGE UTAH 84770

REVISIONS

REV	DATE	DESCRIPTION

PROJECT NO:	18448
DRAWN BY:	GLO
CHECKED BY:	ABS
DATE:	10/19/18
SHEET:	03 OF 14

TYPICAL STRUCTURAL DETAILS

S0.02

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NOTES:

- ALL FOOTINGS ADJACENT TO AREAS EXPOSED TO FREEZING TEMPT SHALL BE BELOW FINISH GRADE
- FOR LOCATION OF CONCRETE-ENCASED GROUNDING ELECTRODE (UFER) - PER E3508.12, NEC 250.50, REFERENCE ELECTRICAL PLANS
- PROVIDE ½" A.B. FOR 2X OR 3X SILL PLATE WITH 7" MIN EMBEDMENT @ 32" O.C. @ ALL SHEARWALL OR EXTERIOR LOCATIONS (UNLESS NOTED OTHERWISE) INTERIOR BEARING WALLS TO BE AT 32" ON CENTER WITH APPROVED 3x3x5/16" SQUARE PLATE WASHERS.
- HOLDOWN CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE APPROVED PLATE WASHERS; AND HOLDOWNS SHALL BE TIGHTENED JUST PRIOR TO COVERING THE WALL FRAMING.
- ALL BOLT HOLES SHALL BE DRILLED 1/16" TO 1/8" OVERSIZED
- ALL WASHERS TO BE APPROVED PLATE WASHERS
- DIMENSIONS ARE FOR REFERENCE ONLY. VERIFY ALL DIMENSIONS WITH ARCH PLANS.
- DO NOT SCALE PLANS. REFERENCE ARCH PLANS FOR ALL DIMENSIONS
- SEE ARCH PLANS FOR SLOPE OF SLABS.
- -CCX= INDICATES CONCRETE COLUMN SEE SHEET S4.00 FOR SCHEDULE AND DETAIL 2-D/S5.00 FOR CONCRETE COLUMN AT FOOTINGS
- -CWX= INDICATES CONCRETE FOUNDATION WALL SEE SHEET S4.00 FOR SCHEDULE
- -F-XX= INDICATES CONTINUOUS FOOTING SEE SHEET S4.00 FOR SCHEDULE
- -FS-XX=INDICATES SPOT FOOTING SEE SHEET S4.00 FOR SCHEDULE
- - SEE S0.0 FOR FROST DEPTH, VERIFY W/ LOCAL CITY REQUIREMENTS.

LEGENDS

- SW2 — INDICATES WOOD STRUCTURAL PANEL SHEAR WALL WITH LENGTH. SEE SHEAR PLAN FOR SHEAR WALL CALLOUTS, HOLDOWNS & SHEATHING INFORMATION
- INDICATES MAIN FLOOR WOOD SHEAR WALLS
- STHD# — -STHD SHEAR WALL HOLD DOWN SEE 2-B/S5.00
- S---------S — -INDICATES APPROXIMATE FOOTING STEP LOCATION, MIN 24" FROM OUTSIDE FACE OF FND
- A.B.@ ? — -INDICATES ANCHOR BOLT SPACING FOR WALL LINE IN INCHES OTHER THAN SPACING INDICATED IN NOTES FOR SW

FOOTING AND FOUNDATION PLAN

SCALE: 3/16"=1'-0"

0' 5' 10' 15' 30'

NOTE: DO NOT SCALE PLAN. USE AS REFERENCE ONLY VERIFY ALL DIMENSIONS ON ARCHITECTURAL PLANS

NORTH

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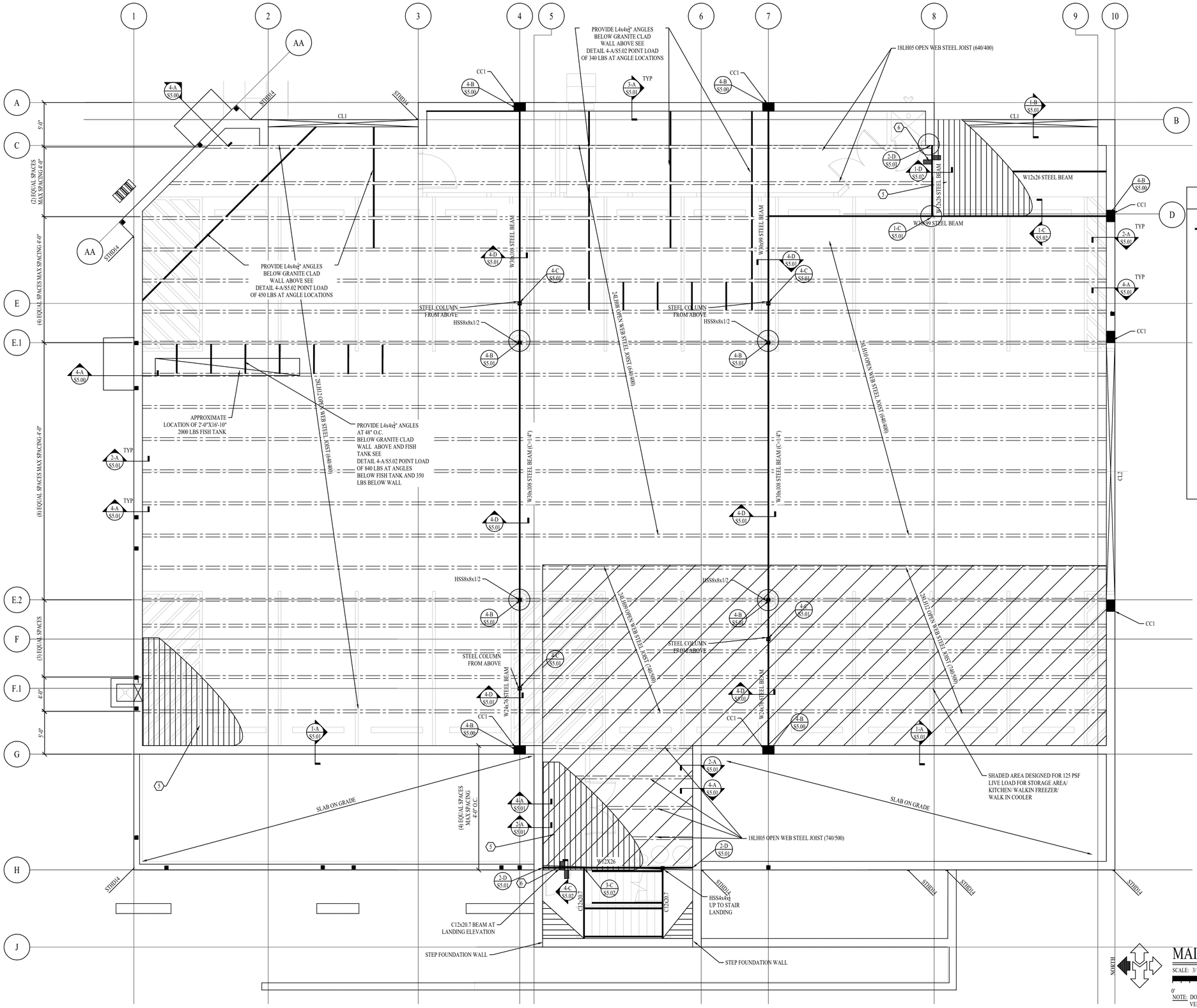
FOOTING & FOUNDATION PLAN

S1.00



KEYED NOTES:

1	12-1/4" THICK PREMIER SIPS PANEL WITH DOUBLE 2x SPLINE AT 48" O.C. SEE DETAIL 4-B/S5.03 FOR SPLINE.
2	APPROXIMATE LOCATIONS OF MECHANICAL UNITS TRUSS MANUFACTUER TO COORDINATE WITH MECHANICAL PLAN LOCATIONS AND WEIGHTS.
3	APPROXIMATE LOCATION OF ROOF ACCESS HATCH COORDINATE WITH ARCHITECTURAL DRAWINGS.
4	2x10 DF-L #2 ROOF RAFTERS AT 12" O.C.
5	FLOOR DECK: USE 'VULCRAFT' 1.5VL22 METAL DECK WITH 2-1/2" OF CONCRETE WITH 6x6 - W1.4xW1.4 WELDED WIRE FABRIC PLACED MID SLAB OVERALL THICKNESS 4". (3) SPAN MIN. WELD SHEETS TO SUPPORT W/ 5/8" DIA. FUSION WELDS W/ 36/4 PATTERN AND AT 18" O.C. WHEN FLUTES ARE PARALLEL TO SUPPORTS. WELD SIDE LAPS WITH 1-1/2" LONG TOP SEAM WELDS AT 24" O.C. (ALTERNATE EQUIVALENT FASTENERS MAY BE USED WITH PERMISSION FROM STRUCTURAL ENGINEER)
6	PROVIDE STEP IN DECK PER ARCHITECTURAL PLANS

FRAMING NOTES

- SEE $0.0 FOR LOADING INFORMATION
- INDICATES BEAM
- ALL EXTERIOR WALL FRAMING TO BE 2x6 DF-L STUD GRADE FRAMING AT 16" O.C. UNLESS NOTED OTHERWISE.
- PROVIDE FULL BEARING FOR ALL BEAMS ENDS, WIDTH OF COLUMN TO EQUAL WIDTH OF BEAM, U.N.O.. PROVIDE SQUASH BLOCKING AT FLOOR LEVEL
- PROVIDE (2) 2x6 TRIM STUDS AND (2) 2x6 KING STUDS FOR OPENINGS OVER 8'-0".
- STRAP & TIE ALL BEAMS TO POSTS, TO BE KING POST, COIL STRAP OR POST CAPS
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- SEE $0.00 FOR FASTENERS IN PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD
- SEE $0.02 FOR ADDITIONAL TYPICAL STRUCTURAL DETAILS

CCX -INDICATES CONCRETE COLUMN SEE SHEET $4.00 FOR SCHEDULE AND DETAILS
CLX -INDICATES LOCATION OF CONCRETE LINTEL SEE SHEET $4.00 FOR SCHEDULE AND DETAILS
(C=X) -INDICATES CAMBER ON STEEL BEAMS

MAIN FLOOR FRAMING PLAN

SCALE: 3/16"=1'-0"

0' 5' 10' 15' 30'

NOTE: DO NOT SCALE PLAN. USE AS REFERENCE ONLY
VERIFY ALL DIMENSIONS ON ARCHITECTURAL PLANS

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SHEET:	05 OF 14

MAIN FLOOR FRAMING PLAN

S1.01

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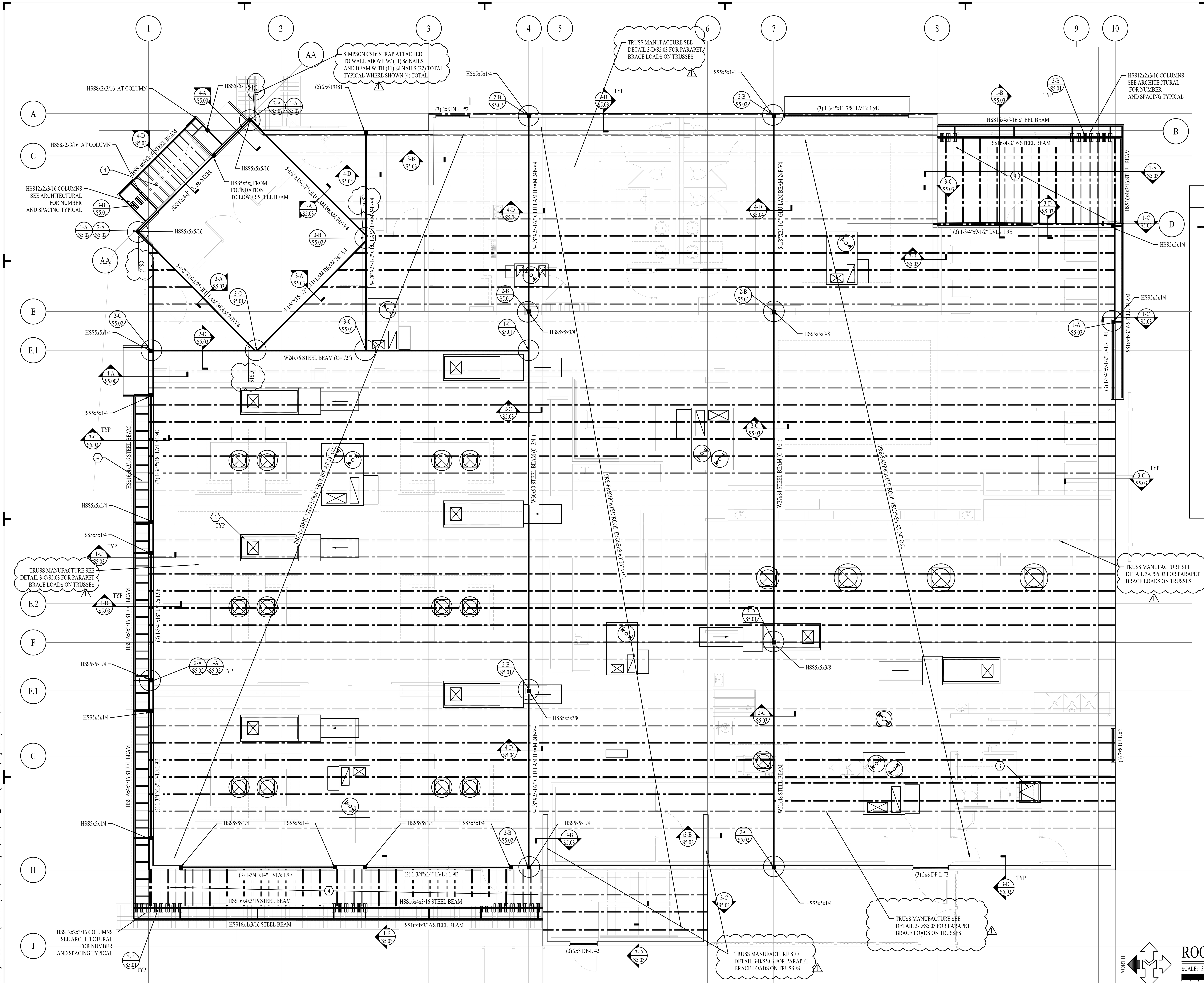

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- ROOF SHEATHING TO BE 5/8" OSB SHEATHING WITH A MIN. SPAN RATING OF 40/20 W/ 8d NAILS AT 6" O.C. PERIMIETER AND EDGE NAILING AND 12" O.C. FIELD NAILING.
- PROVIDE FULL BEARING FOR ALL BEAMS ENDS, WIDTH OF COLUMN TO EQUAL WIDTH OF BEAM, U.N.O.. PROVIDE SQUASH BLOCKING AT FLOOR LEVEL
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- SEE S0.02 FOR ADDITIONAL TYPICAL STRUCTURAL DETAILS

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CLX -INDICATES LOCATION OF CONCRETE LINTEL SEE SHEET S4.00 FOR SCHEDULE AND DETAILS
(C=X) -INDICATES CAMBER ON STEEL BEAMS

ROOF FRAMING PLAN

SCALE: 3/16"=1'-0"
0' 5' 10' 15' 30'

NOTE: DO NOT SCALE PLAN. USE AS REFERENCE ONLY
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02-13-19

NEW JAPANESE STEAKHOUSE & SUSHI

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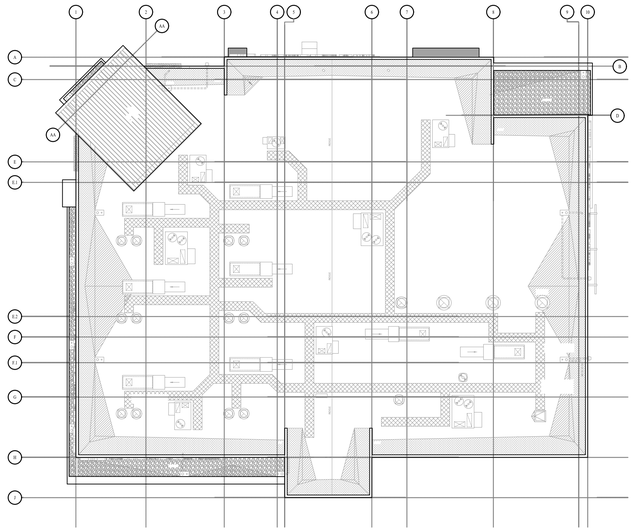
REVISIONS

REV	DATE	DESCRIPTION
1	02-13-19	REVIEW COMMENTS

PROJECT NO:	18448
DRAWN BY:	GLO
CHECKED BY:	ABS
DATE:	02/13/19
SHEET:	06 OF 14

ROOF FRAMING PLAN

S1.02



ROOF LAYOUT

SCALE 1/16" = 1'-0"

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⚠1 - ROOF SHEATHING TO BE 7/16" OSB SHEATHING WITH A MIN. SPAN RATING OF 40/20 W/ 8d NAILS AT 6" O.C. PERIMETER AND EDGE NAILING AND 12" O.C. FIELD NAILING

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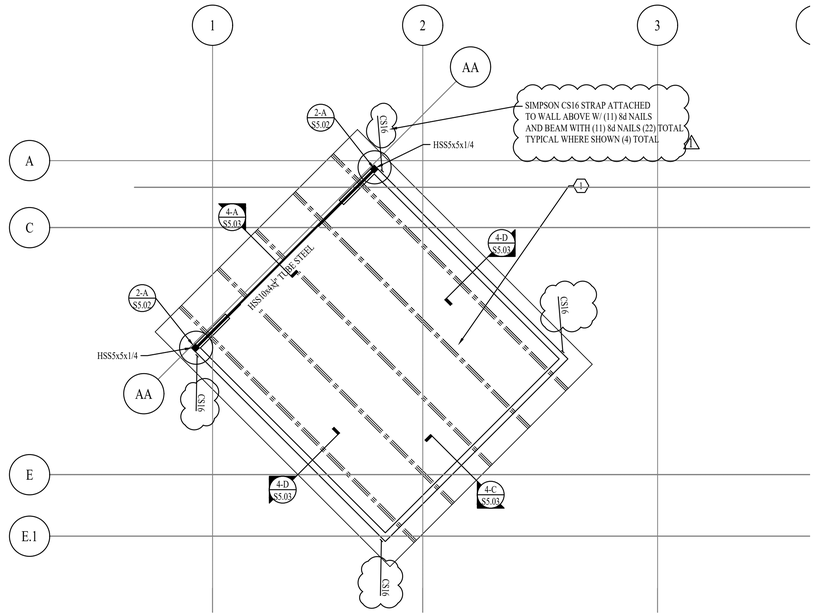

NORTH

UPPER ROOF FRAMING PLAN

SCALE: 3/16"=1'-0"

0' 5' 10' 15' 30'

NOTE: DO NOT SCALE PLAN. USE AS REFERENCE ONLY
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SHEET:	07 OF 14

UPPER ROOF FRAMING PLAN

S1.03

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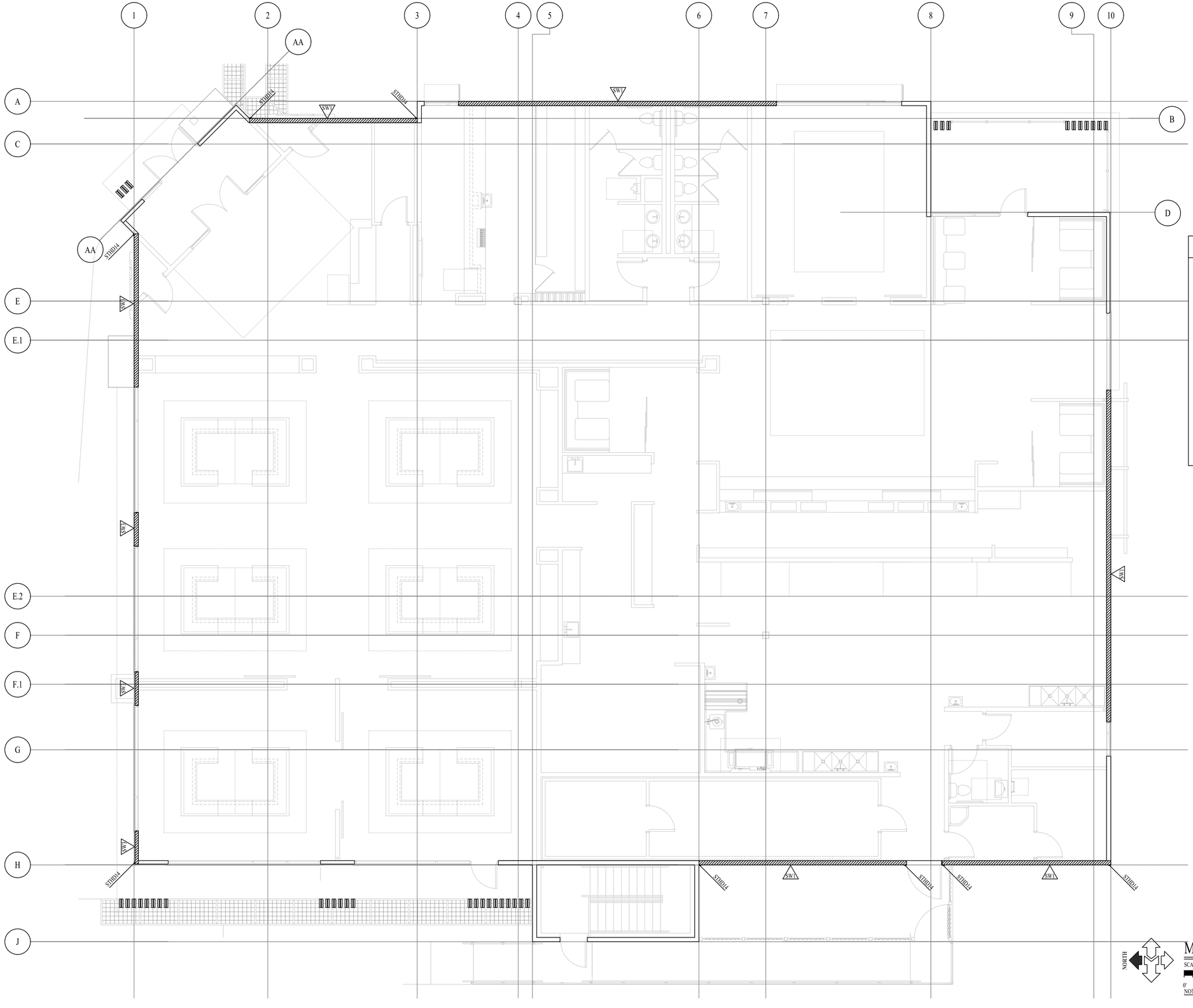

SHEAR WALL NOTES

SW2 - SEE S0.0 FOR LOADING INFORMATION
- INDICATES WOOD STRUCTURAL PANEL SHEAR WALL AND APPROXIMATE LENGTH SEE DETAIL 1-B/S5.04 FOR SHEAR WALL SCHEDULE AND SHEAR WALL DETAIL.

STHDXX - INDICATES LOCATION OF SIMPSON STHDXX SEE DETAIL 2-B/S5.00

NOTE:
HOLD DOWNS SHOWN ON THIS SHEET ARE FOR THESE SHEAR WALLS AND SHOULD EXTEND DOWN TO LOWER LEVEL.

NORTH

MAIN FLOOR SHEAR WALL PLAN

SCALE: 3/16"=1'-0"

0' 5' 10' 15' 30'

NOTE: DO NOT SCALE PLAN. USE AS REFERENCE ONLY
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MAIN FLOOR SHEAR WALL PLAN

S1.04

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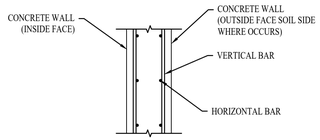
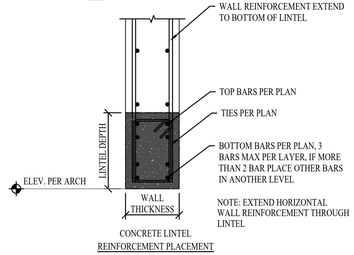
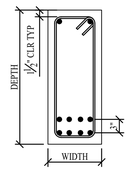
CONCRETE WALL REINFORCING SCHEDULE							
MARK	THICK	INSIDE FACE		OUTSIDE FACE		TOP & BOTTOM	REMARKS
		VERT. REINF.	HORIZ. REINF.	VERT. REINF.	HORIZ. REINF.		
CW8A	8"	#5 @ 12" O.C.	#5 @ 12" O.C.	-	-	(1) #5	PLACE REINF. CENTER OF WALL
CW12A	12"	#6 @ 12" O.C.	#5 @ 18" O.C.	#5 @ 18" O.C.	#5 @ 18" O.C.	(2) #5	2 CURTAINS OF REINF.
CW12B	12"	#5 @ 18" O.C.	#5 @ 18" O.C.	#5 @ 18" O.C.	#5 @ 18" O.C.	(2) #5	2 CURTAINS OF REINF.
CW12C	12"	#6 @ 12" O.C.	#5 @ 9" O.C.	#5 @ 18" O.C.	#5 @ 9" O.C.	(2) #5	2 CURTAINS OF REINF.
NOTE: PROVIDE (2) VERTICAL #5 BARS AT CORNERS, END OF WALLS, AND JAMBS OF OPENINGS							



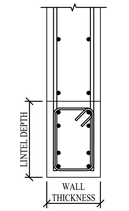
CONCRETE LINTEL REINFORCING SCHEDULE							
MARK	MAX SPAN	MIN. DEPTH	MIN. WIDTH	STEEL		TIES	COMMENTS
				TOP	BOTTOM		
CL1	19'-0"	36"	WALL	(2) #5	(4) #5	#3 @ 14" O.C.	
CL2	30'-0"	42"	WALL	(2) #5	(8) #8	#4 @ 14" O.C.	PLACE BOTTOM REINFORCING IN (2) LAYERS



CONCRETE LINTEL
REINFORCEMENT PLACEMENT

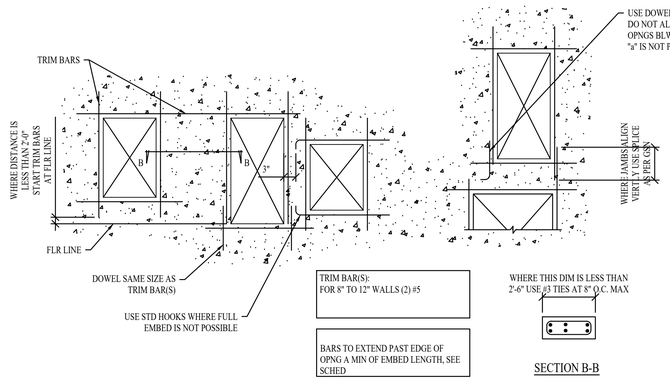


LINTEL NO WALL ABOVE



CONCRETE LINTEL REINFORCEMENT PLACEMENT (CL2 SHOWN)

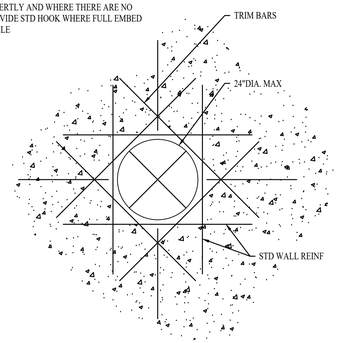
FOOTING SCHEDULE													
MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				REMARKS	DEPTH OF STRUCTURAL FILL*
				NO	SIZE	LENGTH	SPACING	NO	SIZE	LENGTH	SPACING		
F-24	24"	CONT	12"	-	-	-	-	2	#5	CONT	EQ	CONTINUOUS FOOTING	2'-0"
F-36	36"	CONT	14"	-	-	-	-	3	#5	CONT	EQ	CONTINUOUS FOOTING	2'-0"
F-48	48"	CONT	14"	-	#5	3'-6"	18" O.C.	4	#5	CONT	EQ	CONTINUOUS FOOTING	3'-0"
FS-3	3'-0"	3'-0"	12"	3	#5	2'-6"	EQ	3	#5	2'-6"	EQ	SPOT FOOTING	2'-0"
FS-4	4'-0"	4'-0"	12"	4	#5	3'-6"	EQ	4	#5	3'-6"	EQ	SPOT FOOTING	2'-0"
FS-4.5	4'-6"	7'-6"	12"	7	#5	4'-0"	EQ	4	#5	7'-0"	EQ	SPOT FOOTING	2'-0"
FS-6.5	6'-6"	6'-6"	14"	7	#5	6'-0"	EQ	7	#5	6'-0"	EQ	SPOT FOOTING	3'-0"
FS-7	7'-0"	7'-0"	14"	8	#5	6'-6"	EQ	8	#5	6'-6"	EQ	SPOT FOOTING	3'-0"
FS-8	8'-0"	8'-0"	16"	10	#5	7'-6"	EQ	10	#5	7'-6"	EQ	SPOT FOOTING	3'-0"
FS-9	9'-0"	9'-0"	24"	16	#5	8'-6"	EQ	18	#5	8'-6"	EQ	SPOT FOOTING PROVIDE (2) MATS OF REINFORCING PLACE HALF THE REQUIRED REINFORACING 2" FROM TOP OF FOOTING AND HALF 3" FROM BOTTOM OF FOOTING	4'-0"
FS-10	10'-0"	10'-0"	24"	22	#5	9'-6"	EQ	22	#5	9'-6"	EQ	SPOT FOOTING PROVIDE (2) MATS OF REINFORCING PLACE HALF THE REQUIRED REINFORACING 2" FROM TOP OF FOOTING AND HALF 3" FROM BOTTOM OF FOOTING	4'-0"
NOTES: *STRUCTURAL FILL PER SOILS REPORT SEE STEPPED FOUNDATION DETAIL WHERE FOOTING STEP IS REQUIRED													



TRIM BAR(S):
FOR 8" TO 12" WALLS (2) #5

BARS TO EXTEND PAST EDGE OF OPNG A MIN OF EMBED LENGTH, SEE SCHED

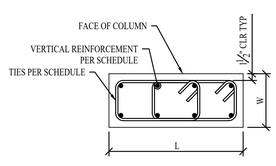
SECTION B-B



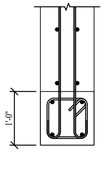
TYP TRIM BARS AROUND MISCELLANEOUS CONC WALL OPENINGS U.N.O.

REINFORCEMENT PLACEMENT

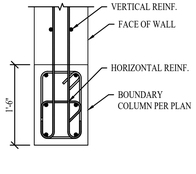
CONCRETE COLUMN SCHEDULE			
MARK	SIZE	VERT. REINF.	TIE CONFIG.
CC1	18" x WALL THICKNESS	(8) #5	#3 TIES @ 8" O.C.



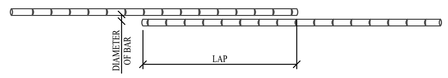
STAND ALONE COLUMNS



TYP. WINDOW & DOOR JAMB



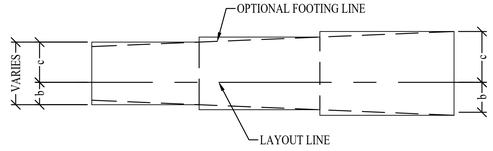
TYP. ENDS OF WALL

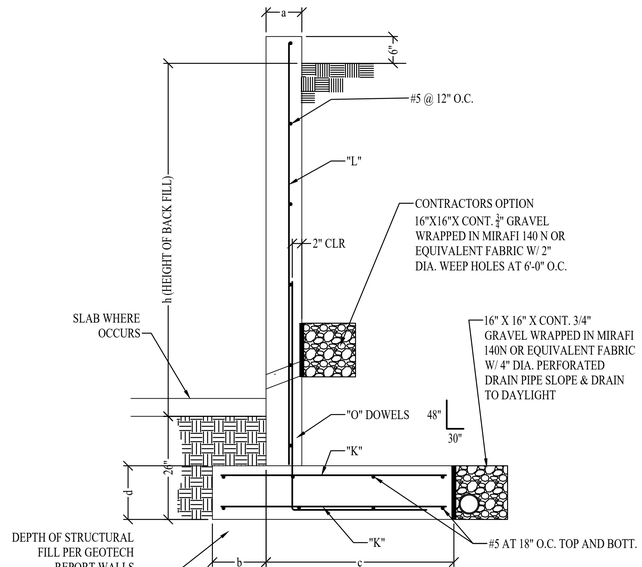


CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE																
REBAR SIZE	f'c = 3000 psi				f'c =4000 psi				f'c = 5000 psi				f'c = 6000 psi			
	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP	
	CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS	
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B
#4	17"	22"	22"	28"	15"	19"	19"	24"	13"	17"	17"	22"	12"	16"	16"	20"
#4	22"	29"	29"	38"	19"	25"	25"	33"	17"	22"	22"	29"	16"	21"	21"	27"
#5	28"	36"	36"	47"	24"	31"	31"	41"	22"	28"	28"	36"	20"	26"	26"	33"
#6	33"	43"	43"	56"	29"	37"	37"	49"	26"	34"	34"	43"	24"	31"	31"	40"
#7	49"	63"	63"	81"	42"	54"	54"	70"	37"	48"	48"	63"	35"	45"	45"	58"
#8	55"	72"	72"	93"	47"	62"	62"	80"	42"	55"	71"	71"	39"	51"	51"	66"
#9	62"	81"	81"	105"	53"	69"	69"	90"	48"	62"	62"	81"	44"	58"	58"	75"
#10	70"	91"	91"	118"	60"	78"	78"	101"	54"	70"	70"	91"	50"	65"	65"	84"
#11	78"	101"	101"	131"	67"	87"	87"	112"	60"	77"	77"	100"	55"	72"	72"	93"

CONCRETE REINFORCING BAR LAP SPLICE NOTES:
1. THIS SCHEDULE SHALL BE USED FOR ALL SPLICES, UNLESS NOTED OTHERWISE.
2. HORIZONTAL BARS ARE CLASSIFIED AS TOP BARS WHERE 12", OR MORE, OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BARS.
3. CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS NOTED OTHERWISE.
4. TIES AND STIRRUPS SHALL NOT BE SPLICED.
5. FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
6. FOR ALL EPOXY COATED BARS, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3 FOR TOP BARS AND 1.5 FOR REGULAR BARS.
7. LAP LENGTHS SHALL BE MULTIPLIED BY 1.25 AT SHEARWALL BOUNDARY ELEMENTS AT FOOTINGS AND AT MOMENT FRAME BEAMS TO COLUMNS.

USE REINFORCEMENT FOR MAX h OF EACH SEGMENT





RETAINING WALL SCHEDULE						
WALL DIMENSIONS					REINFORCING BARS	
					STEM	
h	a	b	c	d	"L"	"O"	"K"
3'-0"	10"	1'-0"	2'-6"	14"	#5 @ 16" O.C.	#5 @ 16" O.C.	-
5'-0"	10"	1'-0"	4'-6"	14"	#5 @ 14" O.C.	#5 @ 14" O.C.	-
7'-0"	10"	1'-0"	6'-0"	14"	#5 @ 10" O.C.	#5 @ 10" O.C.	#5 @ 10" O.C.
9'-0"	AT 9'-0" RAMP WALL AT CORNER USE FOOTING AND WALL REQUIREMENTS FOR 7'-0" WALL						

4-A / S4.00 RAMP RETAINING WALL DETAIL
N.T.S

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PROFESSIONAL ENGINEER #5253654-2203 ANTHONY B. SCHMID STATE OF UTAH
02-13-19

NEW JAPANESE STEAKHOUSE & SUSHI
251 W. HILTON DR #202 P.O. BOX 2775
ST. GEORGE UTAH 84770

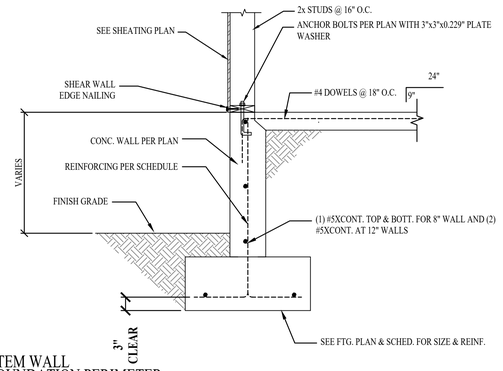
REVISIONS		
REV	DATE	DESCRIPTION
A	02-13-19	REVIEW COMMENTS

PROJECT NO:	18448
DRAWN BY:	GLO
CHECKED BY:	ABS
DATE:	02/13/19
SHEET:	09 OF 14

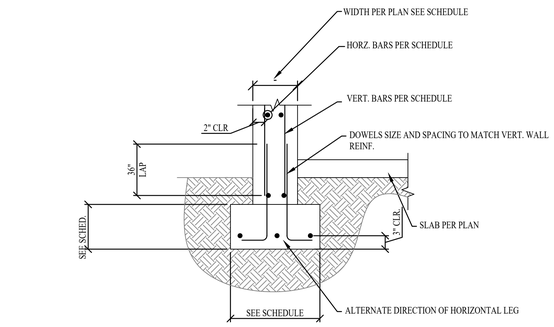
CONCRETE DETAILS & SCHEDULES

S4.00

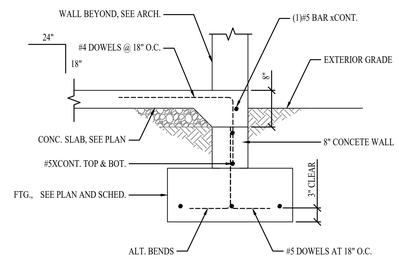
1-D S5.00 NOT USED
N.T.S



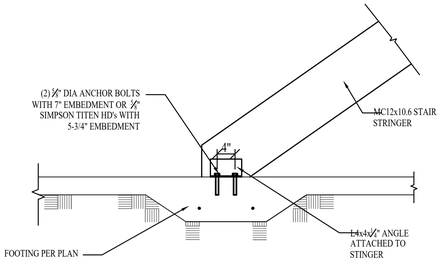
1-C S5.00 STEM WALL FOUNDATION PERIMETER
N.T.S



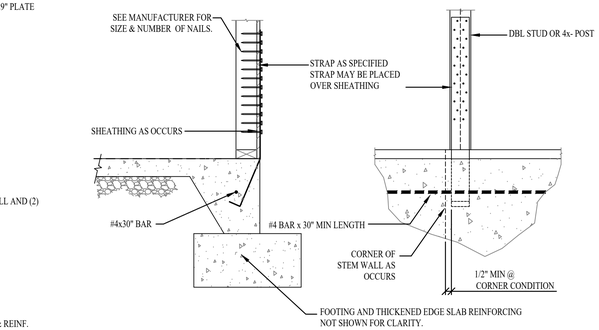
2-D S5.00 WALK OUT BASEMENT FOUNDATION
N.T.S



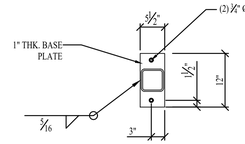
3-D S5.00 EXTERIOR WALL FTG. DETAIL @ OPENING
N.T.S



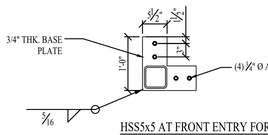
4-D S5.00 STAIR STRINGER TO SLAB
N.T.S



SLABE ON GRADE FOOTING

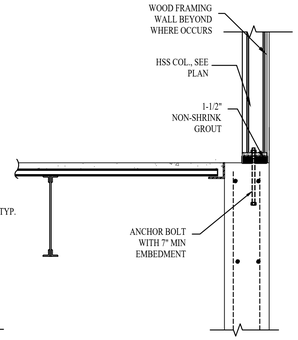


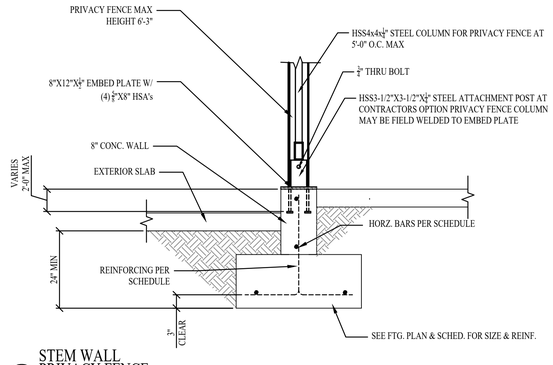
HSS 5X5 AT BASEMENT WALL



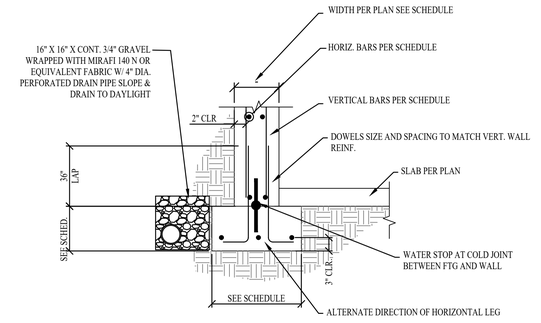
HSS5x5 AT FRONT ENTRY FOR 27'-0" TALL COLULMN

3-C S5.00 STL. COLUMN BASEMENT WALL
N.T.S

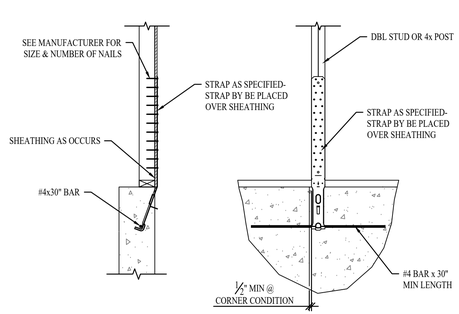




4-C S5.00 STEM WALL PRIVACY FENCE
N.T.S

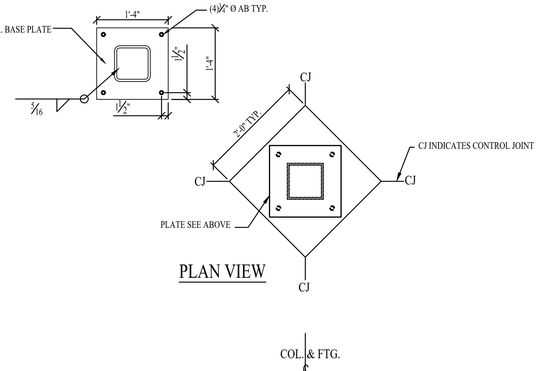


1-B S5.00 BASEMENT FOUNDATION
N.T.S

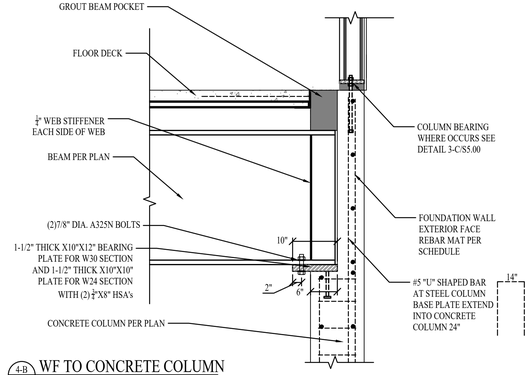


FOUNDATION WALL

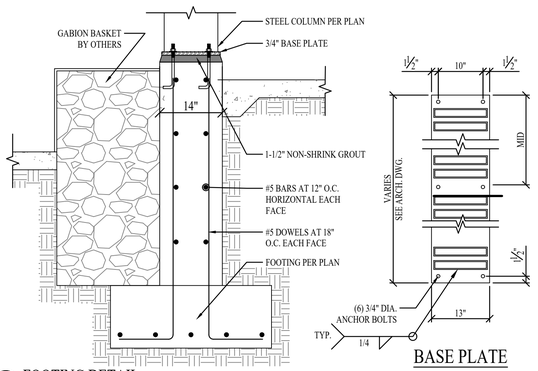
2-B S5.00 STRAP TIE HOLDOWN STEM
N.T.S



PLAN VIEW

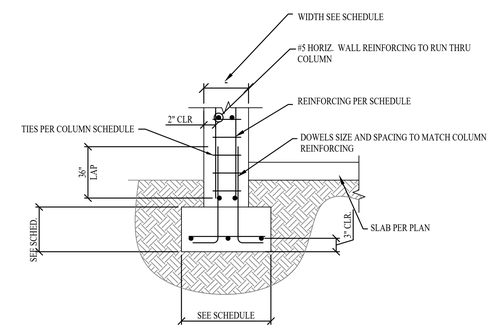


4-B S5.00 WF TO CONCRETE COLUMN
N.T.S



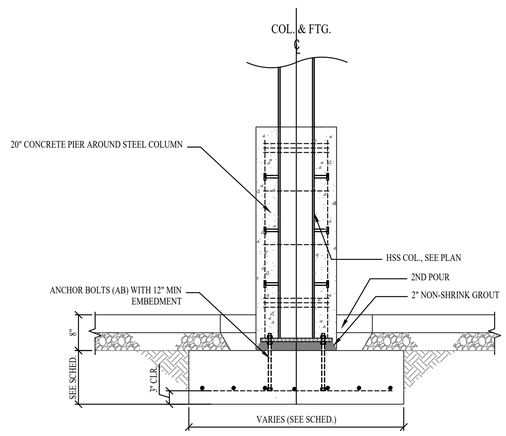
BASE PLATE

1-A S5.00 FOOTING DETAIL
N.T.S

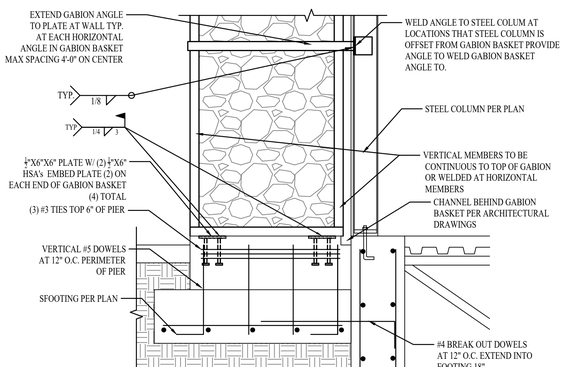


CONCRETE COLUMN BASEMENT FOUNDATION

2-D S5.00 N.T.S



3-A S5.00 STL. COLUMN FOOTING DETAIL
N.T.S



4-A S5.00 SECTION
N.T.S

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PROFESSIONAL ENGINEER #5253654-2203 ANTHONY B. SCHMID STATE OF UTAH
10-19-18

NEW JAPANESE STEAKHOUSE & SUSHI
251 W. HILTON DR #202 P.O. BOX 2775
ST. GEORGE UTAH 84770

REVISIONS		
REV	DATE	DESCRIPTION

PROJECT NO:	18448
DRAWN BY:	GLO
CHECKED BY:	ABS
DATE:	10/19/18
SHEET:	10 OF 14

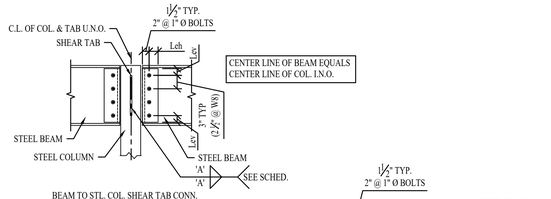
STRUCTURAL DETAILS

S5.00

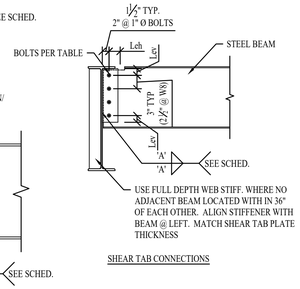
C:\Users\Anthony.MCNEIL-GROUP\Documents\Server\Current Projects\18448\18448_STRUCT (09-06-18).dwg Anthony Oct 19, 2018 - 9:36am

BEAM CONNECTION SCHEDULE

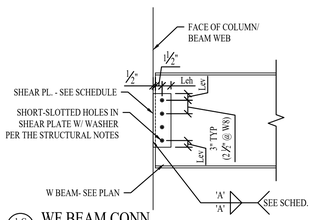
BEAM DEPTH	SHEAR PLATE INFORMATION			BOLTS W/ STANDARD WASHERS OVER SLOTS		WELD 'A'	COMMENTS
	PL. DIMENSIONS W/ SHORT-SLOTTED HOLES	Lev	Leh	NO.	SIZE		
W8 x W10 x	PL 5/16" x 4"	1 1/4"	2"	2	7/8" Ø	5/16"	
W12 x	PL 5/16" x 4"	1 1/4"	2"	3	7/8" Ø	5/16"	
W14 x 90 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	3	7/8" Ø	5/16"	
W16 x 77 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	4	7/8" Ø	5/16"	
W18 x 65 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	5	7/8" Ø	5/16"	
W21 x 73 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	6	7/8" Ø	5/16"	
W24 x 94 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	7	7/8" Ø	5/16"	
W27 x 114 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	7	7/8" Ø	5/16"	
W30 x 124 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	8	1" Ø	5/16"	
W33 x 130 & LIGHTER	PL 5/16" x 4"	1 1/4"	2"	9	1" Ø	5/16"	
W36 x 160 & LIGHTER	PL 5/16" x 4 1/2"	2"	2 1/2"	10	1 1/4" Ø	5/16"	



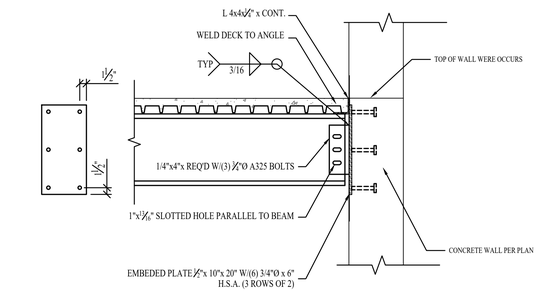
BEAM TO STL. COL. SHEAR TAB CONN.



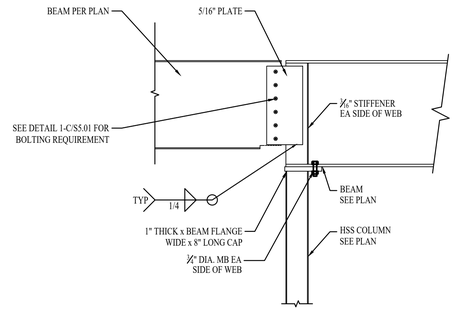
SHEAR TAB CONNECTIONS



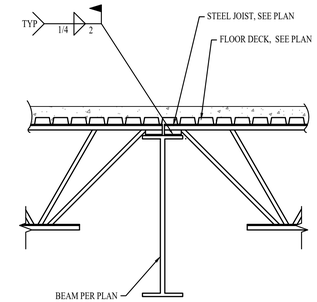
1-C S5.01 WF BEAM CONN.
N.T.S



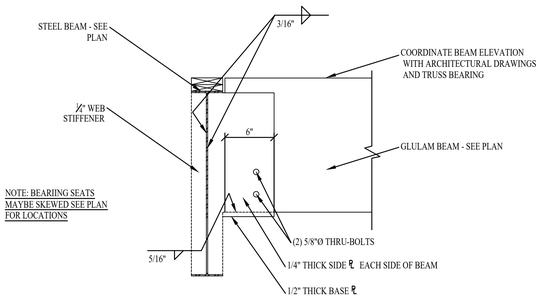
2-D S5.01 BEAMS TO CONCRETE WALL
N.T.S



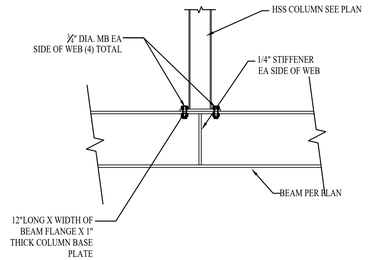
3-D S5.01 HSS COLUMN TOP
N.T.S



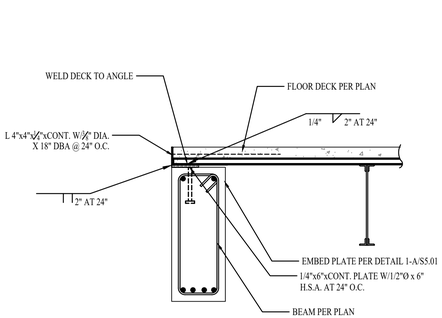
4-D S5.01 JOIST BEARING DETAIL WIDE FLANGE
N.T.S



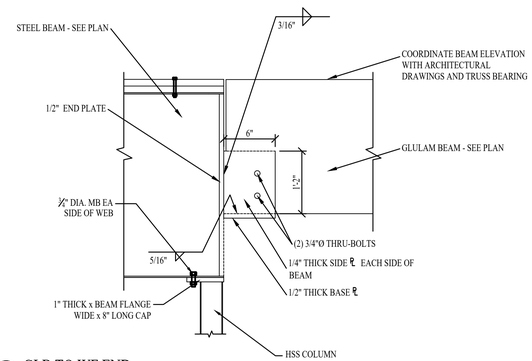
3-C S5.01 GLU LAM TO WF CONN.
N.T.S



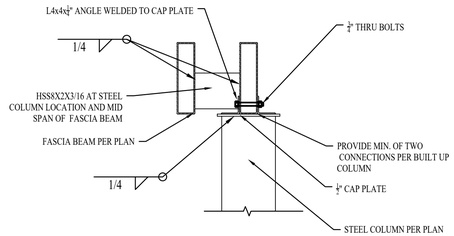
4-C S5.01 COLUMN TO WF BEAM
N.T.S



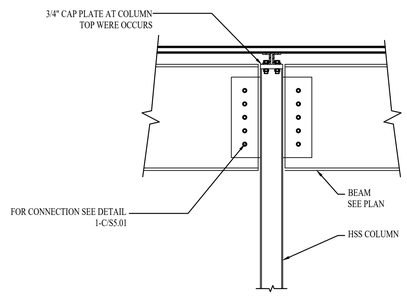
1-B S5.01 SECTION
N.T.S



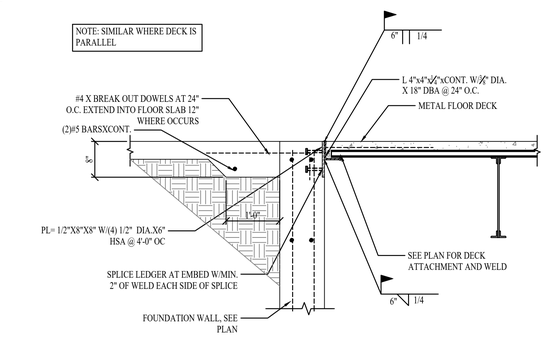
2-B S5.01 GLB TO WF END
N.T.S



3-B S5.01 COLUMN CAP
N.T.S



4-B S5.01 WF BEARING AT COLUMN
N.T.S



1-A S5.01 SECTION
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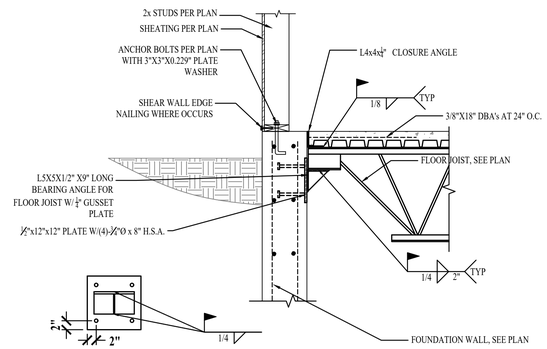

2-A S5.01 FLOOR JOIST BEARING DETAIL
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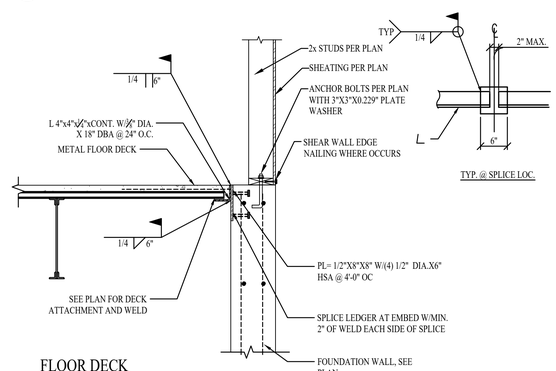

3-A S5.01 FLOOR DECK
BEARING DETAIL-RIBS PURP.
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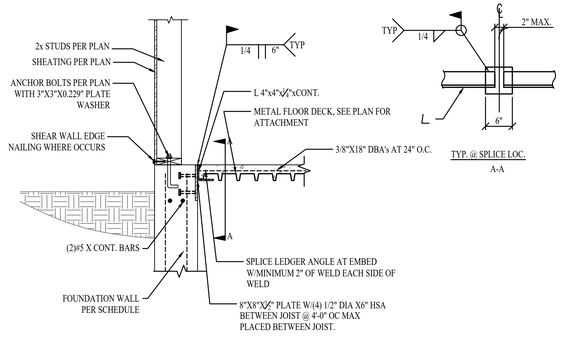

4-A S5.01 DECK BEARING DETAIL- RIBS PARAL.
N.T.S

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REVISIONS

REV	DATE	DESCRIPTION

PROJECT NO:	18448
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CHECKED BY:	ABS
DATE:	10/19/18
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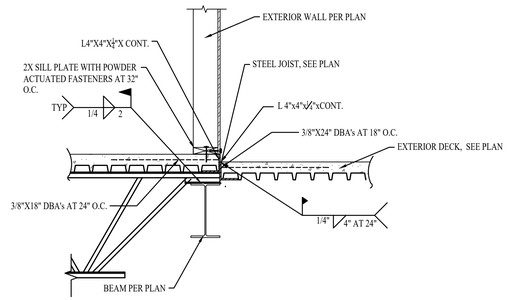
STRUCTURAL DETAILS

S5.01



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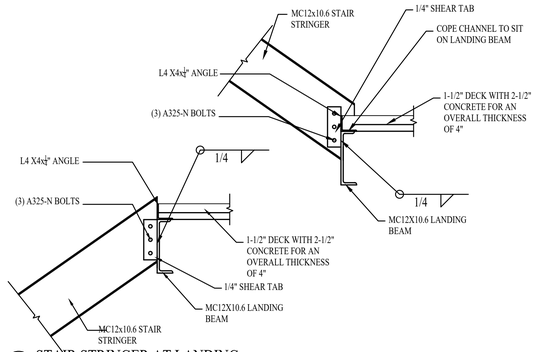

2-D S5.02 STAIR STRINGER AT LANDING
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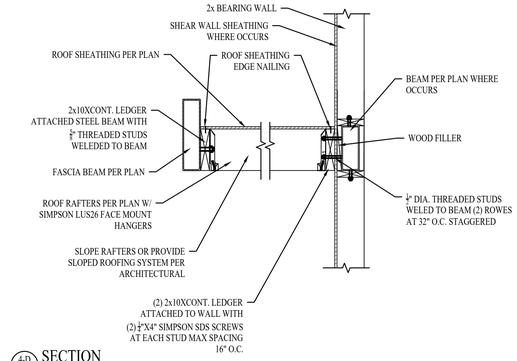

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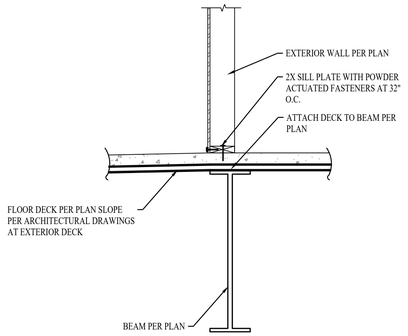

1-C S5.02 SECTION
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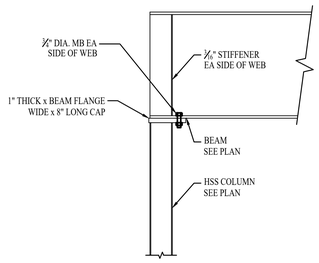

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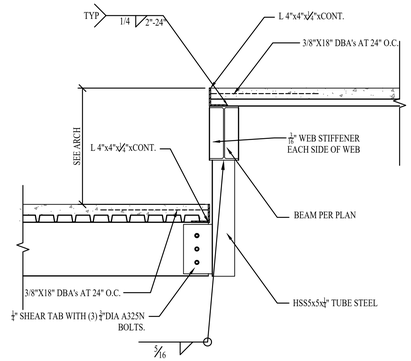

3-C S5.02 CONN. DETAIL
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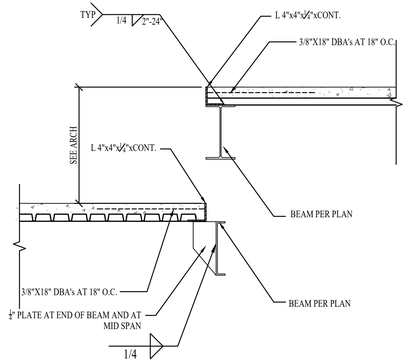

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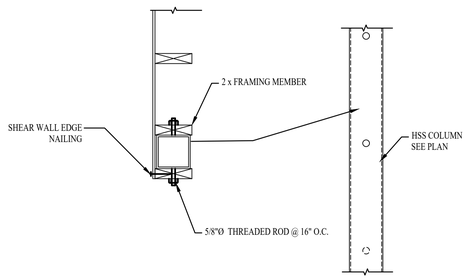

2-A S5.02 STEEL COLUMN AT END OF SHEAR WALL
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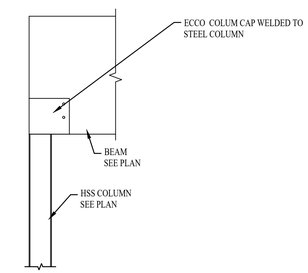

2-B S5.02 GLB TO HSS COLUMN
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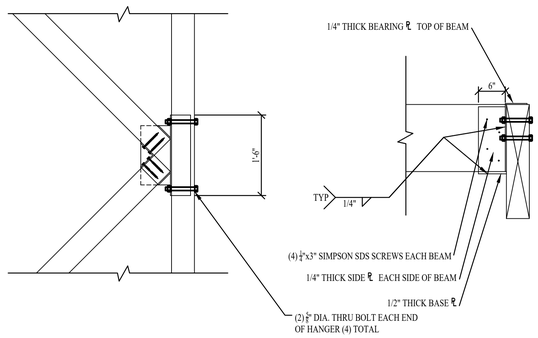

3-B S5.02 GLB TO GLB DETAIL
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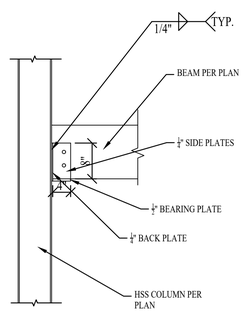

1-A S5.02 WOOD BEAM TO STEEL COLUMN
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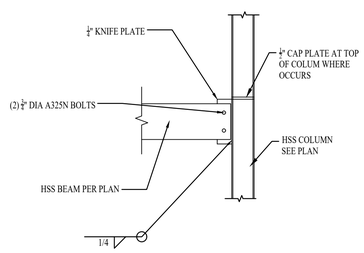

2-A S5.02 HSS BEAM TO HSS COLUMN
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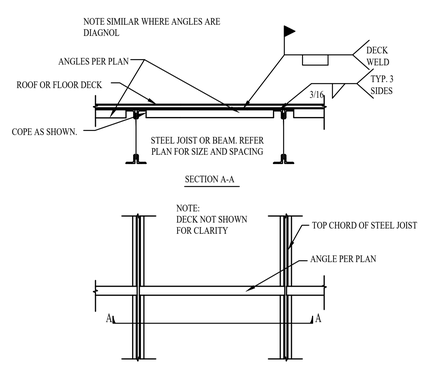

4-A S5.02 ANGLE SUPPORTS
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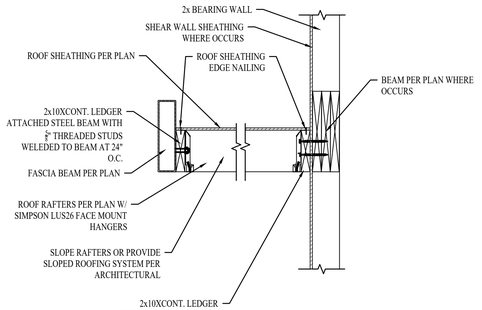

1-D S5.03 SECTION
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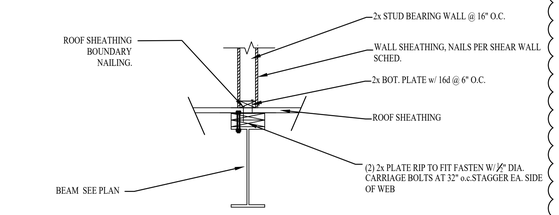

2-D S5.03 SECTION
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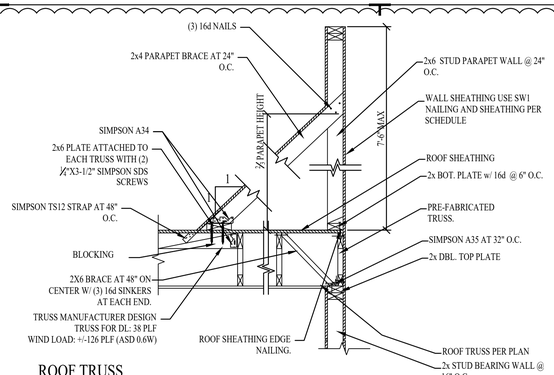

3-D S5.03 ROOF TRUSS BEARING PARALLEL
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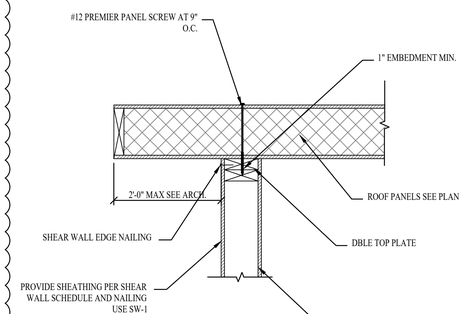
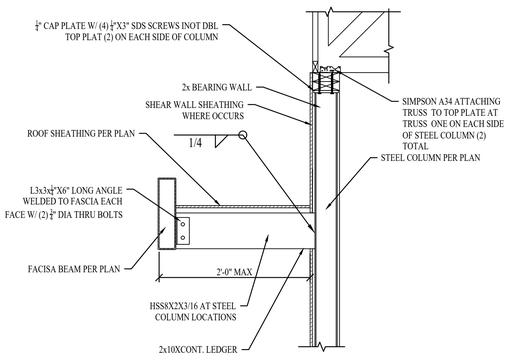


4-D S5.03 SIP's ROOF PARRALLEL TO WALL CONN.
N.T.S



1-C S5.03 SECTION
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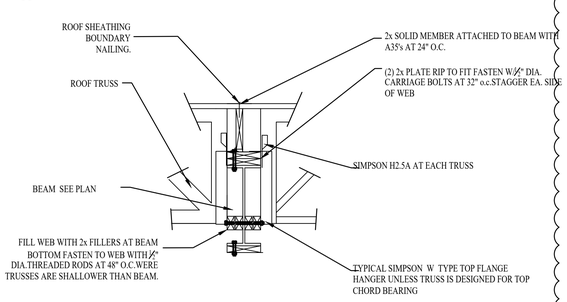

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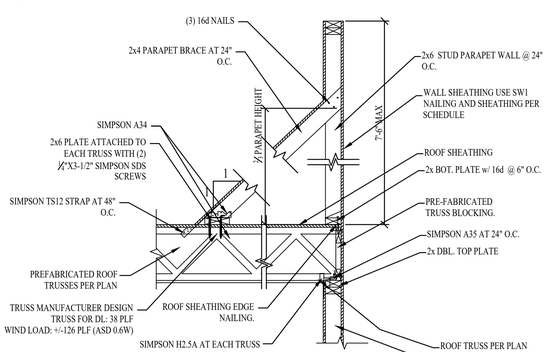

3-C S5.03 ROOF TRUSS BEARING PERP.
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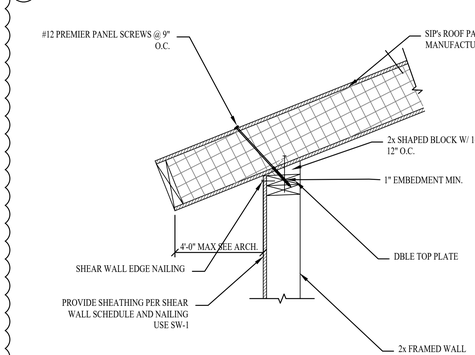
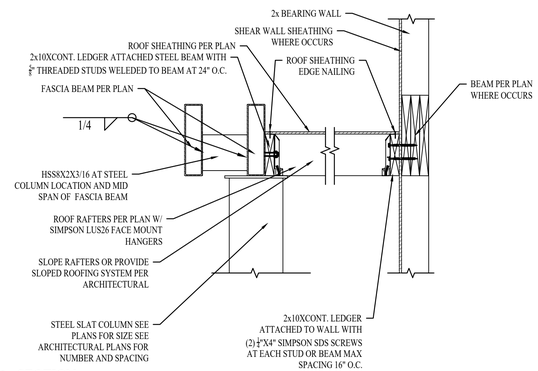


4-C S5.03 SIP's ROOF PERPENDICULAR TO WALL CONN. SLOPED
N.T.S



1-B S5.03 SECTION
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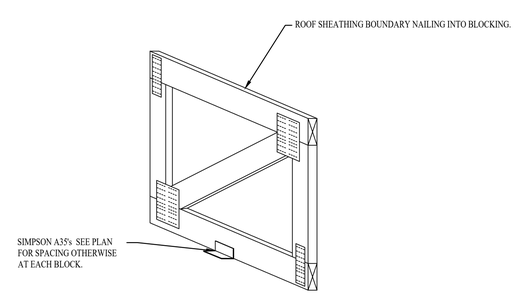
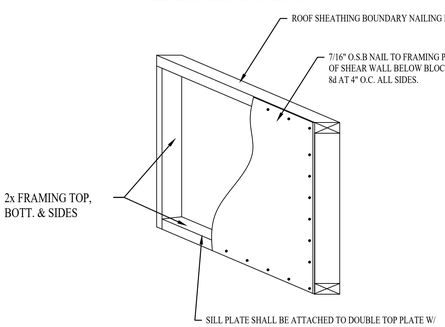
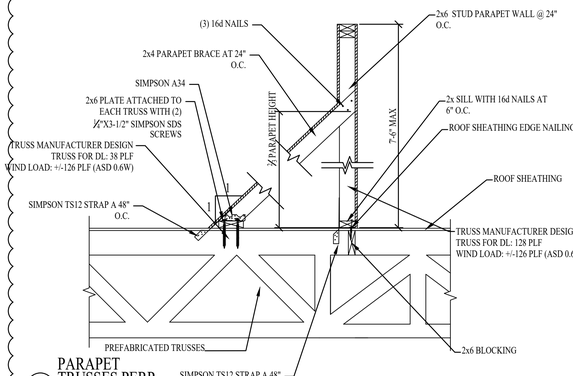


PREFABRICATED TRUSS BLOCK



FIELD BUILT TRUSS BLOCK



3-B S5.03 PARAPET TRUSSES PERP.
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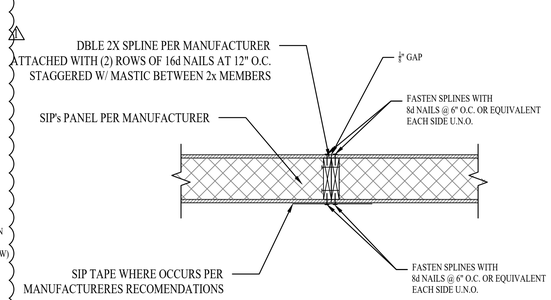

4-B S5.03 SIP's PANEL SPLINE
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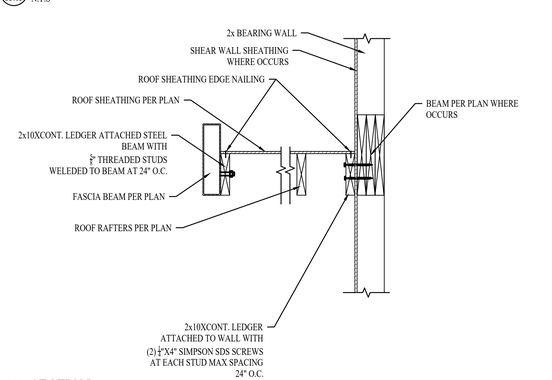

1-A S5.03 SECTION
N.T.S

2-A S5.03 TRUSS BLOCKING
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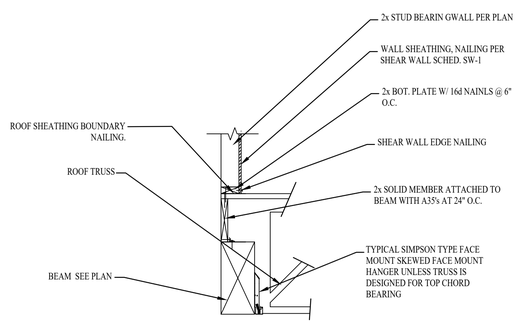

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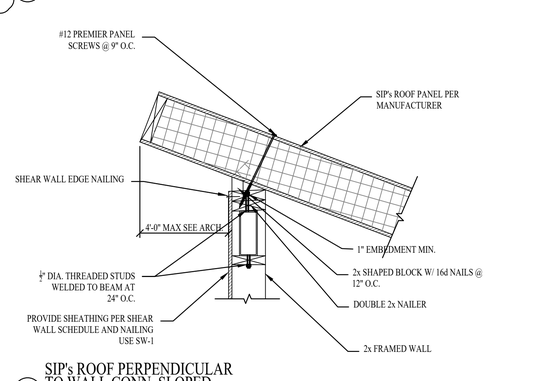

4-A S5.03 SIP's ROOF PERPENDICULAR TO WALL CONN. SLOPED
N.T.S

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SHEATHING SCHEDULE

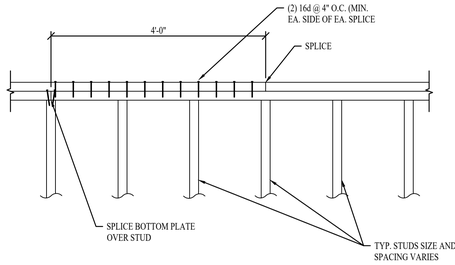
MARK	OSB OR PLYWOOD	COMMON NAILS			16 GA. STAPLE ALT.	LAYERS OF SHEATHING
		SIZE	PERIMETER	FIELD	EDGE SPACING	
SW1	7/16"	8d	6"	12"	3"	1
SW2	7/16"	8d	4"	12"	2"	1
SW3	7/16"	8d	3"	12"	NOT ALLOWED	1
SW4	7/16"	8d	4"	12"	2"	1
SW5	7/16"	8d	3"	12"	NOT ALLOWED	1
SW6	7/16"	8d	2"	12"	NOT ALLOWED	1
ROOF	5/8"	8d	6"	12"	NOT ALLOWED	1

NOTES FOR SHEAR WALLS 3-6:

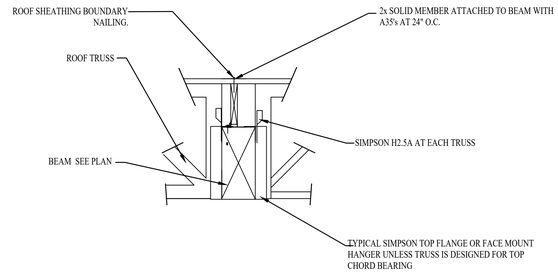
1. FRAMING AT ADJOINING PANEL EDGES SHALL BE 3" NOMINAL OR SHALL BE (2) 2" NOMINAL MEMBERS. STITCH NAIL THE (2) 2X MEMBER WITH A DOUBLE ROW OF 10d COMMON NAILS AT 4" O.C.. AT 3" NOMINAL MEMBER IS NOT REQUIRED FOR SILL PLATE U.N.O.

2. ALL SHEATHED WALLS TO BE TYPE "1" U.N.O.

3. 8" FIELD SPACING FOR STAPLE ALT.



2-D / S5.04 TYP. TOP PL:ATE SPLICE
N.T.S



4-D / S5.04 SECTION
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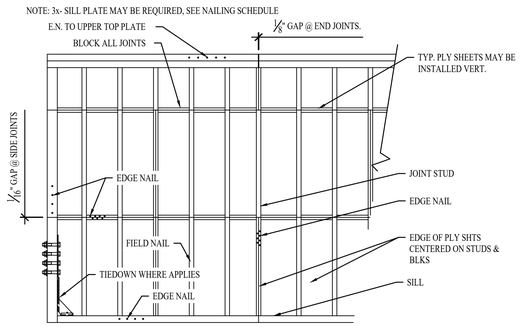
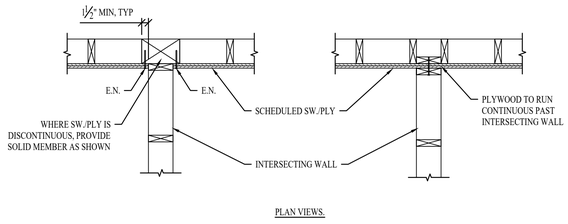
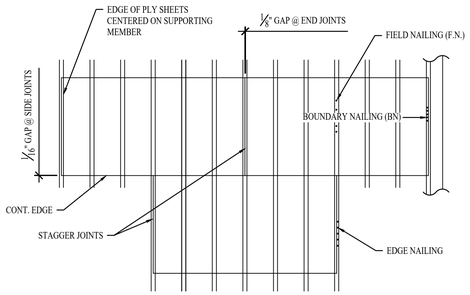


TYPICAL SHEAR WALL



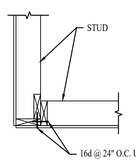
PLAN VIEWS.

2-C / S5.04 SHEAR WALLS WITH INTERSECTING WALLS
N.T.S

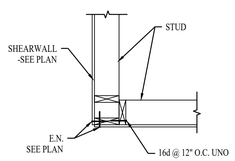


TYPICAL ROOF/ FLOOR SHEATHING

2-B / S5.04 TYP. SHEAR WALL DETAIL & ROOF/FLOOR SHEATHING DETAIL
N.T.S



CORNER- (OPTIONAL)



CORNER

1-A / S5.04 BUILT UP CORNER
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