

# Appendix V - FRPB Final Drawing

## UTAH

### UTAH TRANSIT AUTHORITY

### FRONT RUNNER PAINT BOOTH

### WARM SPRINGS SERVICE CENTER

### SALT LAKE CITY, UTAH

#### UTA PROJECT #SGR-358

#### BID / PERMIT SET - DECEMBER 7, 2020

#### APPROVALS:

_____	Date
_____	Date
_____	Date

APPROVAL DOES NOT RELIEVE A/E OF DESIGN LIABILITY

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_____	Date
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## CODE ANALYSIS

APPLICABLE CODES			
	Year		Year
International Building Code	2018	National Electrical Code	2017
International Mechanical Code	2018	Uniform Code for Building Conservation	N.A.
International Plumbing Code	2018	ADA Accessibility Guidelines	2010
International Fire Code	2018		
International Energy Conservation Code	2018		

A. Occupancy and Group: S1  
Change in Use: Yes No No X Mixed Occupancy: Yes No No X  
Special Use and Occupancy (e.g. High Rise, Covered Mall): IBC SECT. 416; IFC CH. 15

B. Seismic Design Category: D Design Wind Speed: 90 mph

C. Type of Construction (UNCHANGED):  
I I II II III III IV V V  
A A B B A A HT A B

D. Fire Resistance Rating Requirements for the Exterior Walls based on the fire separation distance (in hours):  
North: 0 South: 0 East: 0 West: 0

E. Mixed Occupancies: UNCHANGED Nonseparated Uses: UNCHANGED

F. Sprinklers (Booth Only):  
Required: YES Provided: YES Type of Sprinkler System: WET PIPE/DRY CHEM.

G. Number of Stories: 1 Building Height: UNCHANGED

H. Actual Area per Floor (square feet): APPROX. 130,000 SF - UNCHANGED

I. Tabular Area: UNCHANGED  
Occupancy and/or Area : S1 BOOTH  
Occupancy separation required (Hrs.) 0 1  
Sprinklered: Indicate Yes or No NO YES  
a. Actual Area (ft.): 130,000 +/- 3,000  
UNCHANGED

K. Fire Resistance Rating Requirements for Building Elements (hours).

Element	Hours	Assembly Listing	Element	Hours	Assembly Listing
Exterior Bearing Walls	0	N.A.	Floors - Ceiling Floors	0	N.A.
Interior Bearing Walls	0	N.A.	Roofs - Ceiling Roofs	0	N.A.
Exterior Non-Bearing Walls	0	N.A.	Exterior Doors and Windows	0	N.A.
Structural Frame	0	N.A.	Shaft Enclosures	0	N.A.
Partitions - Permanent	0	N.A.	Fire Walls	0	N.A.
Fire Barriers	0	N.A.	Fire Partitions	1	I.B.C. 302.3.2
			Smoke Partitions	0	N.A.

L. Design Occupant Load: UNCHANGED

M. Minimum Number of Required Plumbing Facilities: NOT REQUIRE PER IBC 2902.4  
a) Water Closets - Required (m) N.A. (f) N.A. Provided (m) N.A. (f) N.A.  
b) Lavatories - Required (m) N.A. (f) N.A. Provided (m) N.A. (f) N.A.  
c) Bath Tubs or Showers: N.A.  
d) Drinking Fountains: N.A. Service Sinks: N.A.

1			2			3			4			5		
△						Designed By: R. CHILDS		UTAH TRANSIT AUTHORITY FRONT RUNNER PAINT BOOTH WARM SPRINGS SERVICE CENTER 900 NORTH 500 WEST SALT LAKE CITY, UT 84116 COVER SHEET			Scale: N/A			
△						Drawn By: R. CHILDS					CADD Filename: FRPB-G000.DWG			
△						Checked By: R. STANISLAW					Submittal Date DEC 7, 2020			
△						Approved By: R. STANISLAW					UTA Project No.: SGR-358			
△											Drawing No.: G000			
0	10/29/20	PERMIT SET												
REV	DATE	Description	7/27/09	Submitted By:		Approved By: _____								



GENERAL NOTES

1. THERE ARE NO ASBESTOS PRODUCTS SPECIFIED AND NONE SHALL BE USED ANYWHERE ON THIS PROJECT. CONTRACTOR SHALL INVESTIGATE THE EXISTENCE OF ANY ASBESTOS ASSOCIATED WITH AREAS OF THE BUILDING TO BE REMODELED. CONTRACTOR IS TO ENSURE ALL ASBESTOS IS REMOVED FROM THESE AREAS.
2. WHENEVER QUESTIONS ARISE OR CONDITIONS ARE ENCOUNTERED WHICH ARE NOT COVERED BY OR ARE IN CONFLICT WITH THE CONTRACT DOCUMENTS, CONSULT WITH THE ARCHITECT PRIOR TO TAKING ANY FURTHER ACTION.
3. CONTRACTOR SHALL PROTECT THE EXISTING BUILDING DURING THE CONSTRUCTION PERIOD AND SHALL REPAIR OR REPLACE ANY AND ALL EXISTING CONSTRUCTION DAMAGED BY NEW CONSTRUCTION. MATCH EXISTING SURFACE FINISH OR MATERIAL.
4. CONTRACTOR SHALL MODIFY OR RELOCATE EXISTING MECHANICAL AND ELECTRICAL SYSTEMS AS INDICATED ON THE DRAWINGS AND SCHEDULE SHUT DOWNS AS REQUIRED FOR INSTALLATION OF NEW WORK WITH THE OWNER.
5. ALL DEMOLISHED OR REMOVED EXISTING MATERIAL SHALL BE LEGALLY DISPOSED OF BY THE CONTRACTOR UNLESS CALLED OUT TO BE DELIVERED TO THE OWNER.
6. CONTRACTOR SHALL LEAVE WORK AREAS BROOM CLEAN AND FREE OF TOOLS, EQUIPMENT, ETC., AT THE END OF EACH SHIFT. ALL CONSTRUCTION ACTIVITY SHALL BE CONTAINED WITHIN CONSTRUCTION BARRICADES OR FENCES. CONTRACTOR SHALL PROTECT OWNERS EXISTING CONSTRUCTION AND EQUIPMENT ADJACENT TO NEW CONSTRUCTION. CONTRACTOR SHALL CLEAN ALL SURFACES TO "LIKE NEW" CONDITION AT THE COMPLETION OF WORK.
7. THE INSTALLATION OF NEW MATERIALS AND EQUIPMENT SHALL BE PERFORMED IN STRICT ACCORDANCE WITH THE MATERIAL AND EQUIPMENT MANUFACTURER'S SPECIFICATIONS AND REQUIREMENTS.
8. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR CLEANING GLASS SURFACES, REMOVING TRASH AND DEBRIS, REMOVING TOOLS AND TEMPORARY CONSTRUCTION MATERIALS PRIOR TO PROJECT COMPLETION.
9. ARCHIPLEX RESPONSIBLE FOR ARCHITECTURAL DRAWINGS ONLY.
10. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR GENERAL CONSTRUCTION PROCEDURES AND JOB SITE SAFETY, AND SHALL ENSURE COMPLIANCE WITH APPLICABLE CODES, RULES AND REGULATIONS OF THE GOVERNING JURISDICTION.
11. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR SECURING BUILDING PERMITS AS REQUIRED FOR WORK AND SHALL BE RESPONSIBLE FOR OBTAINING REQUIRED INSPECTIONS DURING THE COURSE OF THE WORK.

12. THE INFORMATION CONTAINED IN THE DRAWINGS IS BASED ON EXISTING DOCUMENTS AND LIMITED FIELD MEASUREMENT. THE WORK DESCRIBED HEREIN MAY REQUIRE ADJUSTMENT OR MODIFICATION TO CONFORM TO THE EXISTING CONDITIONS. THE GENERAL CONTRACTOR SHALL VERIFY CRITICAL DIMENSIONS AND SHALL BE RESPONSIBLE FOR FIELD MEASURING EXISTING CONDITIONS PRIOR TO STARTING THE WORK AND DURING PROGRESS OF THE WORK.
13. THE GENERAL CONTRACTOR SHALL PROVIDE WRITTEN NOTIFICATION TO THE ARCHITECT PRIOR TO PROCEEDING WITH THE WORK SHOULD DISCREPANCIES BE FOUND IN THE CONTRACT DOCUMENTS. ADDITIONAL WORK OR DEMOLITION REQUIRED, AS A RESULT OF FAILURE TO DO SO SHALL BE AT THE CONTRACTOR'S EXPENSE.
14. THE DRAWINGS SHALL NOT BE SCALED FOR DIMENSIONS AND/OR SIZES. THE GENERAL CONTRACTOR SHALL NOT DEViate FROM DIMENSIONS INDICATED ON THE DRAWINGS WITHOUT PRIOR WRITTEN APPROVAL OF THE ARCHITECT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURACY OF MEASUREMENT AND THE PRECISE FITTING OF THE WORK.
15. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION AND SECURITY OF DELIVERED MATERIALS.
16. THE GENERAL CONTRACTOR SHALL MAINTAIN A COMPLETE AND UP TO DATE SET OF CONTRACT DOCUMENTS AT THE JOB SITE.
17. THE GENERAL CONTRACTOR SHALL ENSURE THAT THE WORK BE PERFORMED BY COMPETENT MECHANICS SKILLED IN THEIR TRADE. WORKMANSHIP DEEMED SUBSTANDARD OR UNACCEPTABLE BY THE ARCHITECT WILL BE REJECTED AND SHALL BE CORRECTED BY THE CONTRACTOR.
18. THE CONTRACTOR SHALL PROVIDE THE ARCHITECT AND OWNER WITH A MARKED-UP SET OF DRAWINGS WITH THE APPLICATION FOR FINAL PAYMENT. THE SET WILL INCLUDE ANY CHANGES MADE IN THE FIELD DURING CONSTRUCTION AND WILL BE USED AS THE BASIS FOR DEVELOPING RECORD DRAWINGS.
19. THE CONTRACT DOCUMENTS, INCLUDING THE DRAWINGS AND ASSOCIATED DOCUMENTATION, ARE THE SOLE PROPERTY OF ARCHIPLEX GROUP. UNAUTHORIZED OR INCORRECT USE OF THESE DOCUMENTS IS PROHIBITED.
20. ALL DETAILS APPLY WHETHER OR NOT SPECIFICALLY REFERENCED.
21. UNLESS OTHERWISE NOTED, DIMENSIONING ON PLANS IS AS FOLLOWS: TO FACE OF ALL WALLS AND/OR STRUCTURAL GRID.

22. STRUCTURAL, HVAC, PLUMBING AND ELECTRICAL EQUIPMENT OR SYSTEMS SHOWN ON ARCHITECTURAL DRAWINGS ARE INDICATED FOR GENERAL REFERENCE ONLY. COORDINATE AND LOCATE WITH STRUCTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
23. UNLESS SPECIFICALLY NOTED AS "NOT IN CONTRACT", SYSTEMS SHOWN IN DWG's. ARE INTENDED TO BE CODE COMPLIANT, FURNISHED, INSTALLED & TURNED OVER TO OWNER IN PROPER FUNCTIONING CODE COMPLIANT CONDITION. ALL WORK REQUIRED TO ACCOMPLISH THIS MUST BE CONSIDERED IN BASE BID (CONTRACT SUM).
24. ALL WORK TO BE CONSTRUCTED PER GOVERNING CODES WHICH ARE HEREIN INCORPORATED INTO THESE DOCUMENTS. CODE REQUIRED WORK, INCLUDING WORK OR ITEMS NOT SHOWN IN THE CONSTRUCTION DOCUMENTS ARE TO BE INCLUDED IN BASE BID (CONTRACT SUM).
25. ADEQUACY OF FIRE PROTECTION AND SAFETY DURING CONSTRUCTION SHALL BE THE GENERAL CONTRACTOR'S RESPONSIBILITY. NFPA STANDARDS NO. 10 AND 241 ARE RECOMMENDED AS WELL AS REQUIREMENTS OF INSURANCE CARRIER & FIRE MARSHAL.
26. PROVIDE BLOCKING IN WALLS AS REQUIRED IN ORDER TO SECURE ALL EQUIPMENT (ACCESSORIES, HANDRAILS, CASEWORK, ETC.) PROVIDE BLOCKING AS REQUIRED FOR WALL OR CEILING SUPPORTED ITEMS. VERIFY REQUIREMENTS PER MANUFACTURER'S RECOMMENDATIONS WHERE APPLICABLE.

INDEX OF DRAWINGS

GENERAL

G000	COVER SHEET
G001	GENERAL NOTES, LEGEND, ABBREVIATIONS & SHEET INDEX

ARCHITECTURAL

AE101	FLOOR PLAN, DEMOLITION PLAN AND PARTIAL SITE PLAN
AE201	ELEVATIONS AND SECTIONS
AE401	PAINT KITCHEN ENLARGED FLOOR PLAN, ELEVATIONS AND DETAILS
AE501	PAINT KITCHEN SECTIONS AND DETAILS

STRUCTURAL

S001	GENERAL STRUCTURAL NOTES
S002	SPECIAL INSPECTIONS
S101	FOOTING & FOUNDATION / FRAMING PLANS
S501	DETAILS
S502	DETAILS
S601	SCHEDULES
S602	SCHEDULES

FIRE PROTECTION

F100	FIRE SPRINKLER PLAN - PAINT BOOTH
F101	FIRE SPRINKLERS SITE PLAN

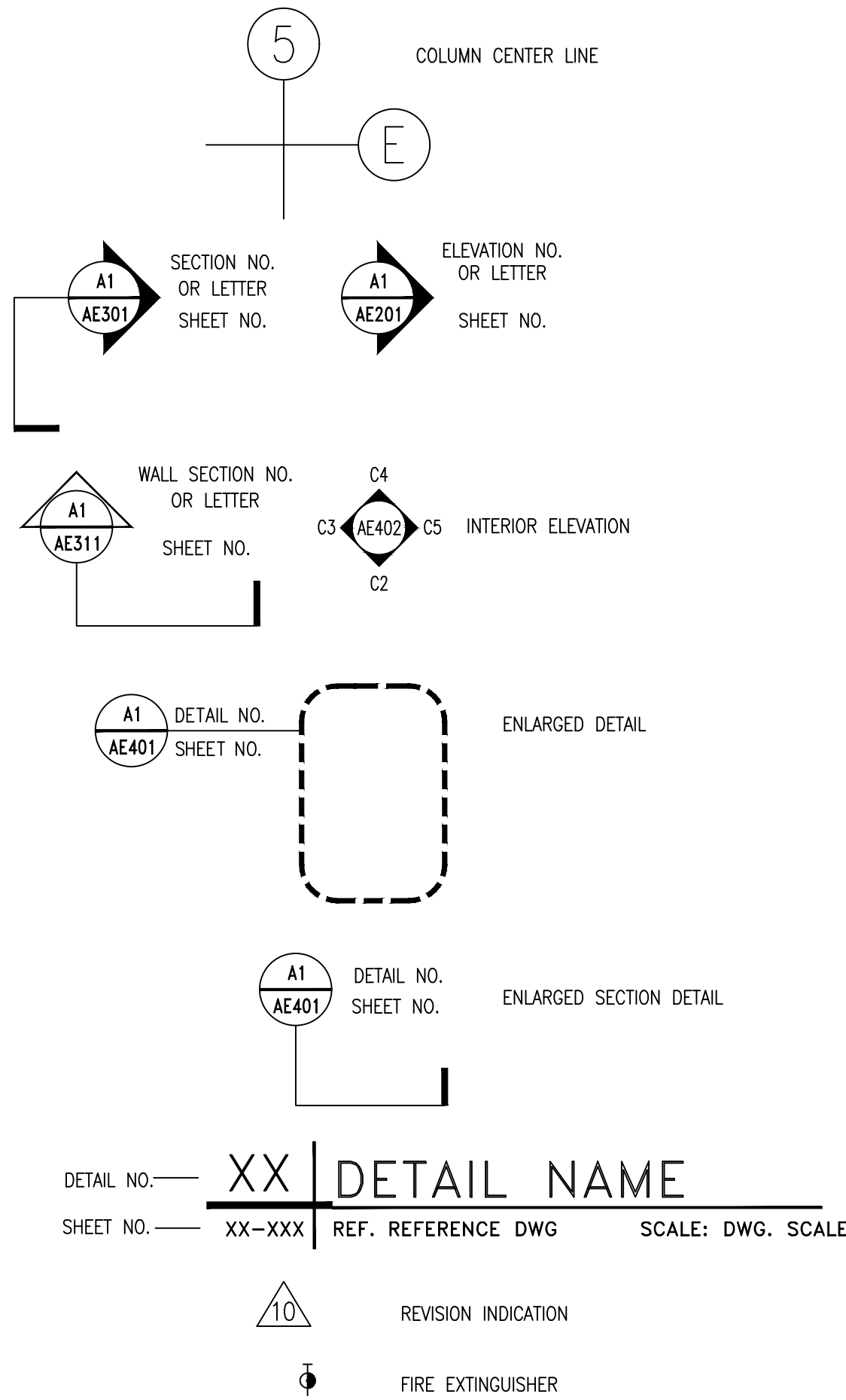
MECHANICAL

M0001	MECHANICAL LEGEND AND GENERAL NOTES
ME101	MECHANICAL FLOOR PLANS
ME102	MECHANICAL NATURAL GAS PLANS
ME501	MECHANICAL SCHEDULES AND DETAILS

ELECTRICAL

EE001	GENERAL NOTES, SYMBOLS, LIGHT FIXTURE SCHEDULE
EE002	ELECTRICAL DETAILS
EE003	POWER SINGLE LINE DIAGRAM, PANEL AND MECHANICAL SCHEDULES
ED101	DEMOLITION FLOOR PLAN - ELECTRICAL
EE101	NEW FLOOR PLAN - POWER
EE102	NEW FLOOR PLAN - LIGHTING

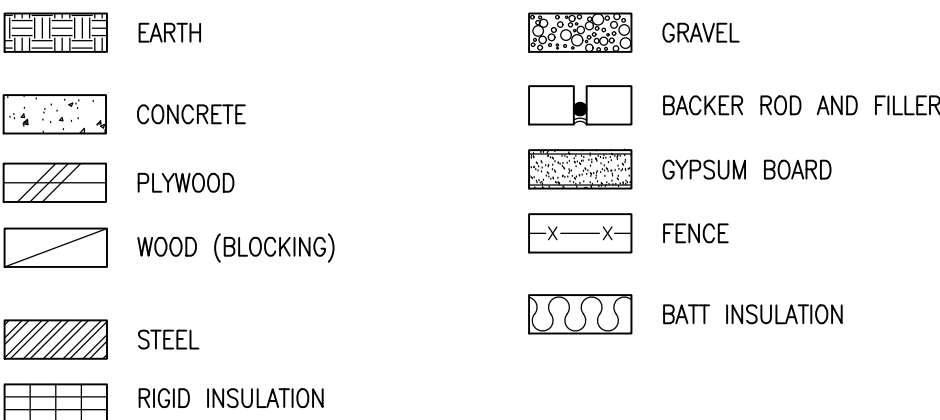
ARCHITECTURAL SYMBOLS LEGEND



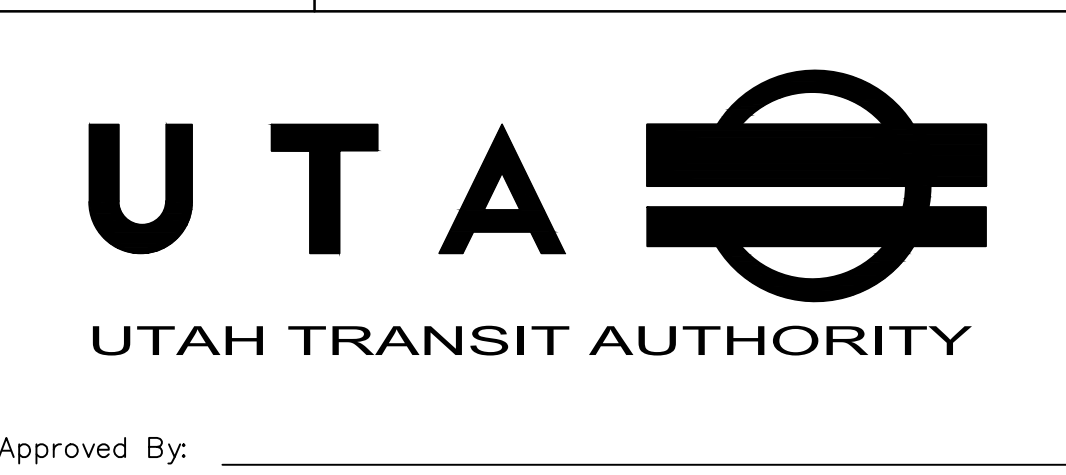
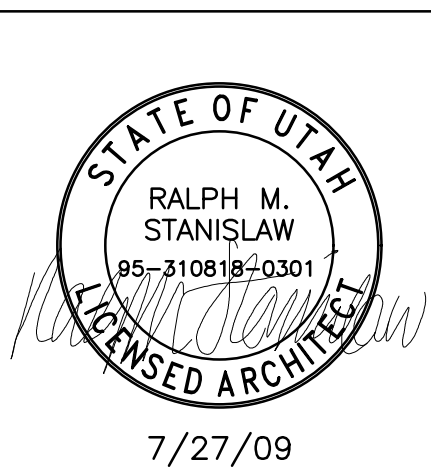
ABBREVIATIONS

ALT.	ALTERNATE	ELEV./EL.	ELEVATION	MECH.	MECHANICAL	SCHED.	SCHEDULE
ALUM.	ALUMINUM	EQ.	EQUAL	MEMB.	MEMBRANE	SEAL.	SEALANT
A.B.	ANCHOR BOLT	EQUIP.	EQUIPMENT	MEN	MEN'S TOILET	SECT.	SECTION
&	AND	EXP.	EXPANSION	MTL./MET.	METAL	S.SK.	SERVICE SINK
ARCH.	ARCHITECTURAL	EXT.	EXTERIOR	MIN.	MINIMUM	SHT.	SHEET
@	AT OR AT THE	EW	EACH WAY	MIR.	MIRROR	SIM.	SIMILAR
BM.	BEAM	FIN.	FINISH	MISC.	MISCELLANEOUS	SL./SLP.	SLOPE
BLK.	BLOCK	F.A.	FIRE ALARM	MTD.	MOUNTED	S.C.	SOLID CORE
BLKG.	BLOCKING	F.E.	FIRE EXTINGUISHER	MUL.	MULLION	SPEC.	SPECIFICATIONS
BD.	BOARD	F.E.C.	F.E. CABINET	NOM.	NOMINAL	SQ.	SQUARE
B.O.	BOTTOM OF	FLR./FL.	FLOOR	N.	NORTH	STD.	STANDARD
BOT.	BOTTOM	F.D.	FLOOR DRAIN	N.I.C.	NOT IN CONTRACT	STL.	STEEL
BLDG.	BUILDING	FTG.	FOOTING	N.I.S.	NOT TO SCALE	STOR.	STORAGE
CLKG.	CAULKING	FDN.	FOUNDATION	NO. OR #	NUMBER	STRUCT.	STRUCTURAL/STRUCTURE
C.I.	CAST IRON	FV.	FIELD VERIFY			SYM.	SYMMETRICAL
CLG.	CEILING					S.STL.	STAINLESS STEEL
CEM.	CEMENT	GALV.	GALVANIZED	OFOI	OWNER FURNISH, OWNER INSTALL	TEL.	TELEPHONE
CTR.	CENTER	G.I.	GALVANIZED IRON	OFCI	OWNER FURNISH, CONTRACTOR INSTALL	TEMP.	TEMPORARY/TEMPERED
	CENTER LINE	GL.	GLASS/GLAZING	OFF.	OFFICE	THK.	THICK (NESS)
CER.	CERAMIC	GR.	GRADE	O.C.	ON CENTER	T & G	TONGUE AND GROOVE
C.T.	CERAMIC TILE	GND.	GROUND	OPNG.	OPENING	T/CONC.	TOP OF CONCRETE
CFCI	CONTRACTOR FURNISH, CONTRACTOR INSTALL	GYP.	GYPSUM	OPP.	OPPOSITE	T/CURB	TOP OF CURB
CFOI	CONTRACTOR FURNISH, OWNER INSTALLED	GYP. BD.	GYPSUM BOARD	OPP. H.	OPPOSITE HAND	T.O.P.	TOP OF PLATE
	CLEAR/CLEARANCE			O.D.	OUTSIDE DIAMETER	T/WALL	TOP OF WALL
CLR.	CLOSET	HDWR.	HARDWARE			T.	TREAD
CLO.	CLOSET	HDWD.	HARDWOOD	PTD.	PAINTED	TYP.	TYPICAL
COL.	COLUMN	HT.	HEIGHT	PR.	PAIR	T.O.	TOP OF
CONC.	CONCRETE	H.P.	HIGH POINT	PART.	PARTITION	UNF.	UNFINISHED
CMU	CONCRETE MASONRY UNIT	HORIZ.	HORIZONTAL	PED.	PEDESTRIAN	U.N.O.	UNLESS NOTED OTHERWISE
CONN.	CONNECTION	H.B.	HOSE BIBB	PLAS.	PLASTER		
CONSTR.	CONSTRUCTION	HM	HOLLOW METAL	P. LAM.	PLASTIC LAMINATE	VAR.	VARY OR VARIES
CONTR.	CONTRACTOR	HR.	HOURS (FIRE RATING)	PL	PLATE	VERT.	VERTICAL
C.J.	CONTROL JOINT	I.D.	INSIDE DIAMETER	PM	PRESSED METAL	V.T.R.	VENT THROUGH ROOF
CORR.	CORRIDOR	INSUL.	INSULATION	PLYWD.	PLYWOOD	V.I.F.	VERIFY IN FIELD
CNTR.	COUNTER	INT.	INTERIOR	PLUMB.	PLUMBING		
CTSK.	COUNTERSUNK	JAN.	JANITOR	PT.	POINT	W/	WITH
		JT.	JOINT	Q.T.	QUARRY TILE	WD.	WOOD
		J-BOX	JUNCTION BOX	RAD.	RADIUS	WP.	WATERPROOF
		KIT.	KITCHEN	R.W.L.	RAIN WATER LEADER	WSCT.	WAINSCOT
		LAM.	LAMINATE	RE.	REFER TO	W/O	WITHOUT
		LAV.	LAVATORY	REFL.	REFLECTED	W.P.	WORKING POINT
		LT.	LIGHT	REINF.	REINFORCING	W.R.	WATER RESISTANT
		L.P.	LOW POINT	REQ.	REQUIRED		
		MAINT.	MAINTENANCE	REV.	REVISED		
		MFR.	MANUFACTURER	R.	RISER		
		M.O.	MASONRY OPENING	R.D.	ROOF DRAIN		
		MAX.	MAXIMUM	RM.	ROOM		
				R.O.	ROUGH OPENING		

GRAPHIC SYMBOLS



△		
△		
△		
△		
△		
0	10/29/20	PERMIT SET
REV	DATE	Description



Designed By:	R. CHILDS
Drawn By:	R. CHILDS
Checked By:	R. STANISLAW
Approved By:	R. STANISLAW

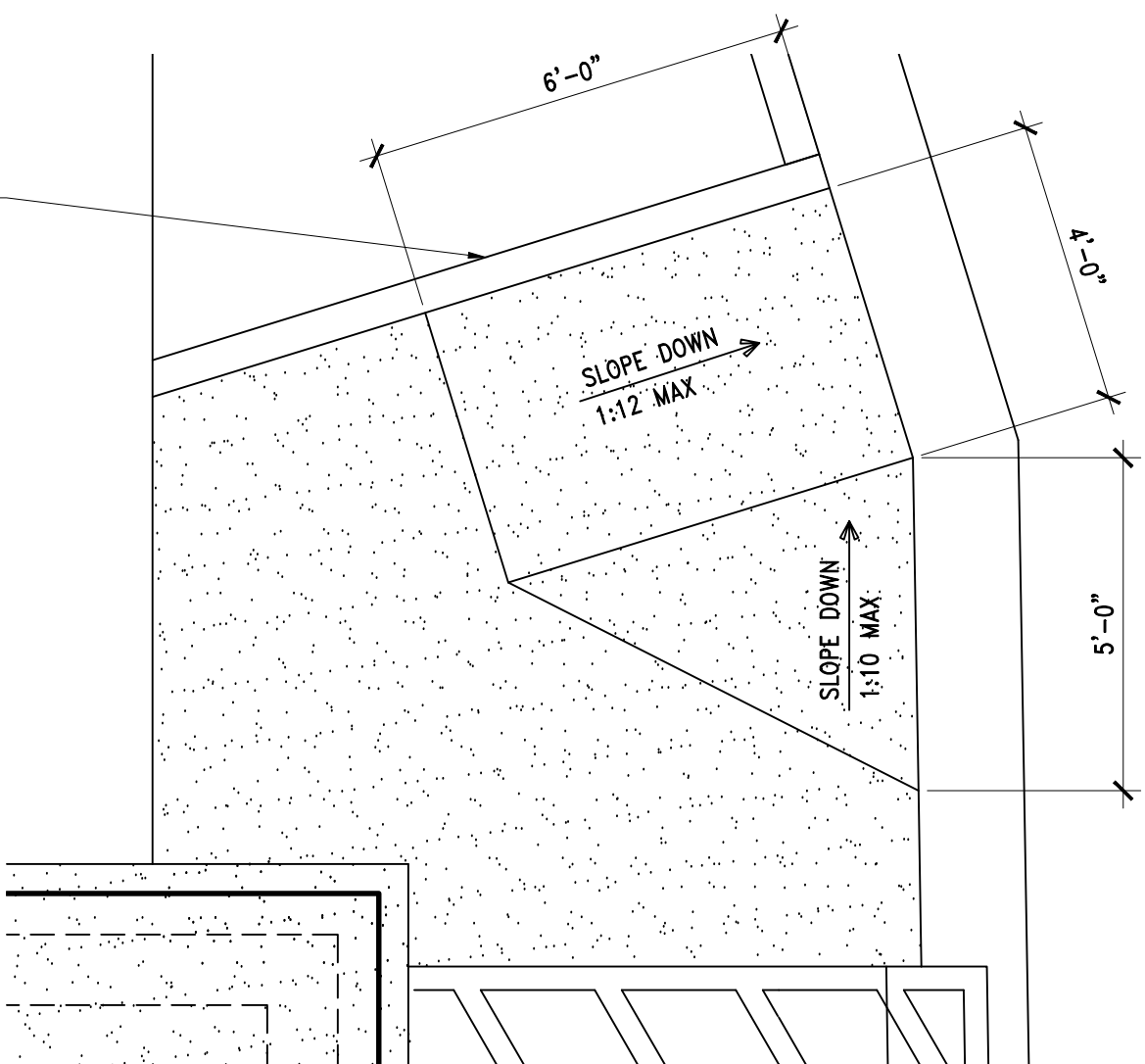
UTAH TRANSIT AUTHORITY FRONT RUNNER PAINT BOOTH WARM SPRINGS SERVICE CENTER 900 NORTH 500 WEST SALT LAKE CITY, UT 84116 GENERAL NOTES AND INDEX OF DRAWINGS		Scale: N / A CADD Filename: FRPB-G001.DWG Submittal Date DEC 7, 2020 UTA Project No.: SGR-358 Drawing No.: Sheet No.: G001
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# GENERAL NOTES

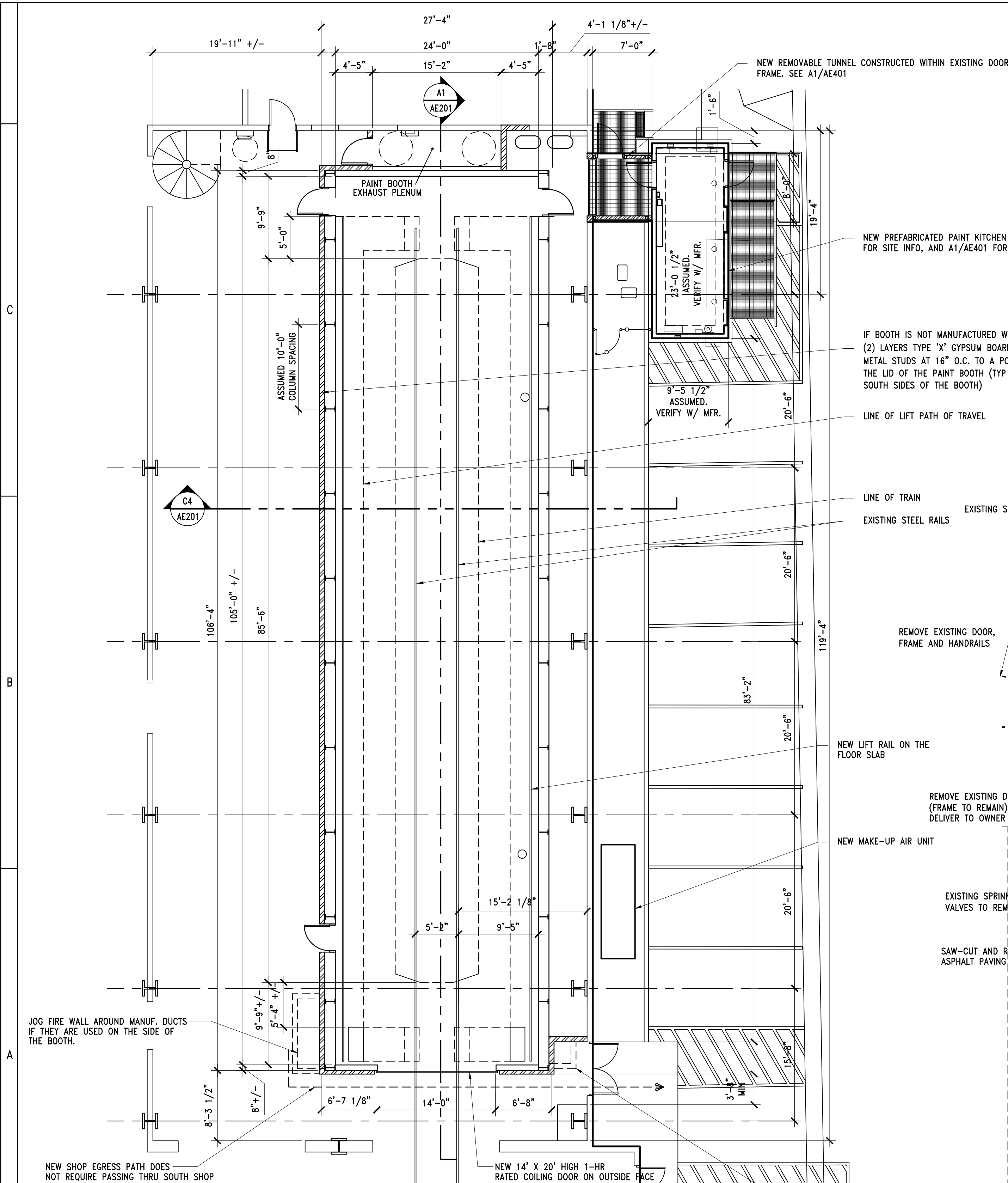
- DIMENSIONS OF PAINT BOOTH AND MIXING BOOTH (PAINT KITCHEN) ARE FOR DESCRIPTIVE PURPOSES ONLY AND ARE ASSUMED BASED ON POSSIBLE MANUFACTURER'S REQUIREMENTS. ACTUAL SELECTED MANUFACTURER'S REQUIREMENTS MAY VARY. INTERIOR CLEARANCES SHOWN FOR OWNER'S EQUIPMENT AND PERSONNEL SHALL BE CONSIDERED MINIMUM PERFORMANCE REQUIREMENTS, AND SHALL BE MET BY THE SELECTED MANUFACTURER. OTHER DIMENSIONS SHOWN AS MINIMUM OR MAXIMUM MUST ALSO BE MET.
- DETAILS OF STRUCTURAL ELEMENTS, MECHANICAL, ELECTRICAL AND OTHER UTILITY CONNECTIONS ARE ESTIMATED LOCATIONS AND DETAIL INFORMATION AND MAY DIFFER FROM MANUFACTURER REQUIREMENTS. FINAL LOCATIONS AND CONNECTION DETAILS SHALL BE BASED ON THE MANUFACTURER'S REQUIREMENTS.

NEW 6" CONCRETE CURB



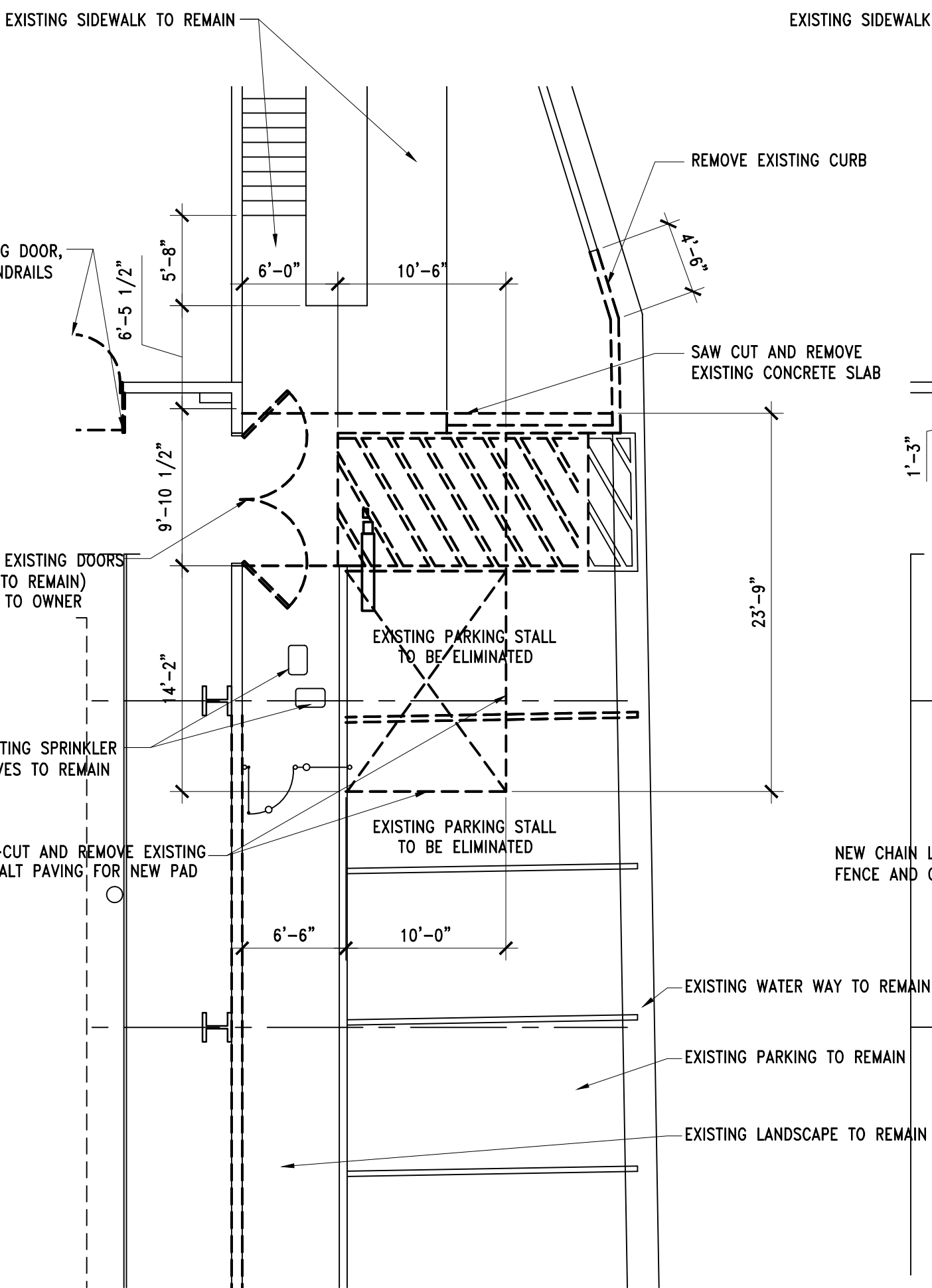
D4 ADA CURB CUT RAMP DETAIL

AE101 REF. SCALE: 3/8" = 1'-0"  
2'-3 3/4" +/-



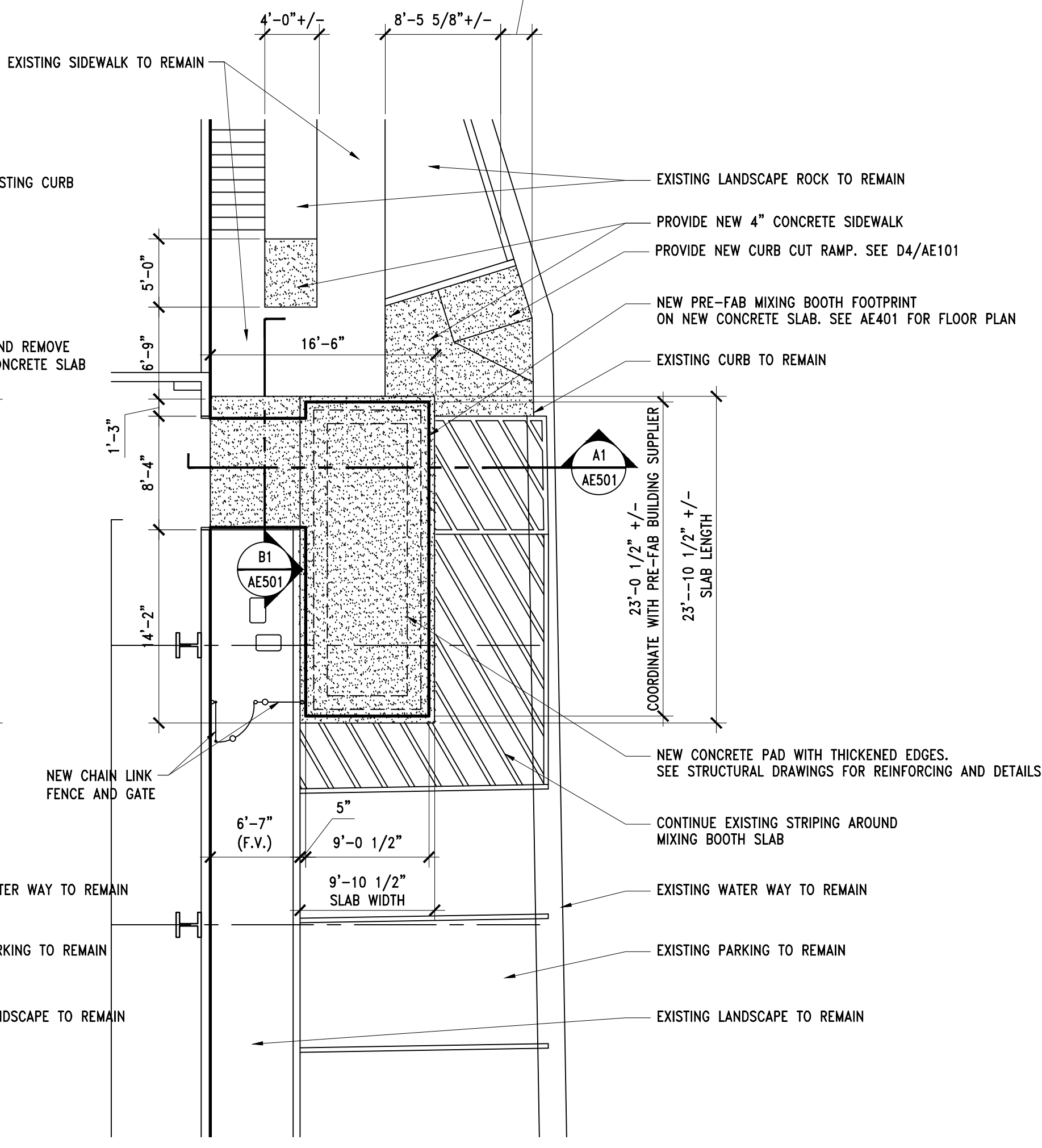
A1 FLOOR PLAN

AE101 REF. SCALE: 1/8" = 1'-0"



A3 DEMOLITION FLOOR PLAN

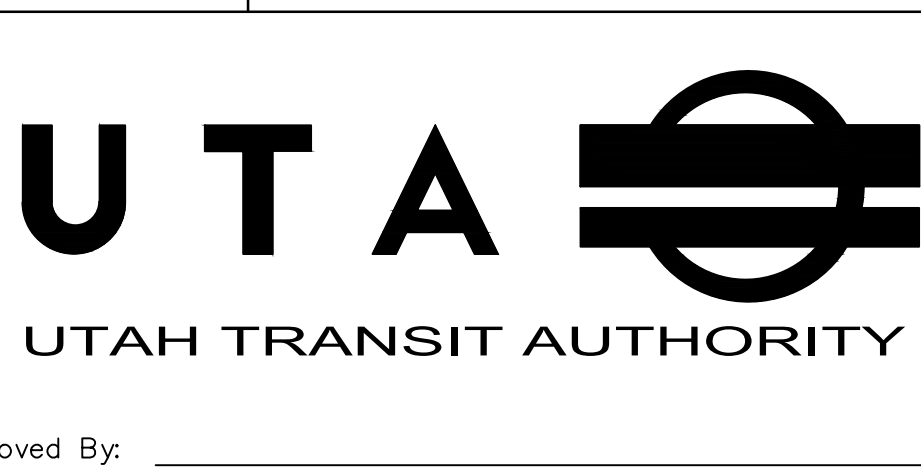
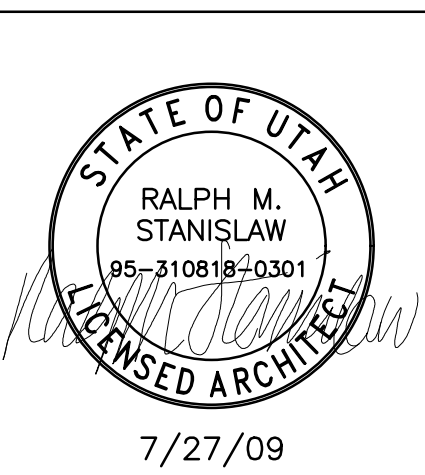
AE101 REF. SCALE: 1/8" = 1'-0"



A4 PARTIAL SITE PLAN

AE101 REF. SCALE: 1/8" = 1'-0"

REV	DATE	DESCRIPTION
0	10/29/20	PERMIT SET



Designed By:	R. CHILDS
Drawn By:	R. CHILDS
Checked By:	R. STANISLAW
Approved By:	R. STANISLAW

UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
FLOOR PLAN, DEMO PLAN AND PARTIAL SITE PLAN

Scale:	1/8"=1'-0"
CADD Filename:	FRPB-AE101.DWG
Submittal Date	DEC 7, 2020
UTA Project No.:	SGR-358
Drawing No.:	AE101



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AE201 REF. SCALE: 1/8" = 1'-0"




























AE201 REF. SCALE: 1/8" = 1'-0"

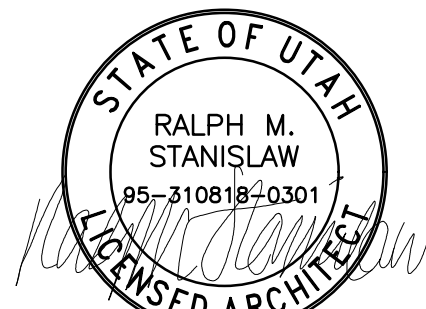


AE201 REF. SCALE: 1/8" = 1'-0"



AE201 REF. SCALE: 1/8" = 1'-0"



7/27/09



255 Crossroad Square  
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P:(801) 961-7070  
F:(801) 961-7373

Submitted By: Rafaela Simoes



Approved By: \_\_\_\_\_

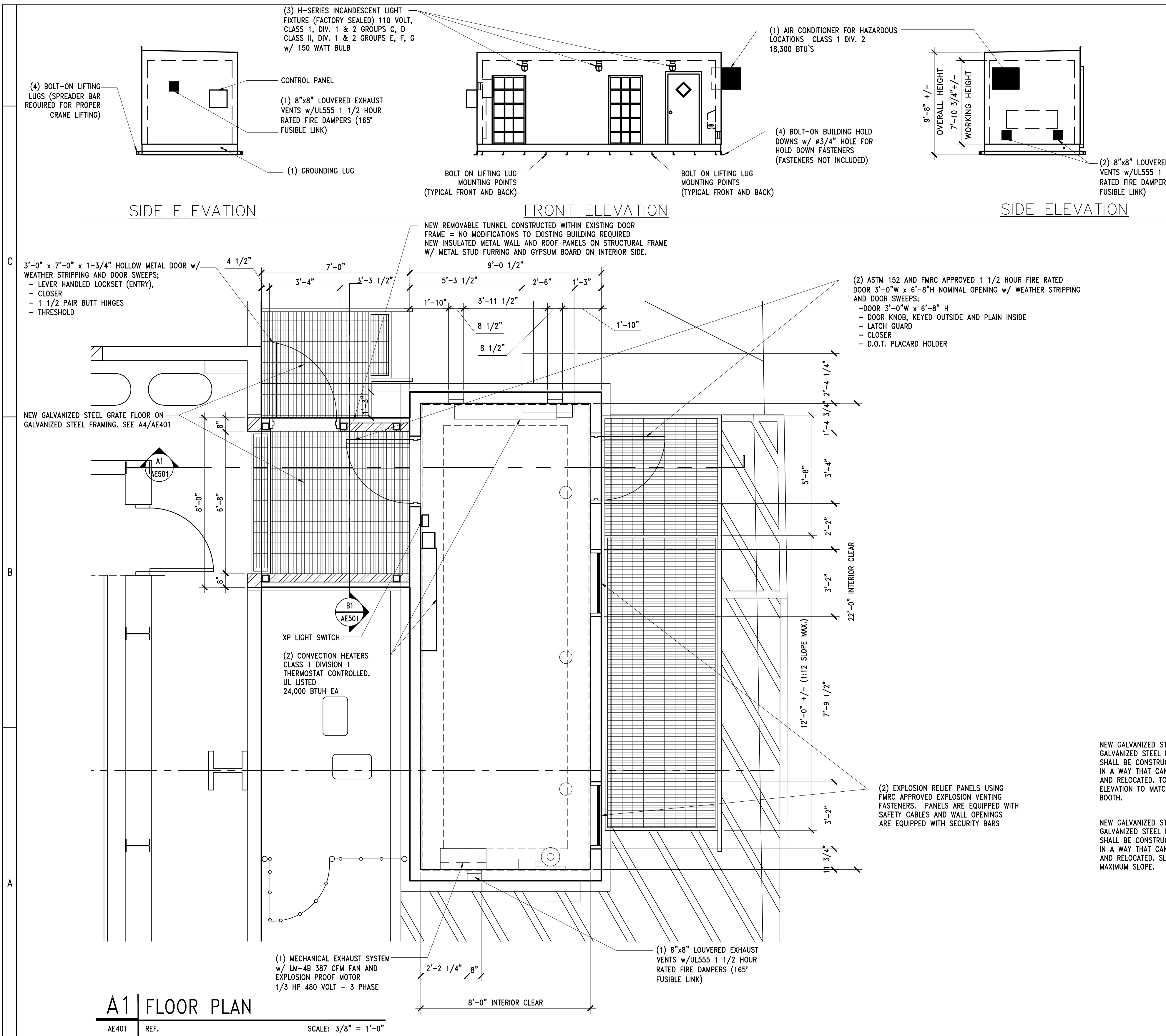
Designed By: R. CHILDS
Drawn By: R. CHILDS
Checked By: R. STANISLAW
Approved By: R. STANISLAW

UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER

## ELEVATIONS AND SECTIONS

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CADD Filename:		
FRPB-AE201.DWG		
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JTA Project No.:		
SGR-358		
Drawing No.:	Sheet No.:	
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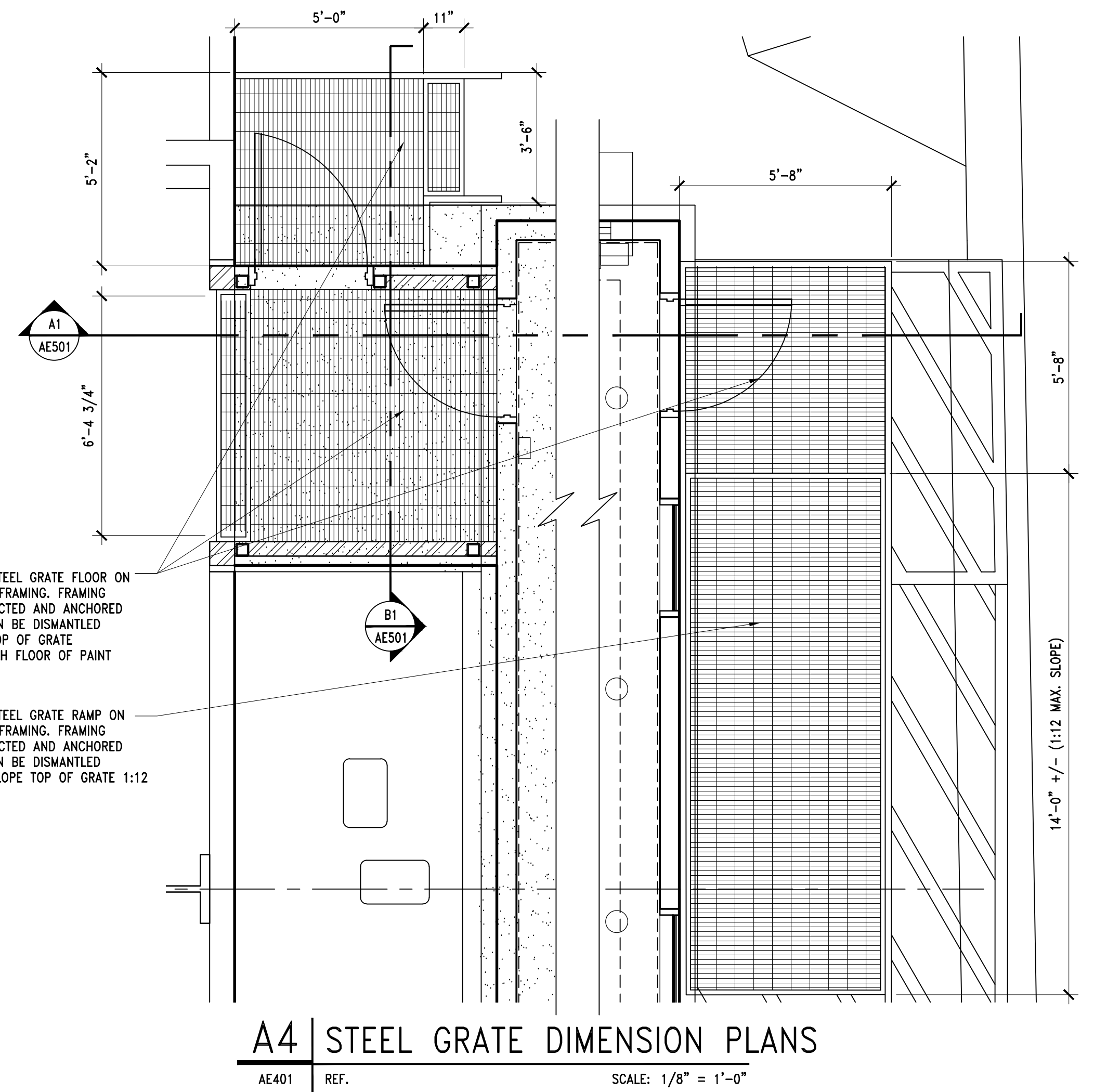




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2. DETAILS OF STRUCTURAL ELEMENTS, MECHANICAL, ELECTRICAL AND OTHER UTILITY CONNECTIONS ARE ESTIMATED LOCATIONS AND DETAIL INFORMATION AND MAY DIFFER FROM MANUFACTURER REQUIREMENTS. FINAL LOCATIONS AND CONNECTION DETAILS SHALL BE BASED ON THE MANUFACTURER'S REQUIREMENTS.



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## GENERAL STRUCTURAL NOTES

## GENERAL

3. The structural notes are intended to complement the project specifications. Specific notes and details in the drawings shall govern over the structural notes and typical details.
2. Typical details and sections shall apply where specific details are not shown.
3. The structural drawings are not all-inclusive and do not contain all dimensions, elevations, openings, mechanical shafts and penetrations needed to build the structure. The contractor shall coordinate these items with the Architectural, Mechanical and Electrical drawings.
4. Omissions or conflicts between the contract drawings and/or specifications shall be brought to the attention of the architect/engineer before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the architect/engineer at no additional cost to the owner.
5. The contractor shall submit a written request to the architect/engineer before proceeding with any changes, substitutions or modifications. Any work done by the contractor before receiving written approval will be at the contractor's risk.
6. The contractor shall coordinate with all trades any items that are to be integrated into the structural system such as openings, penetrations, mechanical and electrical equipment, etc. Sizes and locations of mechanical and other equipment that differs from those shown on the contract drawings shall be reported to the architect/engineer.
7. The contractor shall provide adequate shoring and bracing as required for the chosen method of erection. Shoring and bracing shall remain in place until final connections for the permanent members are completed. The building shall not be considered stable until all connections are completed. Walls shall not be considered self-supporting and shall be braced until the roof system is completed.
8. Site observations by BHB Consulting Engineers' field representative shall not be construed as approval of construction procedures nor special inspection.
9. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings. The structural drawings shall be used in conjunction with the architectural and other consultant's drawings. Some dimensions and elements such as elevations, depressions, slopes, mechanical housekeeping pads, etc. are not shown in the structural drawings. All dimensions shown on structural drawings shall be verified by contractor with architectural, mechanical and electrical drawings.
10. Contractor shall review shop drawings for compliance with contract documents, and stamp shop drawings with review stamp prior to submission to architect for review. Review of shop drawings by BHB Consulting Engineers is for general compliance only and is not intended for approval. The shop drawing review shall not relieve the contractor from the responsibility of completing the project according to the contract documents. Fabrication shall not begin until shop drawings review process is complete. Shop drawings made from reproductions of the contract drawings will be rejected unless the contractor signs a release agreement prior to the shop drawings being reviewed.
11. Only an authorized representative of BHB Consulting Engineers may make changes to these contract drawings. BHB Consulting Engineers shall not be held responsible or liable for any claims arising directly or indirectly from changes made without written authorization by an authorized representative of BHB Consulting Engineers.

## BASIS OF DESIGN

- |  |  |
|--|--|
| 1. Governing Code                                  | International Building Code 2018   |
| a. Risk Category                                   | II   |
| 2. Snow Loads                                      |  |
| a. Ground Snow Load                                | $P_g = 28 \text{ psf}$   |
| b. Snow Importance Factor                          | $I_s = 1.0$  |
| c. Snow Exposure Coefficient                       | $C_{se} = 1.0$   |
| d. Thermal Exposure Coefficient                    | $C_t = 1.0$  |
| e. Roof Snow Load                                  | $P_f = 0.7 \cdot C_{se} \cdot C_t \cdot I_s \cdot P_g = 20 \text{ psf plus Snow Drift}$    |
| 3. Rain Loads                                      |  |
| a. Rain Intensity                                  | $i = 1.5 \text{ in/hr}$  |
| 4. Roof Live Load                                  | 20 psf   |
| 5. Seismic Loads                                   |  |
| a. Seismic Importance Factor, $I_e$                | 1.0  |
| b. Seismic Design Category                         | D  |
| c. Site Specific Ground Motion Hazard Analysis     | Not Required per exceptions in section 11.4.8 of ASCE 7                                    |
| d. Mapped Spectral Acceleration                    | $S_s = 1.498g$<br>$S_1 = 0.550g$   |
| e. Soil Site Class                                 | D  |
| f. Soil Site Coefficients                          | $F_a = 1.2$<br>$F_v = 1.7$   |
| g. 5% Damped Design Spectral Response Acceleration |  |
|  | $S_{DS} = 2/3 \cdot F_a \cdot S_s = 1.198g$<br>$S_{D1} = 2/3 \cdot F_v \cdot S_1 = 0.642g$ |
| h. Seismic-Force-Resisting System                  | Steel Ordinary Moment Frame  |
| i. Response Modification Coefficient               | $R = 3.5$  |
| j. System Over-strength Factor                     | $\Omega_s = 3.0$   |
| k. Deflection Amplification Factor                 | $C_d = 3.0$  |
| l. Redundancy Factors                              | $p_x = 1.3; p_y = 1.3$   |
| m. Fundamental Building Period                     | $T = 0.189 \text{ seconds}$  |
| n. Seismic Response Coefficient                    | $C_s = S_{DS} \cdot I_e / R$<br>$C_{s1} = S_{D1} \cdot I_e / (R \cdot T)$                  |
| o. W   | Dead Loads of Structure  |
| p. Base Shear                                      | $V_x = C_s \cdot W = 0.342 \cdot W$<br>$V_y = C_s \cdot W = 0.342 \cdot W$                 |
| q. Analysis Procedure                              | Equivalent Lateral Force (Static)  |
| 6. Wind Loads                                      |  |
| a. Basic Wind Velocity (3 Second Gust)             | 103 mph  |
| b. Exposure Type                                   | C  |
| c. Internal Pressure Coefficient, GCpi             | +/-0.18  |
| d. Topographic Factor, Kzt                         | 1.0  |
| e. Ground Elevation Factor, Ke                     | 1.0  |

## EXISTING CONDITIONS

1. Structural connections and the framing systems shown in the structural drawings are based on a limited site survey. The contractor shall verify the existing conditions. If existing conditions vary from the information in the contract documents, the contractor shall notify the architect/engineer prior to proceeding with the fabrication or construction of any affected elements.
2. Existing framing systems and foundations taking new loads are assumed to be in good condition, unless noted otherwise in the contract documents. The contractor shall immediately notify the architect/engineer of any deficiencies in the existing structure that are observed or revealed during construction (e.g. corrosion of steel members, cracking or crumbling of concrete, checking or splitting of wood members) prior to proceeding with the fabrication or construction of any affected elements.
3. The contractor shall use the foundation systems indicated on the plans for reference only, and shall field verify foundation sizes, locations, and thicknesses during construction. The contractor shall notify the architect/engineer if existing foundations vary from the information in the contract documents prior to proceeding with the fabrication or construction of any affected elements.
4. While performing work adjacent to existing structures, the contractor shall be responsible for adequate shoring and protection of all existing structures, utilities, and services which will be affected by the work in the contract documents.

## FOUNDATION

- |  |  |
|--|--|
| 1. Soils Investigation Report:   | None   |
| 2. Assumed Soil bearing pressure:  | 1500 psf - Contractor shall verify at time of construction |
| 3. Frost Protection:   | 30" minimum.   |
| 4. Clear excavations of debris and loose soil prior to placing footings. All footings shall bear on undisturbed natural sub-grade or engineered compacted fill as noted in these drawings. |  |

## EARTHWORK

1. Prior to construction, the contractor shall verify that the soil conditions are adequate for 1,500 psf allowable soil bearing pressure. If needed, structural fill shall be provided beneath footings.
2. Clearing. Remove all existing structures and associated foundations, slabs, fencing, asphalt, concrete, and incidental structures as necessary for project completion. The building area shall be stripped of all vegetation, topsoil and debris. Following stripping, all fill soils and any remaining loose natural soils shall be excavated to expose compact natural soils.
3. Proof roll the entire building pad area with normal compaction equipment to check for the presence of unsuitable fills, soft spots, or other undesirable materials or conditions. Remove sub-grade materials that are unsuitable and replace with compacted structural fill or 2,000 psi lean concrete.
4. Compacted structural fill: All fill material shall be a well-graded granular material with a maximum size less than 3" and with not more than 15 percent passing a No. 200 sieve. It shall be compacted to at least 95 percent of the maximum laboratory density as determined by ASTM D 1557 for fill beneath footings and 90 percent for fill beneath floor slabs. All fill shall be tested. Compacted structural fill shall be placed in lifts not exceeding 8" in uncompacted thickness.
5. Floor slabs thickness shall be required by the plans and underlain by a granular layer at least 4" thick. The granular layer shall have a maximum size less than 1" with not more than 5 percent passing a #200 sieve and shall be compacted to at least 90 percent of the maximum laboratory density as determined by ASTM D 1557.
6. Consult the project specifications for further earthwork requirements.

## CONCRETE

- Materials, unless noted otherwise:
- a. Normal weight aggregates
    - i. Combined aggregate gradation for slabs on grade and other designated concrete shall be 8% - 18% for large top size aggregates (1 1/2" or 8% - 22% for smaller top size aggregates (1" or 3/4") retained on each sieve below the top size and above the No. 100. The range for the No. 30 and No. 50 sieves shall be 8% - 15% retained in each. To avoid gap gradation the following shall occur:
      - 1. The percent retained on two adjacent sieves shall not fall below 5%.
      - 2. The percent retained on three adjacent sieves shall not fall below 8%.
      - 3. When the percent retained on two adjacent sieves is less than 8%, the total retained on either of these sieves and the adjacent outside sieve shall be at least 13%. See ACI 302 Section 5.4.3.3 for more information.
    - ii. Maximum Aggregate Size shall not be larger than:
      - 1. 3 1/2" or 1/5 the narrowest dimension of the forms
      - 2. 1/3 the depth of the slab
      - 3. 3/4 the minimum clear spacing between bars
  - b. Reinforcing Steel
    - i. ASTM 615 Grade 60 (Fy = 60 ksi)  
Use Grade 40 (Fy = 40 ksi) for field bent dowels with spacings indicated reduced by 1/3.
  - c. Anchor Rods
    - i. Typical, uno
      - ASTM F1554, Grade 36, with ASTM A563 heavy hex nuts and hardened washers Grade A
    - ii. Moment Frame Columns
      - ASTM F1554, Grade 55 (or 105) with ASTM A563 heavy hex nuts and hardened washers Grade A
  - d. Admixtures:
    - i. Air-entraining admixtures shall comply with ASTM C 260 (when used).
    - ii. Calcium chloride shall not be added to the concrete mix.
    - iii. Water-reducing admixture shall comply with ASTM C 494/C 494M, Type A (when used)
    - iv. Retarding admixture shall comply with ASTM C 494/C 494M, Type B (when used).
    - v. Water-reducing and retarding admixture shall comply with ASTM C 494/C 494M, Type D (when used).
    - vi. High-range, water-reducing admixture shall comply with ASTM C 494/C 494M, Type F (when used).
    - vii. High-range, water-reducing and retarding admixture shall comply with ASTM C 494/C 494M Type G (when used).
  - viii. Admixture manufacturer shall have ISO 9001 Quality Certification. To ensure compatibility all admixtures shall be from the same manufacturer.
  - e. Type I/II cement complying with ASTM C-150 shall be used for all concrete. Cement source shall remain the same for the entire job.
  - f. The water/cementitious materials ratios shall meet the requirements of Table 19.3.2.1 of ACI 318-14.
  - g. Fly Ash - ASTM C618, Class F - 25% maximum cementitious content.
  - h. Provide air entraining as recommended by Table 19.3.3.1 of ACI 318-14. Concrete that extends above grade and is exposed to freezing and thawing while moist shall be air-entrained. Concrete in unconditioned spaces shall be considered site concrete.
  - i. No aluminum content or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.
5. Construction
- a. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1:25:1. Control joints shall be completed as soon as final set is achieved and it is okay to operate the cutter on the slab. Final set is typically achieved within the first 4 to 12 hours after the slab has been finished in an area (depending on weather conditions and concrete hydration rate, 4 hours in hot weather to 12 hours in cold weather). For early entry saw cutting, joints should be cut within the first 1 to 4 hours (depending on weather conditions and concrete hydration rate, 1 hour for hot weather and 4 hours for cold weather). Where saw cut joints cannot be cut along the entire projected length of the joint, a 90-degree hand grinder or other tool shall be used to complete the joint. Control joints may be installed by:
    - i. Saw cut a depth of 1/4 the thickness of the slab.
    - ii. Tooled joints a depth of 1/4 the thickness of the slab.
  - b. For interior concrete slabs-on-grade that are to receive no floor covering, install construction or control joints in slabs on grade at a spacing not to exceed 24 times the slab thickness in any direction, unless noted otherwise. For interior concrete slabs-on-grade that are to receive floor coverings the contractor has the option to increase the control joint spacing to 36 times the slab thickness in any direction.
6. Construction
- a. Use chairs or other support devices recommended by the CRSI to support and tie reinforcement bars prior to placing concrete. Reinforcing steel for slabs on grade shall be adequately supported. Support reinforcing steel of slabs on grade with precast concrete units. Lifting the reinforcing off the grade during placement of concrete is not permitted.
  - b. Concrete to be mechanically consolidated during placement per ACI standards.
  - c. Contractor shall coordinate placement of all openings, curbs, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
  - d. All embeds, anchors and dowels shall be securely tied to formwork or to adjacent reinforcing prior to the placement of concrete.
  - e. No pipes, ducts, sleeves, etc shall be placed in structural concrete unless specifically detailed or approved by the structural engineer. Penetrations through walls when approved shall be built into the wall prior to concrete placement. Penetrations will not be allowed in footings or grade beams unless detailed. Piping shall be routed around footings and grade beams and unless detailed. Footings shall be stepped to avoid piping.
  - f. Reinforcing Bars shall not be welded. Do not substitute reinforcing bars for DBAs or HSAs.
- POST-INSTALLED ANCHORS
- 1. General Post-Installed Anchor Notes
    - a. Do not install adhesive anchors in concrete if less than 21 days old; do not install mechanical anchors, screw anchor or powder actuated anchors in concrete less than 7 days old. Contractor must obtain written approval from the engineer to install prior to these time periods. Do not apply full load to anchors until concrete has reached 28-day compressive strength.
    - b. Anchors or adhesives specified in details shall be provided; alternative anchors or adhesives may be used if the contractor provides calculations demonstrating that the alternative can achieve the performance values of the specified product. These calculations, along with an ICC-ES ESR or IAPMO-UES ESR approval for use in cracked concrete and compliant with the specified codes herein, must be submitted to the structural engineer prior to use.

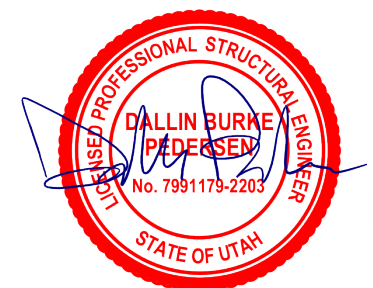
## POST-INSTALLED ANCHORS

1. **General Post-Installed Anchor Notes**
  - a. Do not install adhesive anchors in concrete if less than 21 days old; do not install mechanical anchors, screw anchor or powder actuated anchors in concrete less than 7 days old. Contractor must obtain written approval from the engineer to install prior to these time periods. Do not apply full load to anchors until concrete has reached 28-day compression strength.
  - b. Anchors or adhesives specified in details shall be provided; alternative anchors or adhesives may be used if the contractor provides calculations demonstrating that the alternative can achieve the performance values of the specified product. These calculations, along with an ICC-ES ESR or IAPMO-UES ER approval for use in cracked concrete and compliant with the specified codes herein, must be submitted to the structural engineer prior to use.
  - c. Follow all the manufacturer's recommendations and certification testing reports for anchor installation. See specific anchors below for more information.
  - d. No anchor shall be installed within 1.5 anchor rod diameters of an abandoned hole that has been filled with non-shrink grout; increase distance to 3 anchor rod diameters when the abandoned hole has not been filled.

- 2. Adhesive Anchors**
- a. For anchors in concrete, the adhesives shall be divided into two groups: Standard Adhesives and High Strength Adhesives. Standard adhesives can be used in general applications when details reference the "Standard Adhesive Embedment Schedule" on sheet S601. High Strength adhesive groups will be specified for the particular application in the drawings and details. When a High Strength Adhesive is specified, the contractor has the option to use any of the adhesives in the High Strength group. When a Standard Adhesive is specified, the contractor has the option to use any of the adhesives in either group. See below for the acceptable adhesives in each group.
- i. Standard Adhesive Group for anchors in concrete includes the following adhesives:
1. SET-XP (ICC-ES ESR-2508) by Simpson Strong-Tie
  2. Pure 50+ (ICC-ES ESR-3576) by Dewalt
  3. AC100+ Gold (ICC-ES ESR-2582) by Dewalt
  4. HIT-RE 100 (ICC-ES ESR-3829) by Hilti, Inc.
- ii. High Strength Adhesive Group for anchors in concrete includes the following adhesives:
1. SET-3G (ICC-ES ESR-4057) by Simpson Strong-Tie
  2. Pure 110+ (ICC-ES ESR-3298) by Dewalt
  3. AC200+ (ICC-ES ESR-4027) by Dewalt
  4. HIT-RE 500-V3 (ICC-ES ESR-3814) by Hilti Inc.
  5. HIT-HY 200 (ICC-ES ESR-3187) by Hilti Inc.
- b. Adhesive shall be within the manufacturer's recommended life time and prior to expiration date. Do not use adhesive that has not been stored per manufacturer's recommendations or may have experienced freeze thaw cycles or extreme heat.
- c. Do not install adhesive anchor in wet or damp hole unless product is approved for such conditions without strength reduction. Do not install adhesive anchors if concrete temperature is below 50-degree F unless adhesive is approved for lower temperature without strength reduction. Refer to manufacturer's published installation instructions.
- d. Follow all the manufacturer's recommendations and certification testing reports regarding hole cleaning prior to adhesive installation. All holes shall be drilled with ANSI standard bits designed for concrete. Diamond core drilled holes are not allowed unless indicated in specific details or approved by the structural engineer prior to use.
- 3. Mechanical Anchors**
- a. For concrete, the mechanical anchor shall be Kwik Bolt TZ (ICC-ES ESR-1917) by Hilti Inc., Strong-Bolt 2 (ICC-ES ESR-3037) by Simpson Strong-Tie Inc. or Power-Stud+ SD2 (ICC-ES ESR-2502) by Dewart.
- 4. Screw Anchors**
- a. For concrete, the screw anchors shall be Titen HD (ICC-ES ESR-2713 for concrete only) by Simpson Strong-Tie, or Screw-Bolt+ (ICC-ES ESR-3889 for concrete only) by DeWalt, or Kwik HUS-EZ (ICC-ES ESR-3027 for concrete only) by Hilti Inc.
- 5. Powder Actuated Fasteners**
- a. For fasteners driven into steel (except at metal decks), the fastener shall be X U P8 TH Universal Knurled Shank Fastener (ICC-ES ESR-2269) by Hilti Inc., PDPA (ICC-ES ESR-2138) by Simpson Strong-Tie, or 8mm Head Spiral CSI Drive Pin (ICC-ES ESR-2024) by Dewart.

## STRUCTURAL STEEL



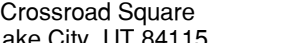
- Material:**
- a. Wide Flanges Section
  - b. All Thread Rods, Other Shapes & Plates
  - c. Square or Rectangular HSS
  - d. Deformed Bar Anchors (DBA)
  - e. Headed Stud Anchors (HSA)
  - f. Anchor Rods
  - Moment Frame Columns
  - g. Bolted Connections:
  - h. All structural steel shapes and plates listed on these structural plans as a Seismic Resisting Element (SRE) shall be tested in accordance with ASTM A6, Supplementary Requirement S30, *Charpy V-Notch Impact Test for Structural Shapes – Alternate Core Location*. The impact test shall meet a minimum average value of 20 ft-lb or greater absorbed energy at 70 degrees Fahrenheit.
    - i. Shapes, with flanges 1 1/2" or thicker
    - ii. Plates, 2" and thicker
    - iii. Test Frequency: Each heat.
2. Fabrication and construction shall comply with the latest edition of the following Codes and Standards:
- a. American Institute of Steel Construction (AISC), "Specification for the Design, Fabrication and Erection of Structural Steel for Buildings," with "Commentary."
  - b. AISC "Code of Standard Practice" excluding the following: Section 3.2, Section 4.4, Section 4.4.1,
  - c. AISC "Specification for Structural Joints Using High-Strength Bolts."
  - d. American Welding Society (AWS), Structural Welding Code (Specific items do not apply when they conflict with the AISC requirements).
  - e. AISC "Seismic Provision for Structural Steel Buildings". ANSI/AISC 341
  - f. All exterior steel elements, including anchor rods and bolts shall be hot-dip galvanized in accordance with ASTM A123 and A153 where applicable.
3. Welding
- a. Field weld flags that have been put in these documents are for suggestion only. The contractor has the option to substitute spot welding for field welding or vice versa. The steel fabrication and steel erection drawings must clearly distinguish between shop welds and field welds prior to any work being performed.
  - b. Steel fabricators shall indicate the shop welds that are excluded from their bids. Steel erectors shall indicate the field welds that are excluded from their bids. It is the responsibility of the contractor to coordinate shop welding and field welding with the appropriate subcontractors.
  - c. All welding and cutting shall be performed by AWS certified welders.
  - d. Use E70 XX (58 ksi yield, 70 ksi tensile) unless noted otherwise. E60 XX may be used for welding steel roof decks.
  - e. All intersecting steel shapes which are not bolted shall be connected by a fillet weld all around, unless noted otherwise. Where fillet weld sizes are not shown they shall be 1/16" less than the thinnest of the connected parts for thicknesses 1/4" and larger. Fillet welds on plates less than 1/4" shall be of the same size as the thinnest of the connected part.
  - f. Reinforcing Bars: Do not weld rebar except as specifically detailed in the drawings. In such cases, use only AWS standards. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors (HSAs).
  - g. Do not weld anchor bolts, including "ack" welds.
  - h. Headed Stud Anchors (HSAs) welding and deformed bar anchor welding shall conform to the manufacturer's specifications.
  - i. Protected zones for braced frames and moment frames shall clearly marked at the job site.
  - j. Welds indicated for elements designated as SRE (Seismic resisting elements) shall be made with a filler metal capable of providing 22% minimum elongation and a minimum Charpy V-Notch (CVN) toughness of 20 ft-lb (27 J) at 0 degrees Fahrenheit. In addition to meeting this requirement, welds indicated as Demand Critical shall be made with a filler metal also capable of providing a minimum Charpy V-Notch (CVN) toughness of 40 ft-lb (54 J) at 70 degrees Fahrenheit. Acceptable electrodes include E70Tc-K2, E71T-B and E71T-1.
  - k. Special Provisions for full penetration welds used in moment frames. Welding methods, procedures and quality control shall comply with ANSI/AWS D1.1 (Latest Edition) and the following:
    - i. Tack weld quality comply with Section 5.18
    - ii. Arc Strikes, gouges and other imperfections within or adjacent to the joint, shall be repaired or removed.
    - iii. Preheat, and interpass requirements as outlined in Section 5.6 or per the electrode manufacturer's guidelines.



12/7/2020



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REV	DATE	Description																		



## GENERAL STRUCTURAL NOTES

- iv. Use weld tabs at beam flange connections; after welding, remove the weld tabs and finish to a smooth contour per Section 5.31.
- v. Backing bars shall be removed from the beam **bottom** flange connections to columns. The root of the weld shall be back gouged to sound metal to remove all slag and cracks. Weld the back gouged region and finish welding using a reinforcing fillet weld, according to Section 5.10.4.
- vi. Backing bars need not be removed from the beam top flange connections to columns provided that the backing bars are 1/4" thick or less and are welded to the column flange with a continuous fillet weld for the entire length of the backing bar.
- vii. Unrepaired cracks, gouges, grooves and notches will not be permitted in the joint area.

4. Bolted Connections:
- a. Use bolts for steel to steel connections, as noted herein or as noted on the drawings. Bolts shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Tighten bolts to a snug tight condition.
- b. Use hardened washers beneath the turned element of all bolts or nuts. Use hardened beveled washers, to compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. At oversized holes hardened washers or plates shall conform with ASTM F-436 and shall completely cover the slot after installation.
- c. Where a steel to steel beam connection is not shown, provide a standard AISC framed connection for one half the total uniform load capacity of the beam for the span and steel specified.
- d. Bolts, nuts and washers shall not be reused.

5. Provide baseplate anchor rod connections to concrete elements that correlate with ACI 117. Circular or square washers are acceptable:

ANCHOR ROD DIAMETER	WASHER SIZE	WASHER THICKNESS (MIN)
3/4"	1 1/2"	1/4"

### METAL DECKING

1. Steel deck shall comply with the latest requirements of the Steel Deck Institute.
2. All deck shall be 3-span continuous minimum. In areas where 3-span conditions are not possible, the contractor shall provide heavier gage deck as required to provide the equivalent loading of the deck under a three span condition.
3. Steel roof deck shall not be used to support loads from plumbing, HVAC ducts, light fixtures, architectural elements or equipment of any kind, unless specifically noted. Light weight suspended acoustical ceilings with a total weight of 50 lbs per attachment may be hung from roof deck. The hangers shall be staggered to distribute the loads over multiple deck flutes.
4. All deck supporting members shall be dry before welding.
5. Clinch seams before welding interlocking seams.

### Steel Roof Deck

- a. Steel roof deck shall be 9/16" deep X **26 gage** minimum painted, shallow deck with interlocking side seams with the following properties:
- Minimum S (in<sup>2</sup>/ft) = 0.041
- Minimum I (in<sup>4</sup>/ft) = 0.013
- b. Minimum allowable deck diaphragm shear values shall be **396 lbs/ft** for a 2'-0" deck span.
- c. Fasten deck to supporting framing members with powder-driven fasteners. Powder-driven fasteners shall be as indicated below based on the steel framing thicknesses.

Steel Framing Thickness	Fastener	ICC-ESR or IAPMO report number
0.125" to 0.375"	Hilti X-HSN-24	ICC-ESR 2776
0.25" and up	Hilti X-ENP-19 L15	ICC-ESR 2776
0.113" to 0.155"	Pneutek SDK61075	ICC-ESR 2941
0.155" to 0.250"	Pneutek SDK63075	ICC-ESR 2941
0.188" to 0.312"	Pneutek K64062	ICC-ESR 2941
0.281" and up	Pneutek K66062	ICC-ESR 2941

- d. Fasteners shall be placed at the following spacings (Closer spacings may be used to develop minimum shear requirements):
- i. 6" o.c. to all supports perpendicular to deck corrugations (7 fasteners per 36" sheet).
- ii. 6" o.c. to all supports parallel to deck corrugations.
- e. Provide a 2" minimum bearing and a 4" lap at the splice points.

### COLD-FORMED STEEL

1. All cold-formed steel shall meet the requirements of "Specifications for the Design of Cold-Formed Steel Structural Members" by American Iron and Steel Institute (AISI).
2. All cold-formed steel connectors shall be provided by The Steel Network. If the contractor elects to substitute for another manufacturer, the contractor shall submit a revised connector list, prior to construction, that includes the following information:
- a. Specified connector indicated on these plans
- b. Requested substitution connector
- c. Allowable capacity of the requested substitution connector
3. Light Gauge Steel Framing:
- a. Galvanized steel shall meet the minimum requirements of ASTM A653 (Fy = 50 ksi) for 97 mil (12 gauge), 68 mil (14 gauge) and 54 mil (16 gauge). For 43 mil (18 gauge) and lighter galvanized steel shall meet and ASTM A653 (Fy = 33 ksi). Galvanized coatings must meet the ASTM A924.
- b. Follow all manufacturers' recommendations for the use of these products.
- c. Unless noted otherwise, all welded connections shall be done according to AWS standards.
- d. All interior non-bearing steel stud walls that extend above the ceiling but do not attach to the structure above shall be brace with diagonal metal-stud braces (45 degrees). The k/l/r ratio of the brace shall not exceed 200 and shall not be spaced further apart than 10'-0" o.c. Connect diagonal braces to the top of the steel stud walls and to the top flange of the steel beams with two #10 tek screws minimum. Where a concrete deck occurs above, use two powder-driven fasteners per diagonal brace. Other approved methods may be used.

### LEGEND OF MARKS AND ABBREVIATIONS

ANCHOR ROD(S)	ABOVE	K	KIPS(S) - 1000 POUNDS
ALTERNATE	ALTERNATE	KLF	KIPS PER LINEAL FOOT
APPROX	APPROXIMATE	KSF	KIPS PER SQUARE FOOT
ARCH	ARCHITECTURAL	LBS	POUNDS
		LF	LINEAL FOOT
BUILDG	BUILDING	MAX	MAXIMUM
BLW	BELOW	MECH	MECHANICAL
BM	BEAM	MF-X	MOMENT FRAME
BOT	BOTTOM	MFR	MANUFACTURER
BRG	BEARING	MIN	MINIMUM
BWTN	BETWEEN	MISC	MISCELLANEOUS
		MSW	METAL STUD WALL
CC	CENTER-TO CENTER		
C.J.	CONST/CONTROL JOINT		
CIP	COMPLETE JOINT PENETRATION	NIC	NOT IN CONTRACT
	GROOVE WELD (FULL PEN WELD)	NTS	NOT TO SCALE
COL	COLUMN		
CONC	CONCRETE	O.C.	ON CENTER
CONST	CONSTRUCTION	O.F.	OUTSIDE FACE
CTR	CENTER	OPNG	OPENING
CW-X	CONCRETE WALL	OPP	OPPOSITE
DB	DECK BEARING	PAF	POWDER-ACTUATED FASTENER
DBA	DEFORMED BAR ANCHOR	PCF	POUNDS PER CUBIC FOOT
DBE	DECK BEARING ELEVATION	PL	PLATE
DBL	DOUBLE	PLF	POUNDS PER LINEAL FOOT
DET	DETAIL	PSF	POUNDS PER SQUARE FOOT
DIA	DIAMETER	PSI	POUNDS PER SQUARE INCH
DIM	DIMENSION	PT	POINT
DN	DOWN	P-T	POST-TENSION
DWG	DRAWING		
DWL	DOWEL	REINF	REINFORCING
		REQD	REQUIRED
(E)	EXISTING	ROO DRAIN	ROOF DRAIN
EA	EACH	RTU	ROOF TOP UNITS
E.F.	EACH FACE		
E.J.	EXPANSION JOINT	SBP-X	STEEL BASE PLATE MARK
ELEC	ELECTRICAL	SCW	SEISMIC CRITICAL WELD
ELEV	ELEVATION	SC-X	STEEL COLUMN WELD
EQUIP	EQUIPMENT	SCP-X	STEEL CAP PLATE MARK
EQ	EQUAL	SHT	SHEET
E.W.	EACH WAY	SI	SPECIAL INSPECTION
EXT	EXTERIOR	SIM	SIMILAR
		SMU	SUSPENDED MECHANICAL UNITS
FC-X	CONTINUOUS FOOTING MARK	SOB	SLAB-ON-GRADE
F.D.	FLOOR DRAIN	SQ	SQUARE
FDN	FOUNDATION	SRE	SEISMIC RESISTING ELEMENT
F.F.	FINISHED FLOOR	STAG	STAGGERED
FR-X	RECTANGULAR FOOTING	STD	STANDARD
FS-X	SQUARE FOOTING MARK	STL	STEEL
FT	FOOT	STR	STRUCTURAL
FTG	FOOTING	STS	SELF TAPPING SCREWS
FTS-X	THICKENED SLAB MARK	T&B	TOP AND BOTTOM
		TEMP	TEMPERATURE
GA	GAUGE	THDS	THREADS
GALV	GALVANIZED	T.O.	TOP OF
GSN	GENERAL STRUCTURAL NOTES	TOC	TOP OF CONCRETE
		TOD	TOP OF DECK
HORIZ	HORIZONTAL	TOF	TOP OF FOOTING
HSA	HEADED STUD ANCHOR	TOS	TOP OF STEEL
HT	HEIGHT	TOW	TOP OF WALL
		TYP	TYPICAL
ICC	INTERNATIONAL CODE COUNCIL	UNO	UNLESS NOTED OTHERWISE
IBC	INTERNATIONAL BUILDING CODE		
IF	INSIDE FACE	VERT	VERTICAL
IN	INCH		
INT	INTERIOR	W/	WITH
		WT	WALL THICKNESS
JO	JOIST	WWF	WELDED WIRE FABRIC
JST		WWM	WELDED WIRE MESH

### STATEMENT OF SPECIAL INSPECTION AND QUALITY ASSURANCE

Special inspection and quality assurance (including structural testing), as required by section 1704 and 1705 of the 2018 IBC, shall be provided by an independent agency employed by the owner for the items in this section and other areas of the approved construction documents, unless waived by the building official.

The names and credentials of the Special Inspectors to be used shall be submitted to the Building Official for approval.

#### Responsibilities of the Special Inspector

Special Inspector shall review all work listed in the special inspection schedules herein for conformance with the approved construction plans, specifications and 2018 IBC.

Testing and inspection reports shall be sent on a weekly basis to the architect, engineer, building official and contractor for review. All items not in compliance shall be brought to the immediate attention of the contractor for correction, and if uncorrected, to the architect, engineer and building official.

Once corrections have been made by the contractor, the special inspector shall submit a final signed report to the building official stating that the work requiring special inspection was, to the best of the special inspector's knowledge, in conformance with the approved construction plans, specifications and 2018 IBC.

#### Responsibilities of the Contractor

The contractor shall submit a written statement of responsibility to the owner and the building official prior to the commencement of work in accordance with 2018 IBC section 1704.4. This statement shall indicate that the contractor will coordinate and cooperate with the required inspections contained herein.

The contractor shall notify the designated special inspector that work is ready for inspection at least 24 hours before said inspection is required.

All work requiring special inspection shall remain open and accessible until it has been observed by the special inspector and deemed acceptable through inspection report.

Special inspection during fabrication is not required if the fabricator is registered and approved by the authority having jurisdiction to perform such work without special inspection. Upon completion of fabrication, the approved fabricator shall submit a certificate of compliance for submittal to the building official.

The contractor shall be responsible for their own quality control including materials, fabrication, erection, etc.

### SOILS CONSTRUCTION INSPECTIONS

#### Soils (2018 IBC Section 1705.6)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY	COMMENTS
	CONTINUOUS	PERIODIC
Site Preparation	-	X
Fill Material	X	-
Continuous Footing Backfill: at least one test for each 40 linear feet or less of wall length, but no fewer than 2 tests.	-	X
Spot Footing Backfill: Minimum of one compaction test for each lift for each spot footing.	-	X
See specifications for further requirements.	-	-

### CONCRETE CONSTRUCTION INSPECTIONS

Concrete (2018 IBC Section 1705.3, Table 1705.3, and Section 1705.12) The following concrete elements require special inspection:

All concrete footings, All concrete walls, including foundation walls, Interior concrete slab-on-grade

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY	COMMENTS
	CONTINUOUS	PERIODIC
Protection of concrete during cold and hot weather	-	X
Verify materials used including use of the required mix design	-	X
Formwork	-	X
Bolts installed in concrete	X	-
Embeds and Inserts installed in concrete	X	-
Concrete reinforcing steel placement	-	X
Concrete placement and samples	X	-
See specifications for further concrete testing requirements.	-	-

### STEEL BOLTED CONSTRUCTION INSPECTIONS

Where special inspections are listed under "Random Basis", special inspection of elements and items shall be performed on a random basis. Operations need not be delayed pending these inspections. Where special inspection items are listed under "Every Element", special inspection shall be performed for each element, joint, or member, as applicable based on the task listed below.

#### High Strength bolted connections (2018 IBC section 1705.2.1, section 1705.12.1 and section 1705.13.1 and AISC 360-16 Chapter N and AISC 341-16 Chapter J)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION PLAN	COMMENTS
	Every Element	Random Basis
Inspection Tasks Prior to Bolting		
Manufacturer's certifications available for fastener materials	X	-
Fasteners	-	X
Proper fasteners selected for the joint detail	-	X
Proper bolting procedure selected for joint detail	-	X
Connecting elements	-	X
Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	-	X*
Proper storage	-	X
Inspection Tasks During Bolting		
Fastener assemblies, of suitable condition	-	X
Joint	-	X
Fastener component	-	X
Pretensioned Fasteners	-	X
Inspection Tasks After Bolting		
Document acceptance or rejection of each bolted connection	X*	-
*Required for elements designated in these structural drawings as "Seismic Resisting Elements – SRE".		

### POST-INSTALLED ANCHOR INSPECTIONS

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY	COMMENTS
	CONTINUOUS	PERIODIC
Post-Installed Anchors and Reinforcing Bars (2018 IBC Section 1705.1.1)		
Adhesive Anchors and Reinforcing Bars	X	-
Mechanical Anchors and Screw Anchors	-	X

### STRUCTURAL OBSERVATION PROGRAM

If structural observations are required, they shall be done by the Engineer of Record or an approved subordinate at the stages of construction listed in the Construction Notification Phases section of these notes. At the conclusion of the project, the designated structural observer shall submit to the building official a written statement that the site visits have been made and identify any reported deficiencies that to the best of the structural observer's knowledge have not been resolved (See IBC 2018 1704.6).

STRUCTURAL OBSERVATION PROGRAM REQUIRED BY CODE:	YES	NO
		X

### CONSTRUCTION MILESTONE SCHEDULE

CONTRACTOR TO NOTIFY ENGINEER AT THE FOLLOWING CONSTRUCTION PHASES:

CONCRETE	
Footings, stem walls and piers	Prior to pouring concrete
STEEL	
Roof framing	After substantial portion of framing is erected

### DEFERRED SUBMITTALS

For the purposes of this section, deferred submittals are defined as per section 107.3.4.1 of the IBC 2018. Submittal documents for deferred submittal items shall be submitted to the engineer, architect and building official for their review for general conformance with the design of the building.

#### DEFERRED STRUCTURAL SUBMITTALS FOR THIS PROJECT ARE

None
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### STEEL WELDED CONSTRUCTION INSPECTIONS

#### Definition of Terms

Where special inspections are listed under "Random Basis", special inspection of elements and items shall be performed on a random basis. Operations need not be delayed pending these inspections. Where special inspection items are listed under "Every Element", special inspection shall be performed for each element, joint, or member, as applicable based on the task listed below.

#### Structural Welding (2018 IBC section 1705.2 and section 1705.12.1 and section 1705.13.1 and AISC 360-16 Chapter N and AISC 341-16 Chapter J)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION PLAN	COMMENTS
	Every Element	Random Basis
Inspection Tasks Prior to Welding		
Welding procedures specifications and manufacturer certifications for welding consumables shall be available	X	-
Material identification (type/grade)	-	X
Welder identification system	-	X
Fit-up of groove welds	X*	X
Configuration and finish of access holes	-	X
Fit-up of fillet welds	X*	X
Check welding equipment	-	X
SRE Welds and Demand Critical Welds	X	-
*Required for elements designated in these structural drawings as "Seismic Resisting Elements – SRE". May be reduced from "Every Element" to "Random Basis" after ten satisfactory welds have been completed. Subsequently, the inspector may change the requirements from "Random Basis" to "Every Element" and vice versa at their discretion.		
Inspection Tasks During Welding		
Use of qualified welders	-	X
Control and handling of welding consumables	-	X
Cracked tack welds	-	X
Environmental conditions	-	X
WPS followed	-	X
Welding techniques	-	X
Inspection Tasks After Welding		
Welds cleaned	-	X
Size, length and location of welds	X	-
Welds meet visual acceptance criteria	X**	-
Arc strikes, k-area, weld access holes for flanges greater than 2", backing removed and weld tabs removed (if required), repair activities	X**	-
Ultrasonic testing (UT) for complete joint-penetration (CJP) groove welds, partial penetration groove welds when used in column splices, and welds subject to fatigue	-	X
Placement of reinforcing or contouring fillet welds	X**	-
Ultrasonic testing (UT) for base metal thicker than 1.1/2", loaded in tension in the through-thickness direction in T- and corner-joints where the connected material is greater than 3/4" and contains CJP groove welds	X**	-
Document acceptance or rejection of each welded joint or member	X**	-

Including crack prohibition, weld/base-metal fusion, crater cross section, weld profiles, weld size, undercut and porosity.

When welding of doubler plates, continuity plates, or stiffeners has been performed in the k-area, visually inspect the web k-area for cracks within 3" of the weld.

Perform UT on 100% of welds subject to transversely applied tension loading in butt, T- and corner joints, in material 5/16" thick or greater. For materials less than 5/16" thick, ultrasonic testing is not required. The UT rate must be increased to 100% if the rejection rate exceeds 5% of the welds tested. See Sections N5.5d and N5.5f for more information.

Provide UT for discontinuities found within t/4 of the steel surface shall be accepted or rejected based on criteria of A.W.S. D1.1/D1.1M Table 6.2 where 't' is the thickness of the part subjected to tension load. All deficient welds shall be corrected and tested at no additional cost to the owner.

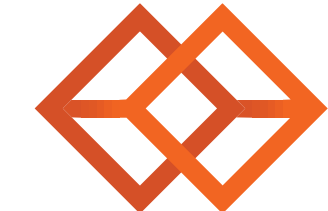
\*\* Required for elements designated in these structural drawings as "Seismic Resisting Elements – SRE". All welding inspections and nondestructive testing shall satisfy the requirements above and AWS D1.8/D1.8M. The inspector shall document the work has been performed in accordance with the contract documents, either in the shop or field, including noncompliant work and whether that work has been satisfactorily repaired.

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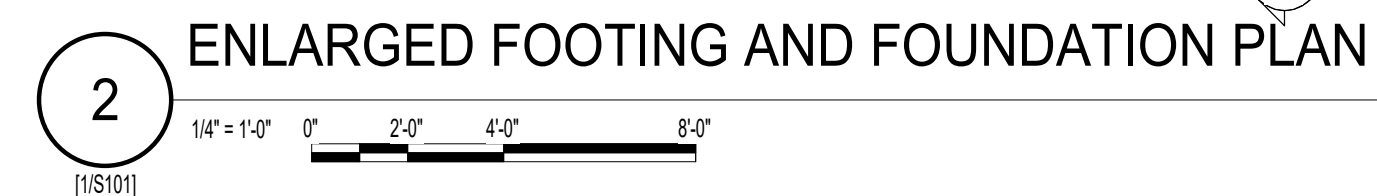
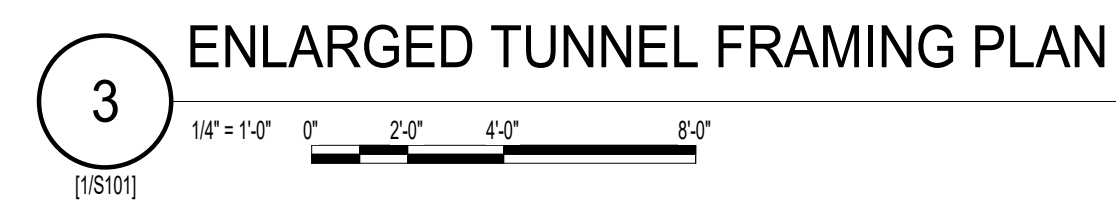
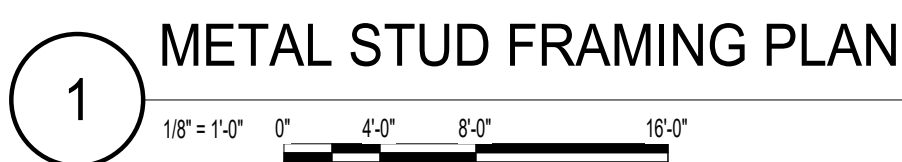
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














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<div> <div>ARCHIPLEX GROUP</div> <div>architecture • planning • design services</div> <div>255 Crossroad Square</div> <div>Salt Lake City, UT 84115</div> <div>P:(801) 961-7070</div> <div>F:(801) 961-7373</div> </div> <div>Submitted By: <i>Naomi Starnik</i></div>	<div> <div>UTAH</div> <div>UTAH TRANSIT AUTHORITY</div> </div> <div>Approved By: _____</div>					
<div> <div>0</div> <div>12/7/20</div> <div>PERMIT SET</div> </div> <div> <div>REV</div> <div>DATE</div> <div>DESCRIPTION</div> </div>						



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	FOOTING DESIGNATION
	TOP OF FOOTING ELEVATION
	SECTION MARK
	SHEET NUMBER
	INDICATES CONCRETE WALL, DASHED WALLS STOP AT DECK
	INDICATES EXISTING WALL
	INDICATES METAL STUD WALL, DASHED WALLS STOP AT DECK
	INDICATES CONCRETE FOUNDATION WALL TYPE. SEE SCHEDULE ON SHEET S801
	INDICATES METAL STUD TYPE. SEE SCHEDULE ON SHEET S802
	INDICATES CONTINUOUS FOOTING. SEE SCHEDULE ON SHEET S801
	INDICATES STEEL COLUMN. SEE SCHEDULE ON SHEET S801
	INDICATES METAL STUD HEADER. SEE SCHEDULE ON SHEET S802
	INDICATES METAL ROOF DECK. SEE GENERAL STRUCTURAL NOTES ON SHEET S002
	INDICATES MOMENT CONNECTION. SEE DETAILS
	INDICATES SEISMIC RESISTING ELEMENT, SEE ELEVATIONS AND DETAILS AND FOLLOW ADDITIONAL SPECIAL INSPECTIONS.

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.
3. LOCATE TOP OF FOOTING A MINIMUM OF 2'-0" BELOW TOP OF GRADE. CONTRACTOR TO VERIFY AND COORDINATE WITH EXISTING FOOTING.
4. SEE DETAIL 4/S502 FOR BOTTOM TRACK FASTENER DETAIL.

5. SEE DETAIL 5/S502 FOR METAL TOP TRACK SPLICE DETAIL.
6. VERIFY ALL ROOF OPENINGS FOR MECHANICAL SHAFTS, DRAINS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
7. ALL ROOF OPENINGS GREATER THAN, OR EQUAL TO, 12' x 12' SHALL BE FRAMED AS INDICATED IN DETAIL 8/S501.



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Raem Stankow



Raem Stankow

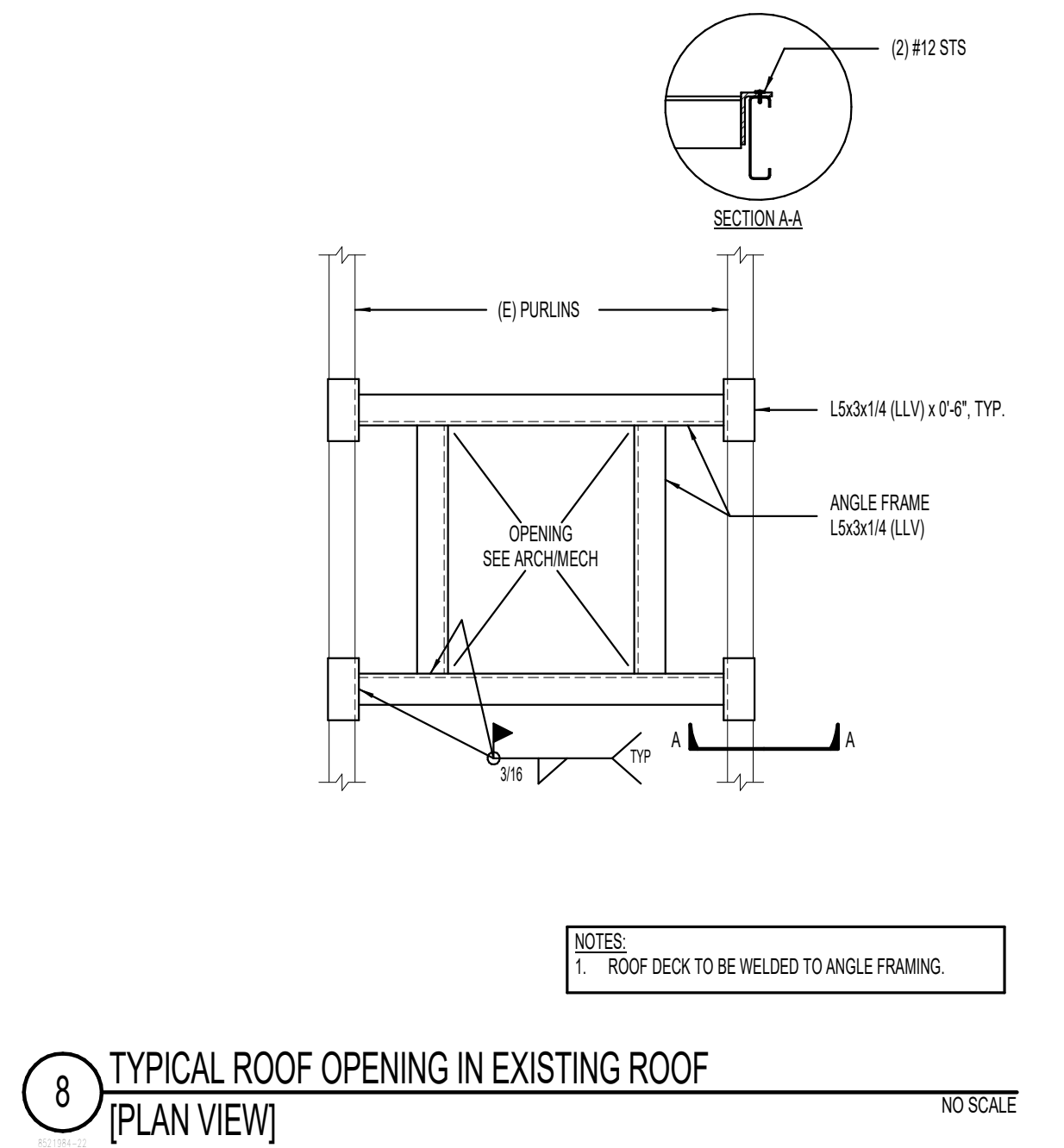
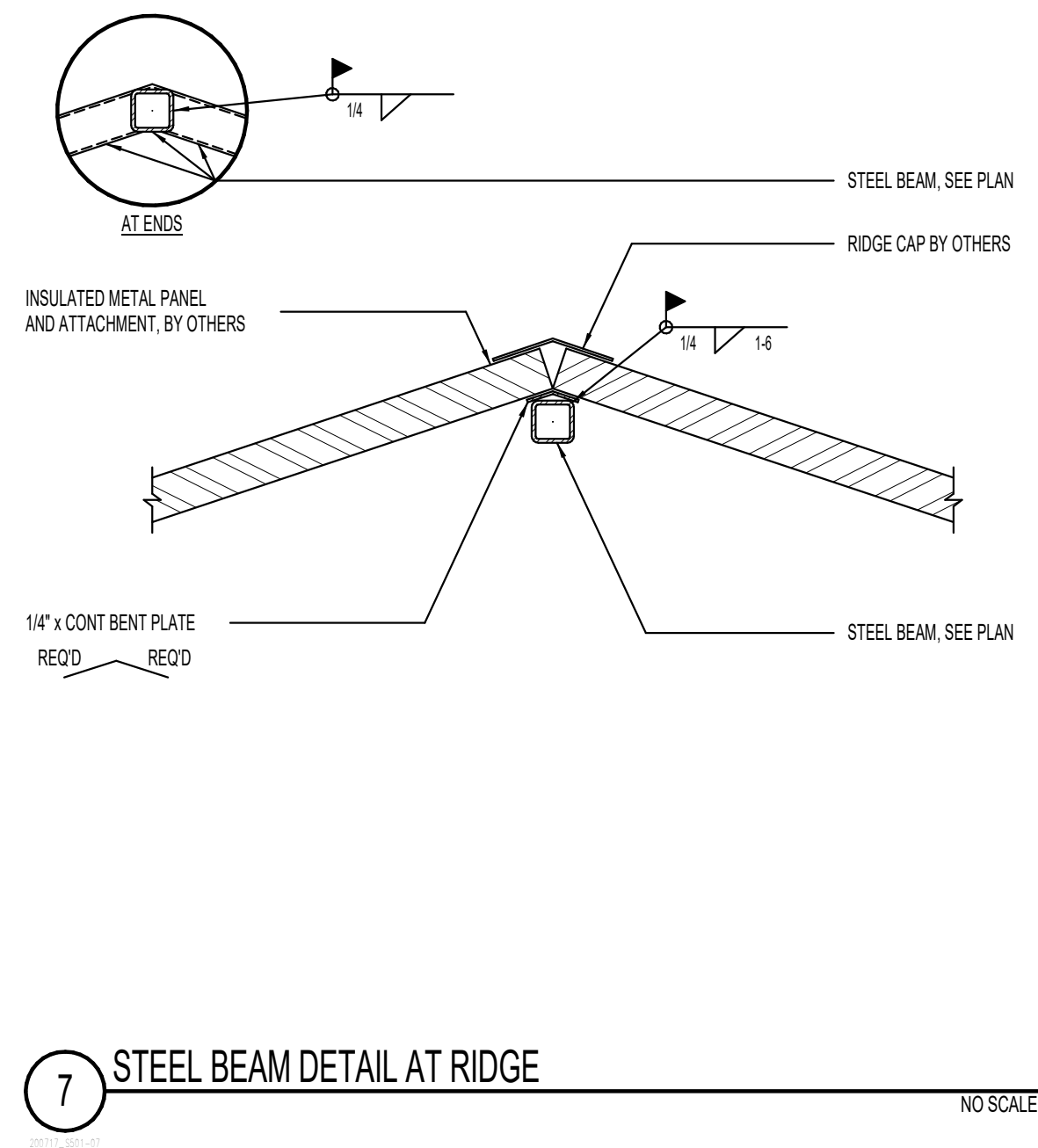
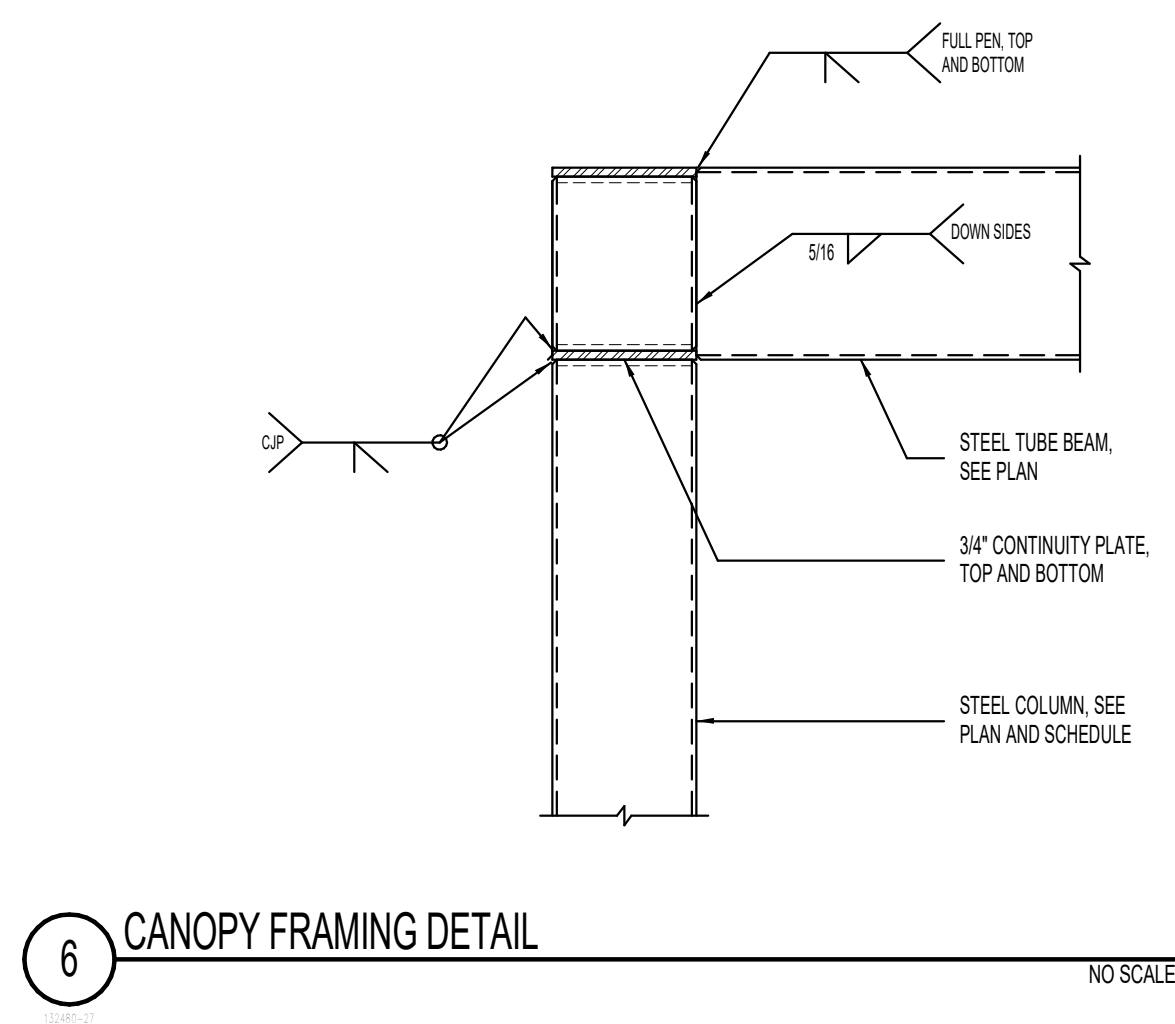
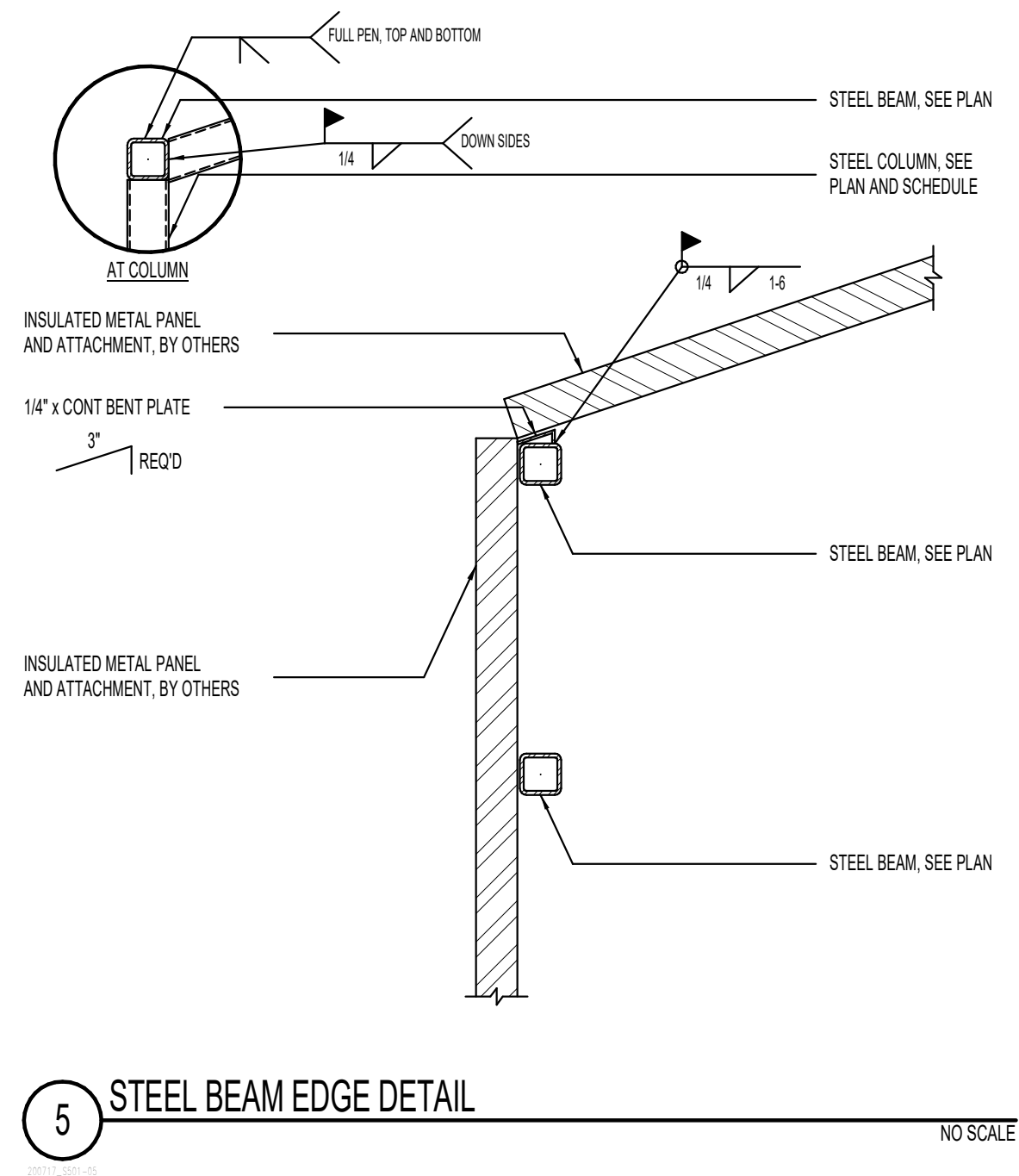
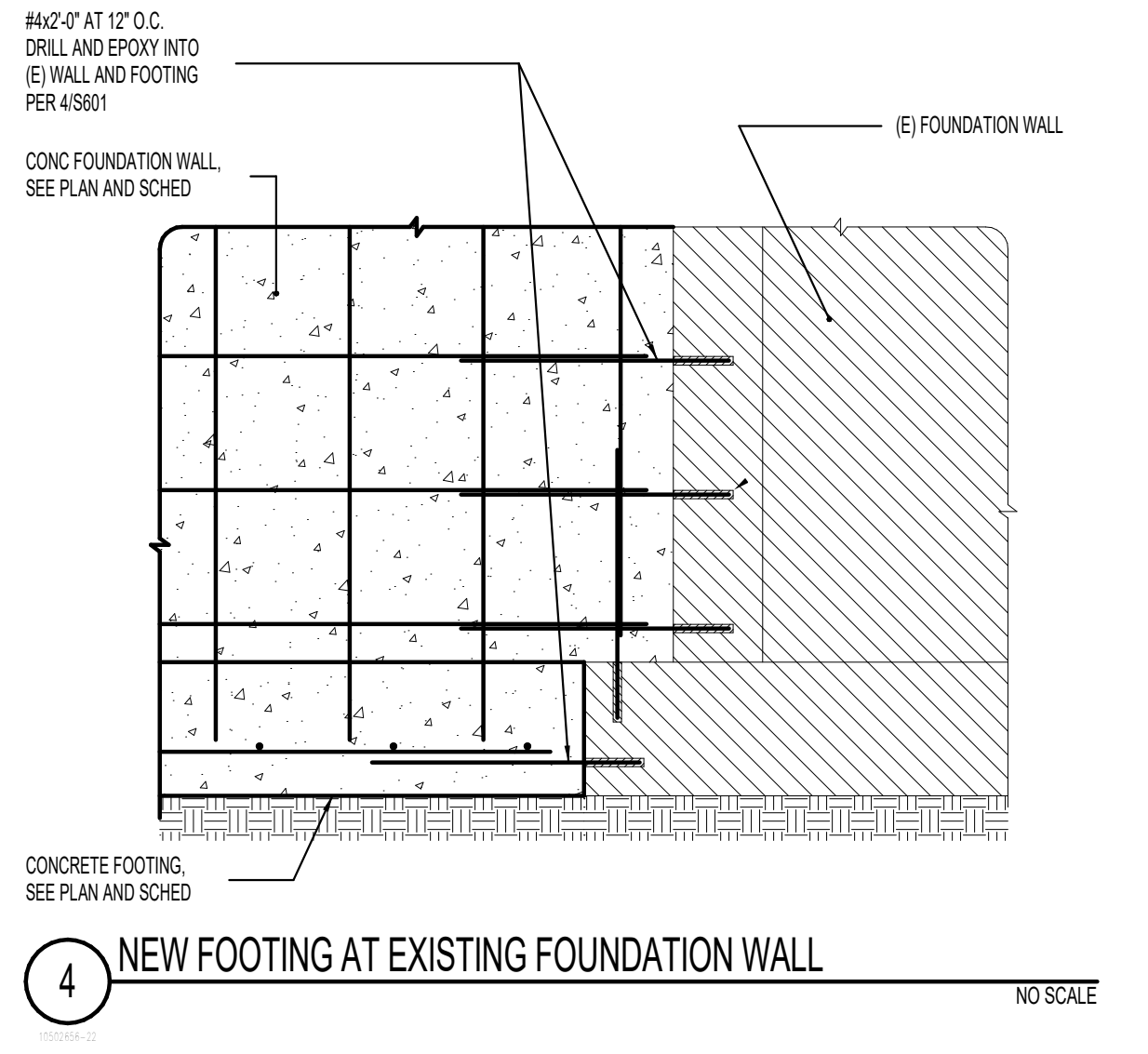
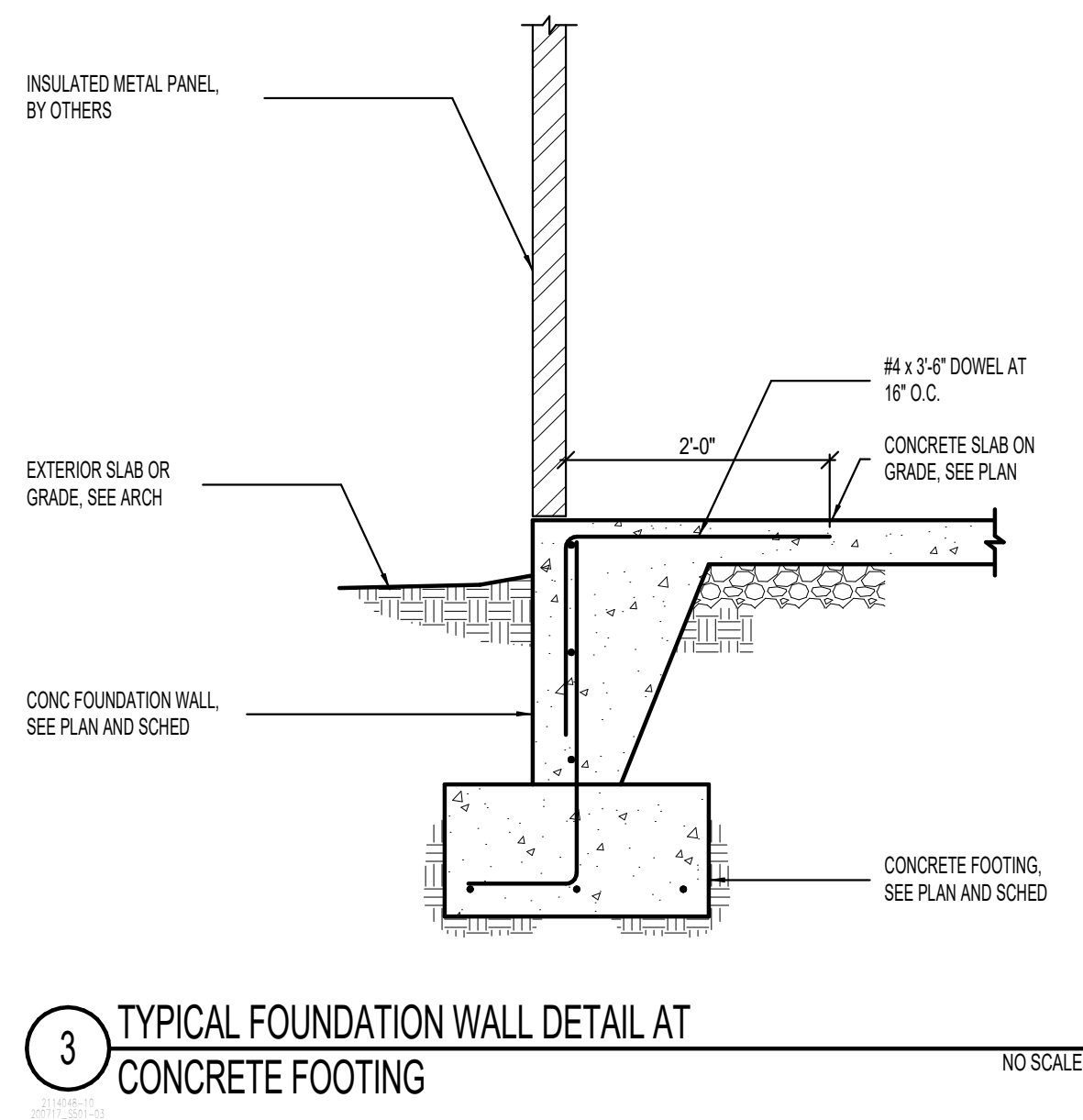
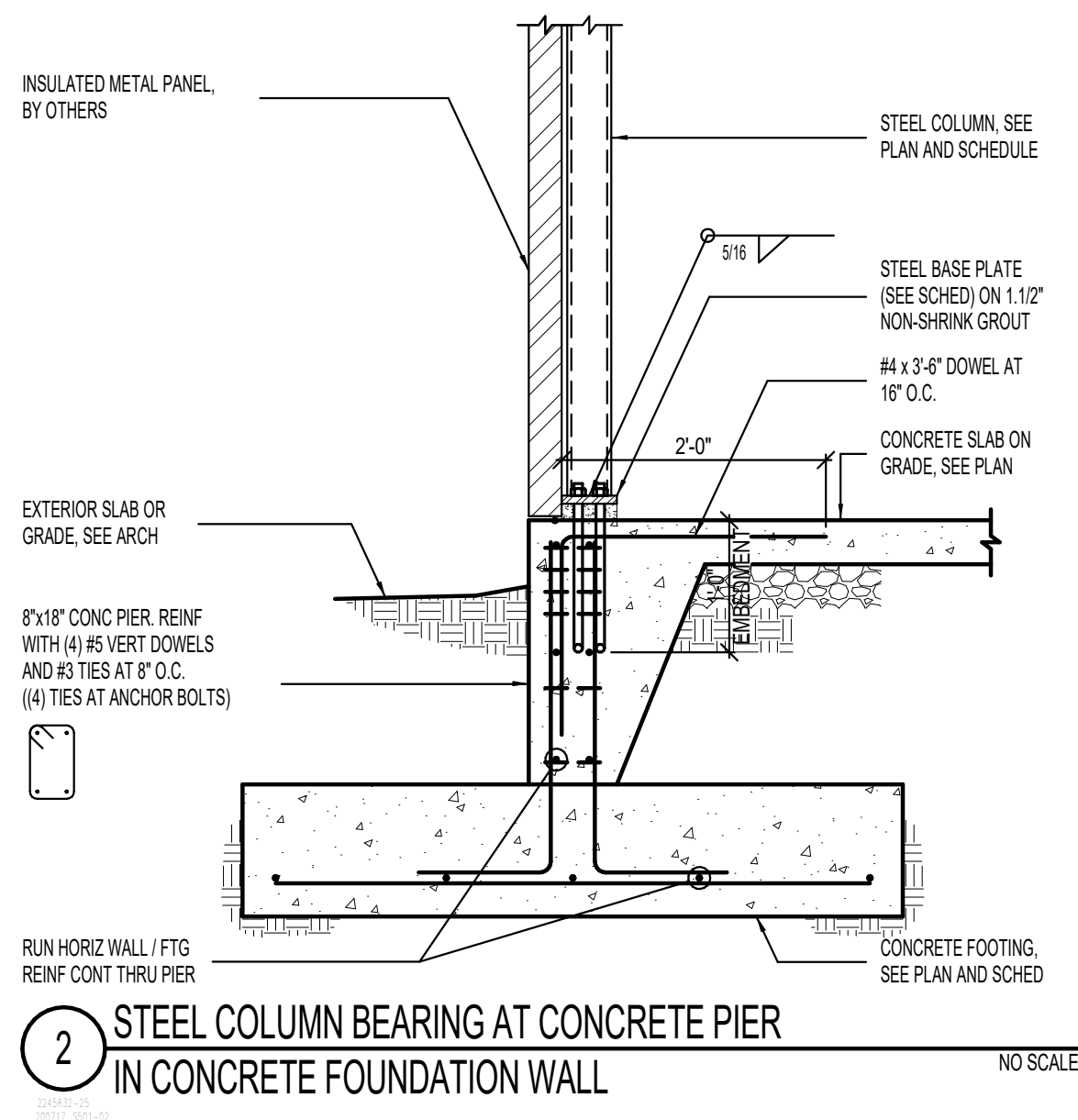
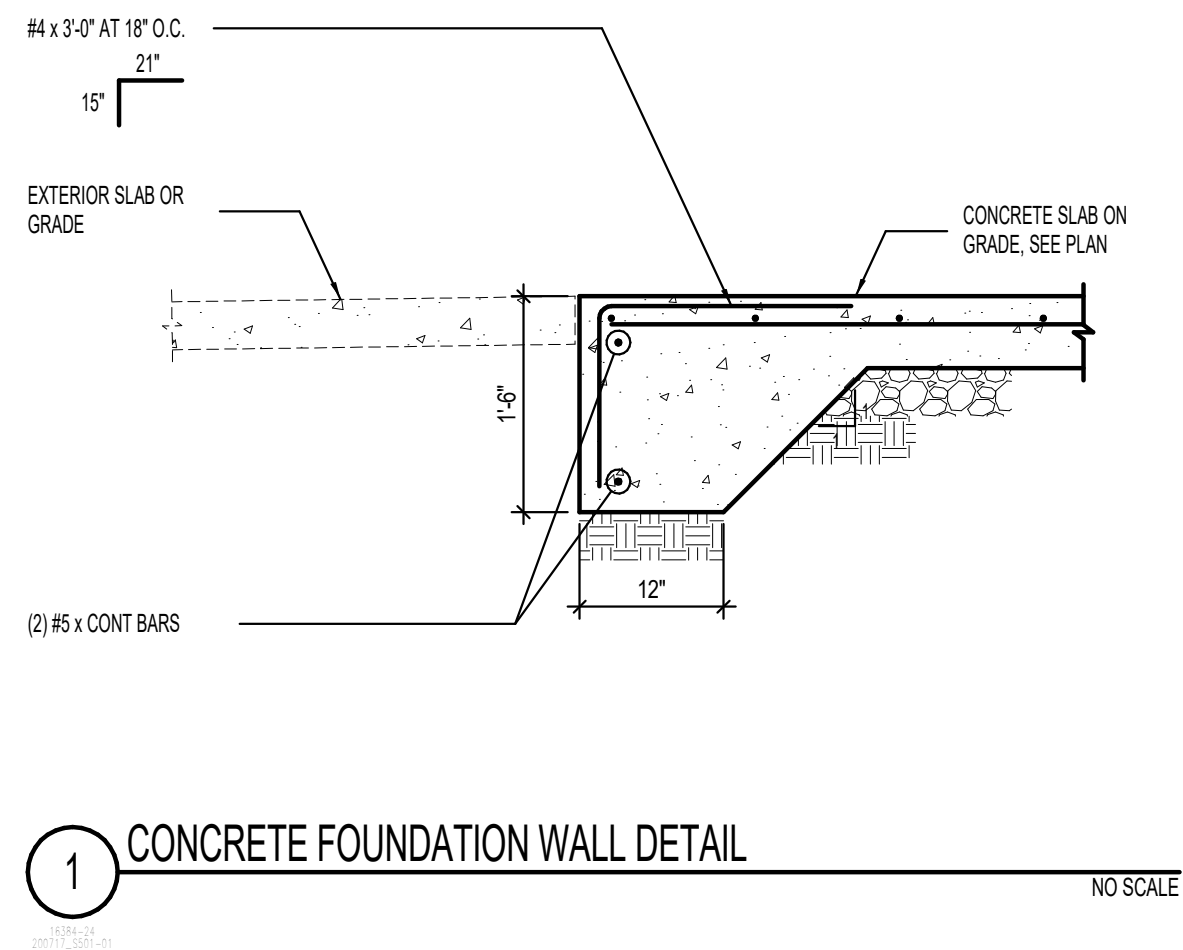
Approved By:

FOOTING AND FOUNDATION / FRAMING PLANS

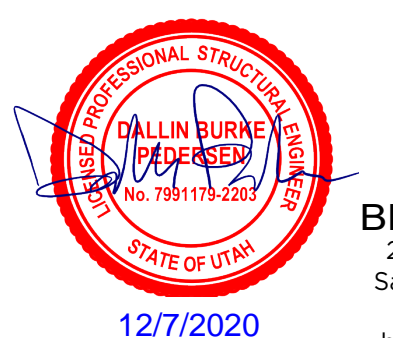
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[illegible]





NOTES:  
1. ROOF DECK TO BE WELDED TO ANGLE FRAMING.



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REV	DATE	DESCRIPTION
0	12/7/20	PERMIT SET

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Submitted By: *Naomi Stenhouse*

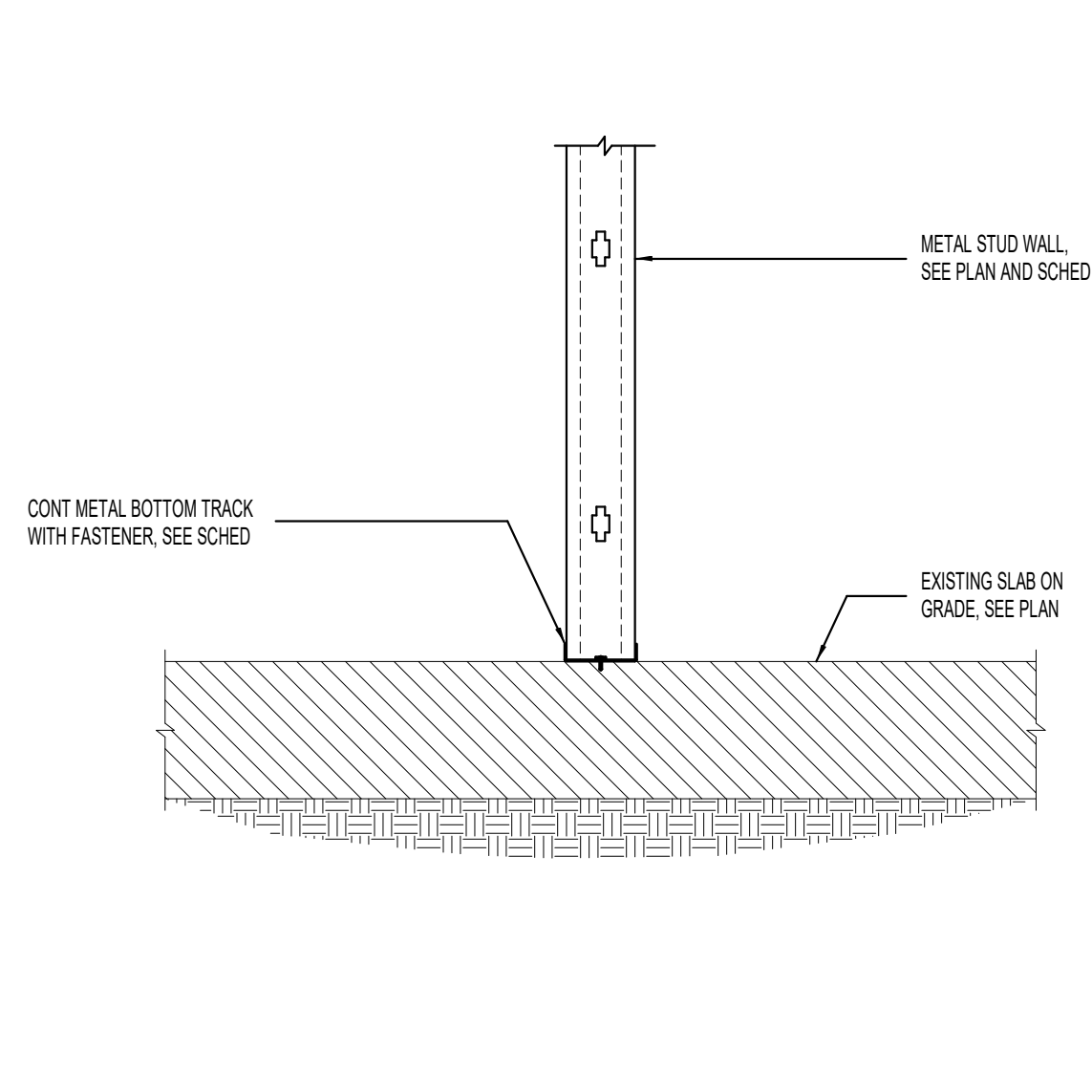
**UTA**  
UTAH TRANSIT AUTHORITY

Approved By: \_\_\_\_\_

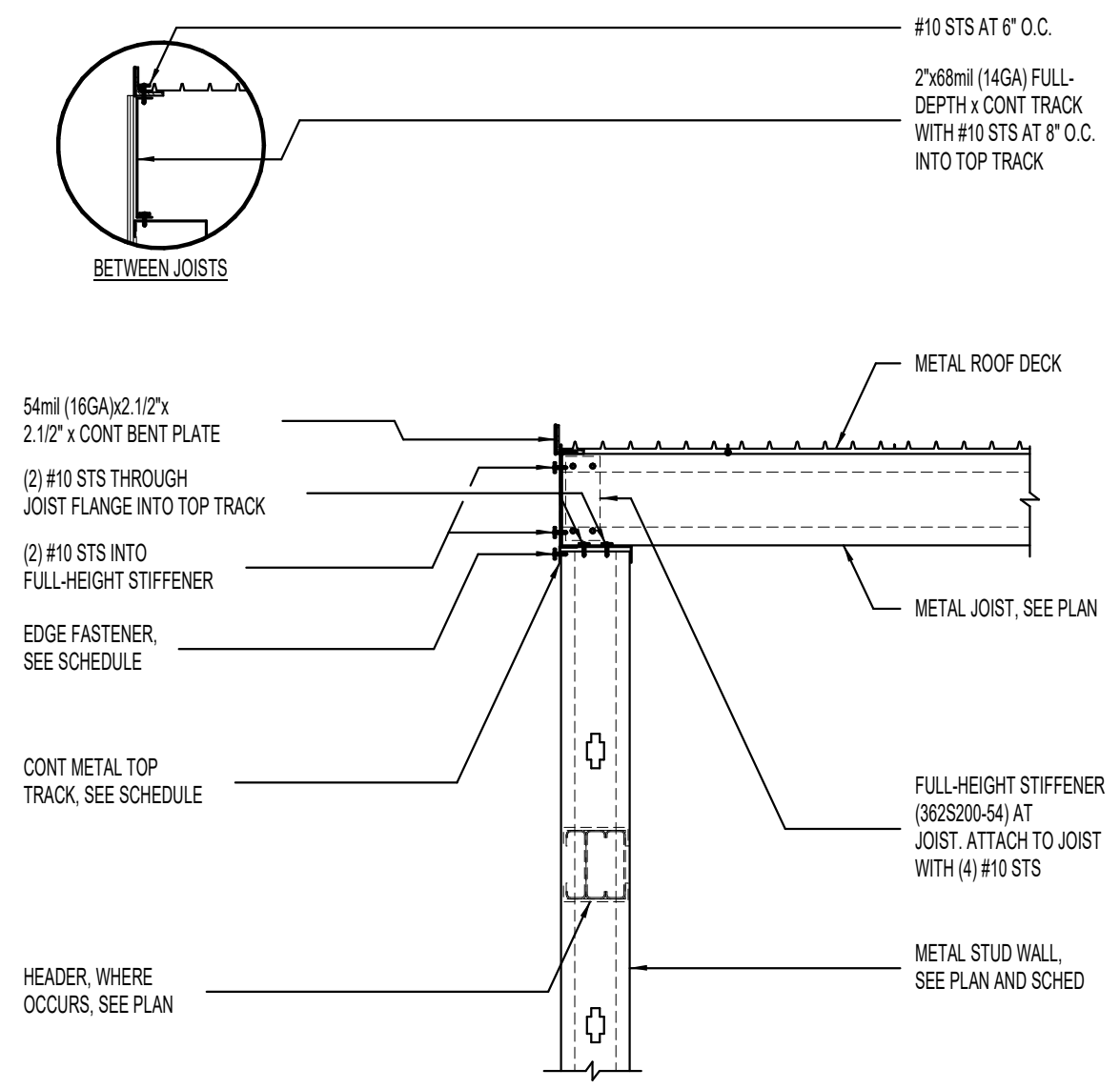
Designed By: \_\_\_\_\_  
Designer  
Drawn By: \_\_\_\_\_  
Author  
Checked By: \_\_\_\_\_  
Checker  
Approved By: \_\_\_\_\_

UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
DETAILS

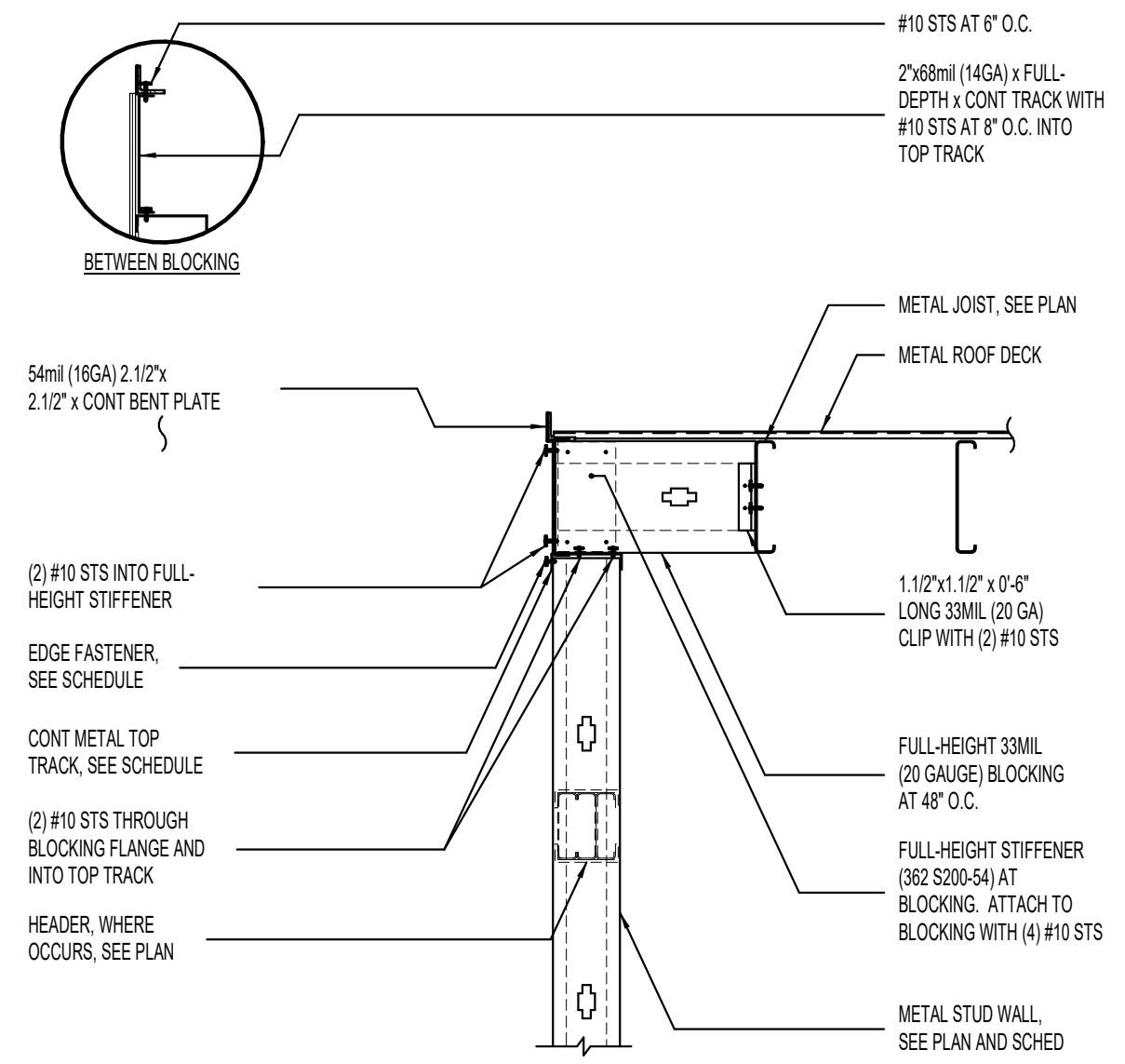
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Submittal Date:	DECEMBER 7, 2020
UTA Project No.:	SGR-358
BHB Project No.:	200717
Sheet No.:	<b>S501</b>



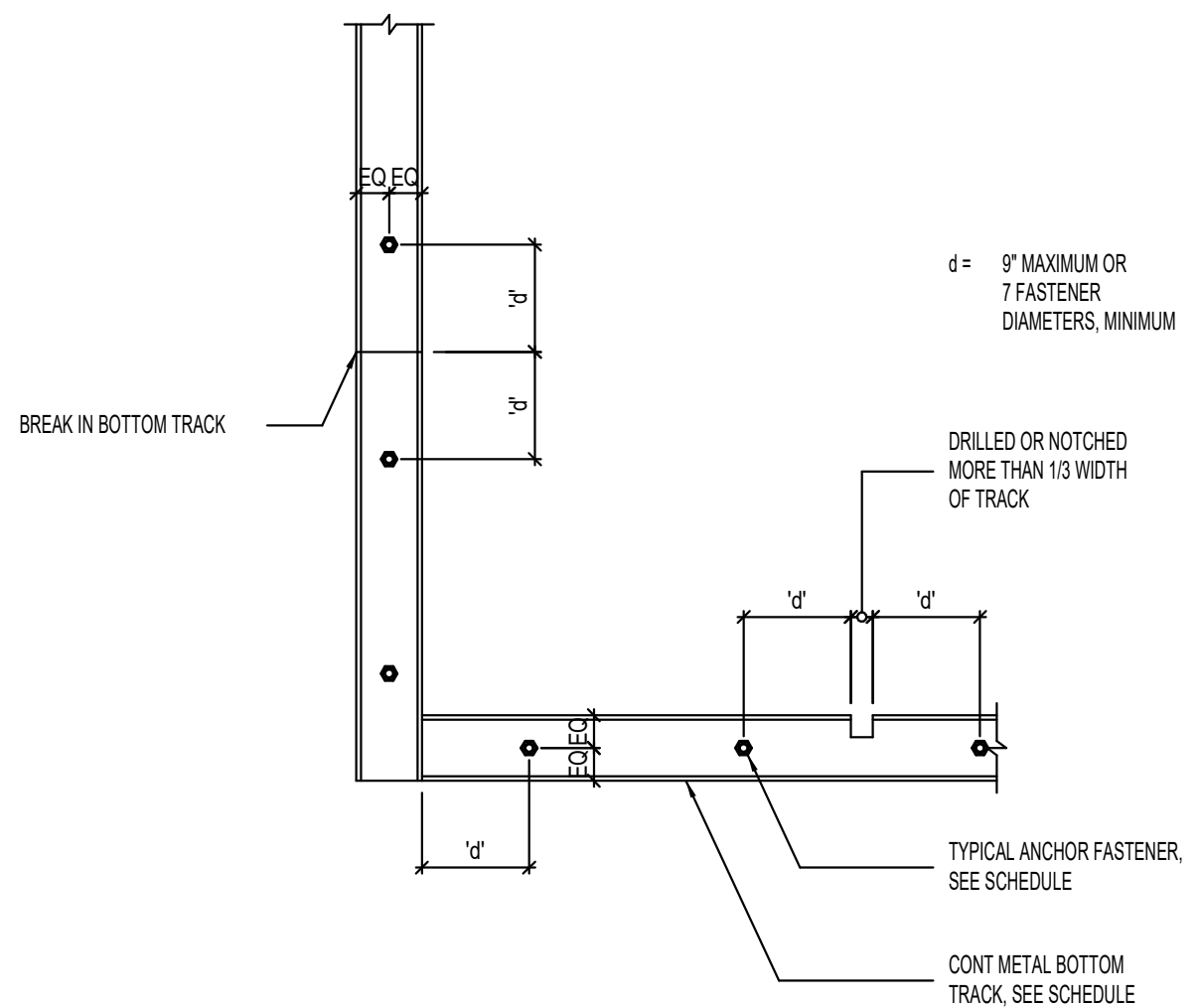
1 METAL STUD WALL ON EXISTING SLAB  
NO SCALE



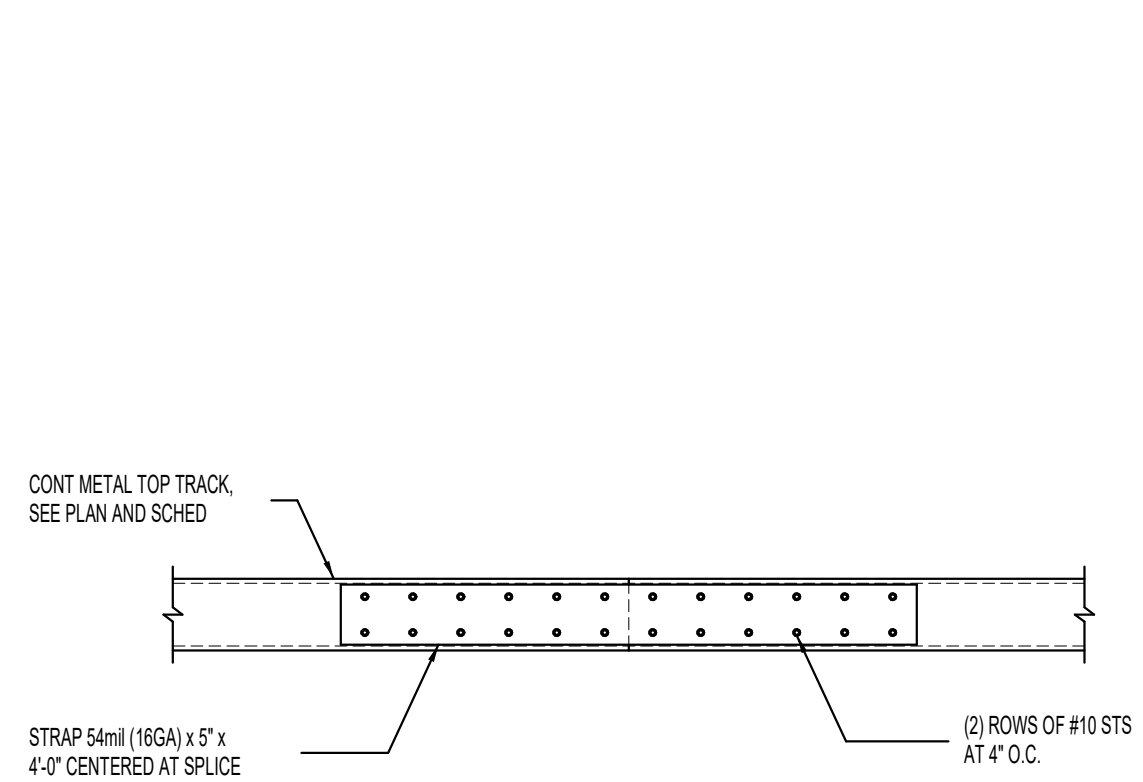
2 JOIST BEARING AT METAL STUD WALL  
NO SCALE



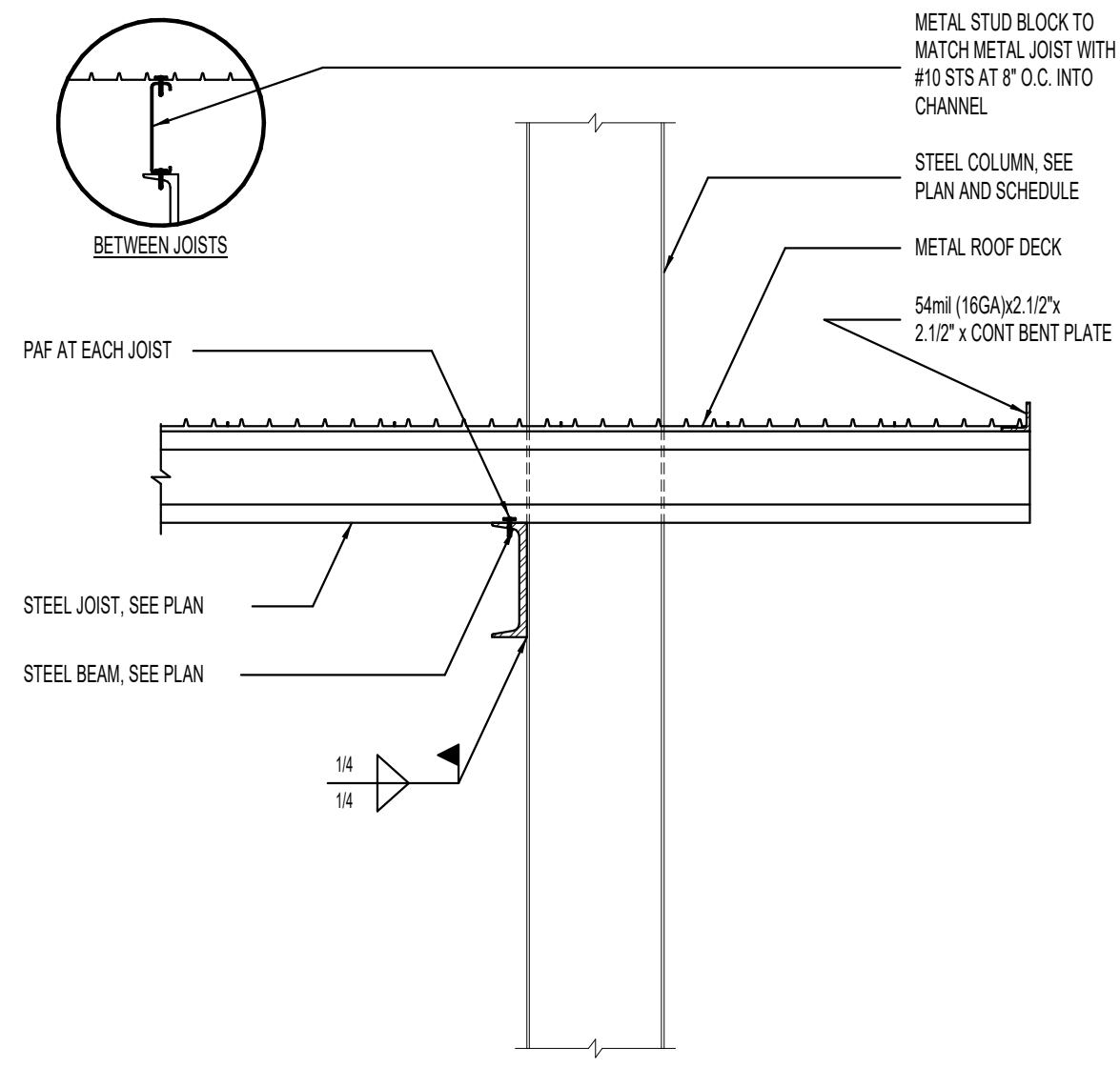
3 JOIST BEARING AT METAL STUD WALL  
NO SCALE



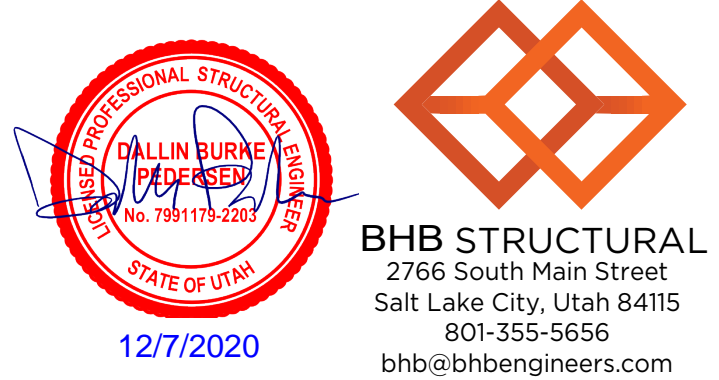
4 BOTTOM TRACK FASTENER DETAIL  
[PLAN VIEW]  
NO SCALE



5 METAL TOP TRACK SPLICE DETAIL  
[PLAN VIEW]  
NO SCALE



6 STEEL JOIST BEARING AT EXISTING STEEL COLUMN  
NO SCALE



1			2			3			4			5					
						<div><div>ARCHIPLEX GROUP</div><div>architecture • planning • design services</div><div>255 Crossroad Square Salt Lake City, UT 84115 P:(801) 961-7070 F:(801) 961-7373</div></div>			<div>UTA</div> <div>UTAH TRANSIT AUTHORITY</div>			<div>Designed By:</div> <div>Designer</div> <div>Drawn By:</div> <div>LM</div> <div>Checked By:</div> <div>JP</div> <div>Approved By:</div>			<div>Scale:</div> <div>Project Status:</div> <div>PERMIT SET</div> <div>Submittal Date</div> <div>DECEMBER 7, 2020</div> <div>UTA Project No.:</div> <div>SGR-358</div> <div>BHB Project No.:</div> <div>200717</div> <div>Sheet No.:</div> <div>S502</div>		



C

B

A

CONCRETE FOOTING SCHEDULE											
MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE			
				No.	SIZE	LENGTH	SPACING	No.	SIZE	LENGTH	SPACING
FC3-0	3'-0"	CONT	12"	-	#5	2'-6"	14"	3	#5	CONT	EQ
THICKENED SLAB											

CONCRETE FOOTING NOTES:

1. PLACE ALL FOOTING REINFORCING IN THE BOTTOM OF THE FOOTING WITH 3" CLEAR CONCRETE COVER (UNO).
2. TOP REINFORCING, WHERE OCCURS, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH-FORMED, FOOTINGS SHALL BE 6" LONGER AND WIDER THAN SCHEDULED.
4. RUN CONTINUOUS FOOTING REINFORCEMENT THROUGH SPOT FOOTINGS.
5. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
6. SOME SCHEDULED FOOTINGS MAY NOT BE USED. SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS.

1 CONCRETE FOOTING SCHEDULE

NO SCALE

CONCRETE REINFORCING BAR LAP SPICE SCHEDULE																				
BAR SIZE	f <sub>c</sub> = 3000psi & f <sub>c</sub> = 3500 psi						f <sub>c</sub> = 4000psi & f <sub>c</sub> = 4500 psi						f <sub>c</sub> = 5000psi				f <sub>c</sub> = 6000psi			
	REGULAR			TOP			REGULAR			TOP			REGULAR		TOP		REGULAR		TOP	
	CLASS		CLASS	CLASS		CLASS	CLASS		CLASS	CLASS		CLASS	CLASS		CLASS	CLASS		CLASS	CLASS	
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B
#3	17"	22"	22"	28"	15"	19"	19"	24"	13"	17"	17"	22"	12"	16"	15"	20"				
#4	22"	29"	29"	37"	19"	25"	25"	32"	17"	22"	22"	28"	16"	20"	20"	27"				
#5	28"	36"	36"	47"	24"	31"	31"	40"	22"	28"	28"	36"	20"	26"	26"	33"				
#6	33"	43"	43"	56"	29"	37"	37"	48"	26"	33"	33"	43"	24"	31"	31"	40"				
#7	48"	63"	63"	81"	42"	54"	54"	70"	37"	49"	49"	63"	34"	44"	44"	58"				
#8	55"	72"	72"	93"	48"	62"	62"	80"	43"	56"	55"	72"	39"	51"	51"	66"				
#9	62"	81"	81"	105"	54"	70"	70"	91"	48"	63"	63"	81"	44"	57"	57"	74"				
#10	70"	91"	91"	118"	61"	79"	79"	102"	54"	70"	70"	91"	50"	64"	64"	83"				
#11	78"	101"	101"	131"	67"	87"	87"	113"	60"	78"	78"	101"	55"	71"	71"	93"				

TABULATED VALUES ARE FOR CASE 1 REINFORCEMENT, WHERE THE REQUIREMENTS OF TABLE BELOW ARE MET. WHERE THESE CONDITIONS ARE NOT MET, MULTIPLY THE LAP LENGTHS (  $l_d$ ) BY 1.5.

REQUIREMENT FOR CASE 1 LAP LENGTHS		
BAR CLEAR SPACING	CLEAR COVER	STIRRUPS OR TIES
$\geq 4d_b$	$\geq 4d_b$	$\geq$ CODE FOR MINIMUM THROUGHOUT $l_d$
$\geq 2d_b$	$\geq 2d_b$	NO REQUIREMENT

$d_b$  = BAR DIAMETER

CONCRETE REINFORCING BAR LAP SPICE NOTES:

1. THIS SCHEDULE SHALL BE USED FOR ALL BAR SPICES IN CONCRETE WALLS, UNLESS NOTED OTHERWISE.
2. CLASS 'A' SPICES MAY BE USED ONLY IN CASES WHERE 50% OR LESS OF THE BARS ARE SPICED WITHIN THE LAP SPICE LENGTH.
3. CLASS 'B' SPICES SHALL BE USED FOR ALL SPICES UNLESS THE REQUIREMENTS OF NOTE No. 2 ABOVE ARE MET.
4. TIES AND STIRRUPS SHALL NOT BE SPICED.
5. DO NOT SPICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY SHOWN.
6. THE VALUES TABULATED IN SCHEDULE ARE FOR GRADE 60 REINFORCING BARS. FOR GRADE 75, MULTIPLY LAP LENGTHS BY 1.25 AND FOR GRADE 80, MULTIPLY BY 1.33.
7. THE VALUES TABULATED IN SCHEDULE ARE MINIMUM REQUIREMENTS. LONGER LENGTHS MAY BE USED FOR CONSTRUCTION.
8. TOP BARS ARE CLASSIFIED AS HORIZONTAL BARS WHERE 12" OR MORE OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BAR.
9. FOR EPOXY-COATED OR ZINC AND EPOXY DUAL-COATED BARS WITH CLEAR COVER  $< 3d_b$  OR CLEAR SPACING  $< 4d_b$ , MULTIPLY LAP LENGTHS BY 1.5. FOR ALL OTHER CASES MULTIPLY BY 1.2.
10. FOR LIGHT WEIGHT CONCRETE, MULTIPLY LAP LENGTHS BY 1.33 UNLESS THE AVERAGE SPLITTING TENSILE STRENGTH (F<sub>t</sub>) IS SPECIFIED. REFER TO A0318-14 SECTION 19.2.4.3.
11. SPICES FOR BUNDLED BARS:
  - a. FOR BUNDLED BARS OF THREE OR LESS, LAP SPICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
  - b. FOR BUNDLED BARS OF FOUR OR MORE, LAP SPICE LENGTHS SHALL BE MULTIPLIED BY 1.33.
  - c. INDIVIDUAL BAR SPICES WITHIN A BUNDLE SHALL NOT OVERLAP.
  - d. ENTIRE BUNDLES SHALL NOT BE LAP SPICED.
12. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

2 CONCRETE REINFORCING BAR LAP SPICE SCHEDULE

NO SCALE

CONCRETE WALL SCHEDULE					
MARK	THICKNESS	REINFORCING			COMMENTS
		VERTICAL	HORIZONTAL	TOP AND BOTTOM	
CW-8A	8"	#4 AT 18" O.C.	#4 AT 12" O.C.	(1) #4	A

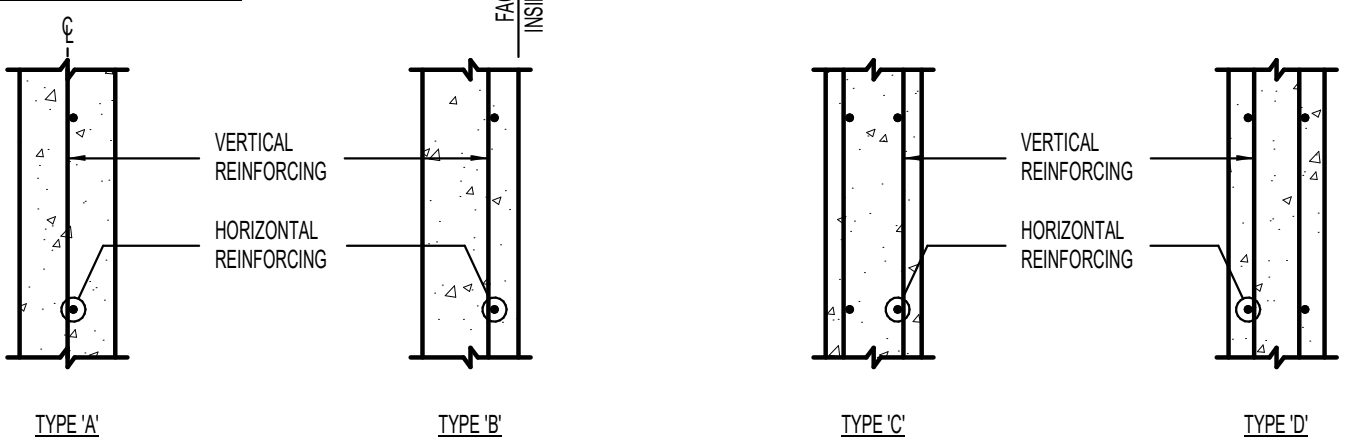
ABBREVIATIONS:  
E.F. EACH FACE  
I.F. INSIDE FACE  
O.F. OUTSIDE FACE

WALLS NOT DESIGNATED IN PLAN		
THICKNESS	REINFORCING	
	VERTICAL	HORIZONTAL
6"	#4 AT 18" O.C.	#4 AT 16" O.C.
8"	#4 AT 18" O.C.	#4 AT 12" O.C.
10"	#4 AT 18" O.C.	#5 AT 16" O.C.
12"	#4 AT 18" O.C. E.F.	#4 AT 16" O.C. E.F.

CONCRETE FOUNDATION WALL NOTES:

1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

WALL REINFORCING PLACEMENT TYPES:



3 CONCRETE WALL SCHEDULE

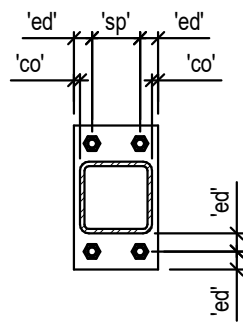
NO SCALE

STEEL COLUMN SCHEDULE				
MARK	SIZE	STEEL BASE PLATE	STEEL CAP PLATE	COMMENTS
SC-4A	HSS4x4x1/4	3/4" (SBP-1)	1/2" (SCP-1)	.

STEEL COLUMN NOTES:

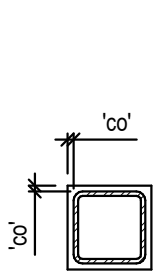
1. UNLESS NOTED OTHERWISE, ALL COLUMNS SHALL BE INSTALLED WITH (4) 3/4"DIA ANCHOR RODS. PROJECT ANCHOR RODS 3" MINIMUM ABOVE THE TOP OF THE BASE PLATE. EMBEDMENT SHALL BE 12" MINIMUM. ALL RODS SHALL BE INSTALLED WITH HARDENED WASHERS BENEATH THE NUT. ANCHOR RODS LARGER THAN THE ROD DIAMETER PLUS 5/16" SHALL HAVE 5/16" PLATE WASHERS INSTALLED BENEATH THE HARDENED WASHERS.
2. ANCHOR RODS SHALL NOT BE WELDED (INCLUDING TACK WELDS).
3. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

STEEL BASE PLATE TYPES:



BASE PLATE SBP-1

STEEL CAP PLATE TYPES:



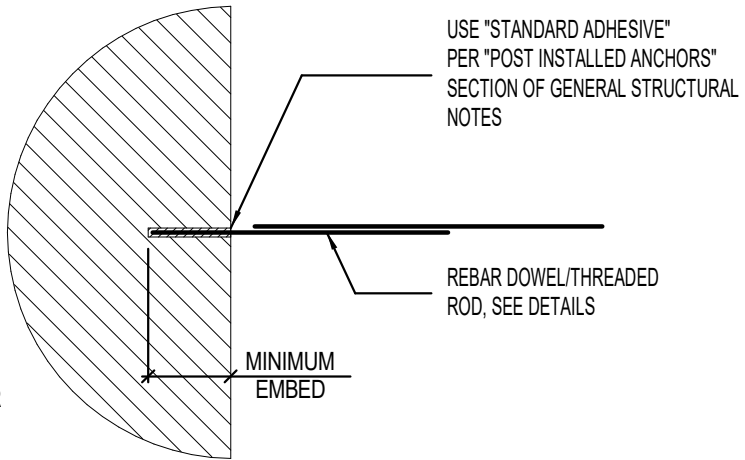
CAP PLATE SCP-1

PLATE LEGEND  
cd = 1/2" MINIMUM  
ed = 1 1/2" MINIMUM  
sp = 3" MINIMUM  
bc = BEAM OR GIRDER GAGE  
w = BEAM OR GIRDER GAGE + 3"  
OR  
BEAM OR GIRDER WIDTH + 1"  
COLUMN WIDTH + 1"  
WHICHEVER IS GREATER

STANDARD ADHESIVE EMBEDMENT SCHEDULE	
REBAR DOWEL (THREADED ROD SIZE)	MINIMUM EMBEDMENT INTO CONCRETE OR GROUTED MASONRY
#3 (3/8")	3.38"
#4 (1/2")	4.12"
#5 (5/8")	5.08"
#6 (3/4")	6.34"

STANDARD ADHESIVE EMBEDMENT NOTES:

1. SPECIFIC EMBEDMENTS, NOTES AND DETAILS IN DRAWINGS SHALL GOVERN OVER THIS SCHEDULE.
2. HOLE DIAMETER SHALL BE DOWEL/ROD DIAMETER PLUS 1/8". FOLLOW MANUFACTURER'S INSTRUCTIONS FOR HOLE PREPARATION.
3. PROVIDE A 3" MINIMUM EDGE DISTANCE TO CENTER OF HOLE.
4. CONTACT STRUCTURAL ENGINEER IF MINIMUM EMBEDMENTS INDICATED ABOVE ARE NOT ACHIEVABLE.
5. SEE "POST INSTALLED ANCHORS" SECTION OF GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



4 STANDARD ADHESIVE EMBEDMENT SCHEDULE

NO SCALE

5 STEEL COLUMN SCHEDULE

NO SCALE

1

2

3

4

5

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Submitted By: *Naomi Starnik*

UTAH TRANSIT AUTHORITY

Approved By: \_\_\_\_\_

Designed By: \_\_\_\_\_

Drawn By: LM

Checked By: JP

Approved By: \_\_\_\_\_

UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
SCHEDULES

Scale: \_\_\_\_\_

Project Status: PERMIT SET

Submittal Date: DECEMBER 7, 2020

UTA Project No.: SGR-358

BHB Project No.: 200717

Sheet No.: S601

12/7/20

PERMIT SET

REV DATE DESCRIPTION

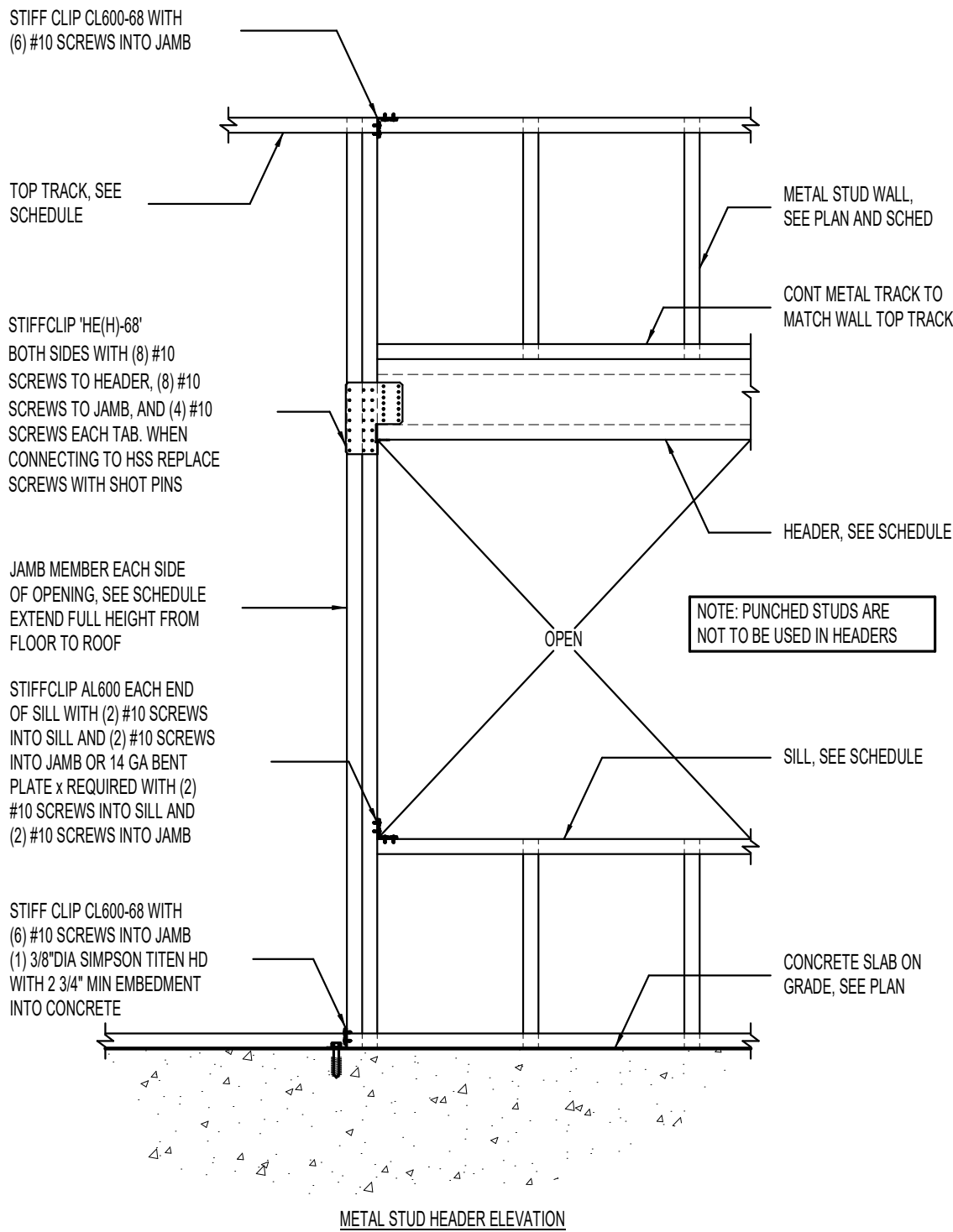
C

B

A

HEADER/JAMB SCHEDULE					
MARK	CONDITION	JAMB TYPE	HEADER TYPE	SILL	COMMENTS
MH-1	METAL STUD	J1	H1	-	

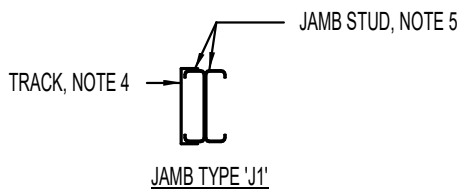
- HEADER/JAMB NOTES:
1. ATTACH ALL COMPONENTS TOGETHER WITH #10 SCREWS AT 6" O.C. OR ATTACH COMPONENTS 18 GA OR HIGHER WITH 1/8"x 1" FILLET WELDS AT 12" O.C.
  2. SCREWS SHALL PENETRATE THROUGH FRAMING MEMBER WITH AT LEAST THREE THREADS.
  3. STUDS SHALL BE A MINIMUM OF 1.5/8" WIDE WITH A 3/8" MINIMUM RETURN LIP. JAMB STUD TO MATCH WALL STUD.
  4. TRACKS SHALL BE A MINIMUM OF 1.1/4" WIDE. TRACK GAUGE AND DEPTH TO MATCH WALL STUD.
  5. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



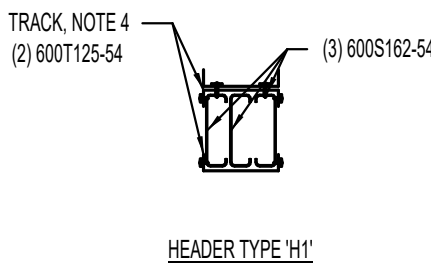
1 STEEL HEADER/JAMB SCHEDULE

NO SCALE

JAMB TYPES



HEADER TYPES

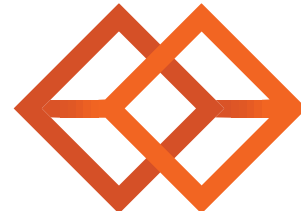


METAL STUD WALL SCHEDULE					
WALL MARK	WALL FRAMING				COMMENTS
	METAL STUDS	TOP TRACK	BOTTOM TRACK	BOTTOM TRACK FASTENERS	
MSW-6A	600S162-54 AT 16" O.C.	600T125-54	600T125-54	HILTI 'X-U' x 1.1/2" POWER ACTIVATED FASTENERS AT 24" O.C.	

- METAL STUD WALL NOTES:
1. SCREWS SHALL PENETRATE THROUGH FRAMING MEMBER WITH AT LEAST THREE THREADS.
  2. STUDS SHALL BE A MINIMUM OF 1.5/8" WIDE WITH A 3/8" MINIMUM RETURN LIP.
  3. TRACKS SHALL BE A MINIMUM OF 1.1/4" WIDE.
  4. HORIZONTAL BLOCKING SHALL BE PROVIDED AT 4'-0" O.C. MAX.
  5. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

2 METAL STUD WALL SCHEDULE

NO SCALE



**BHB STRUCTURAL**  
2766 South Main Street  
Salt Lake City, Utah 84115  
801-355-5656  
bhb@bhbengineers.com

REV	DATE	Description
0	12/7/20	PERMIT SET



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Submitted By: *Naomi Stenmark*






Approved By: \_\_\_\_\_

Designed By:	Designer
Drawn By:	LM
Checked By:	JP
Approved By:	

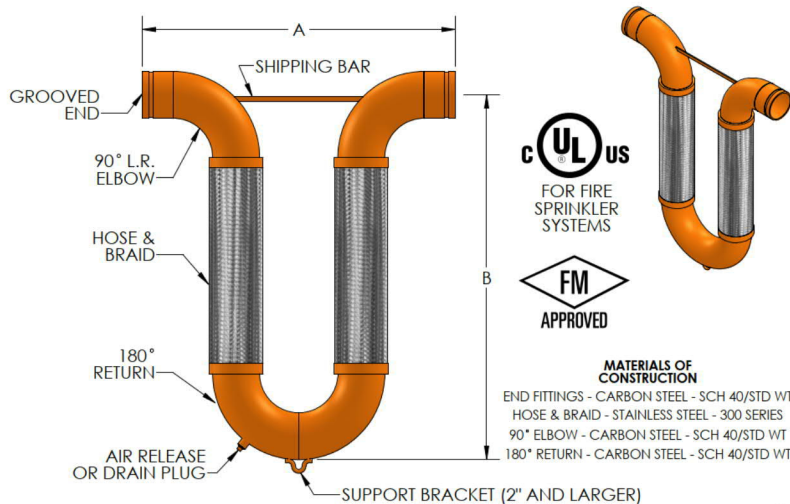
UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
SCHEDULES

Scale:	
Project Status:	PERMIT SET
Submittal Date:	DECEMBER 7, 2020
UTA Project No.:	SGR-358
BHB Project No.:	200717
Sheet No.:	S602



Sprinkler Legend											
Symbol	Manufacturer	SIN	Model	Quantity	K-Factor	Type	Size	Response	Finish	Temperature	Note
	Generic	TY5211	Tyco ELO-231	30	11.2	Pendent	¾"	Standard	Chrome	286°F	401esc & BAGS FOR EACH
	Generic	TY5111	Tyco ELO-231	5	11.2	Upright	¾"	Standard	Chrome	155°F	
	Generic	TY3355	Tyco DS-1	2	5.6	Sidewall	½"	Standard	Chrome	155°F	
				Total = 37							

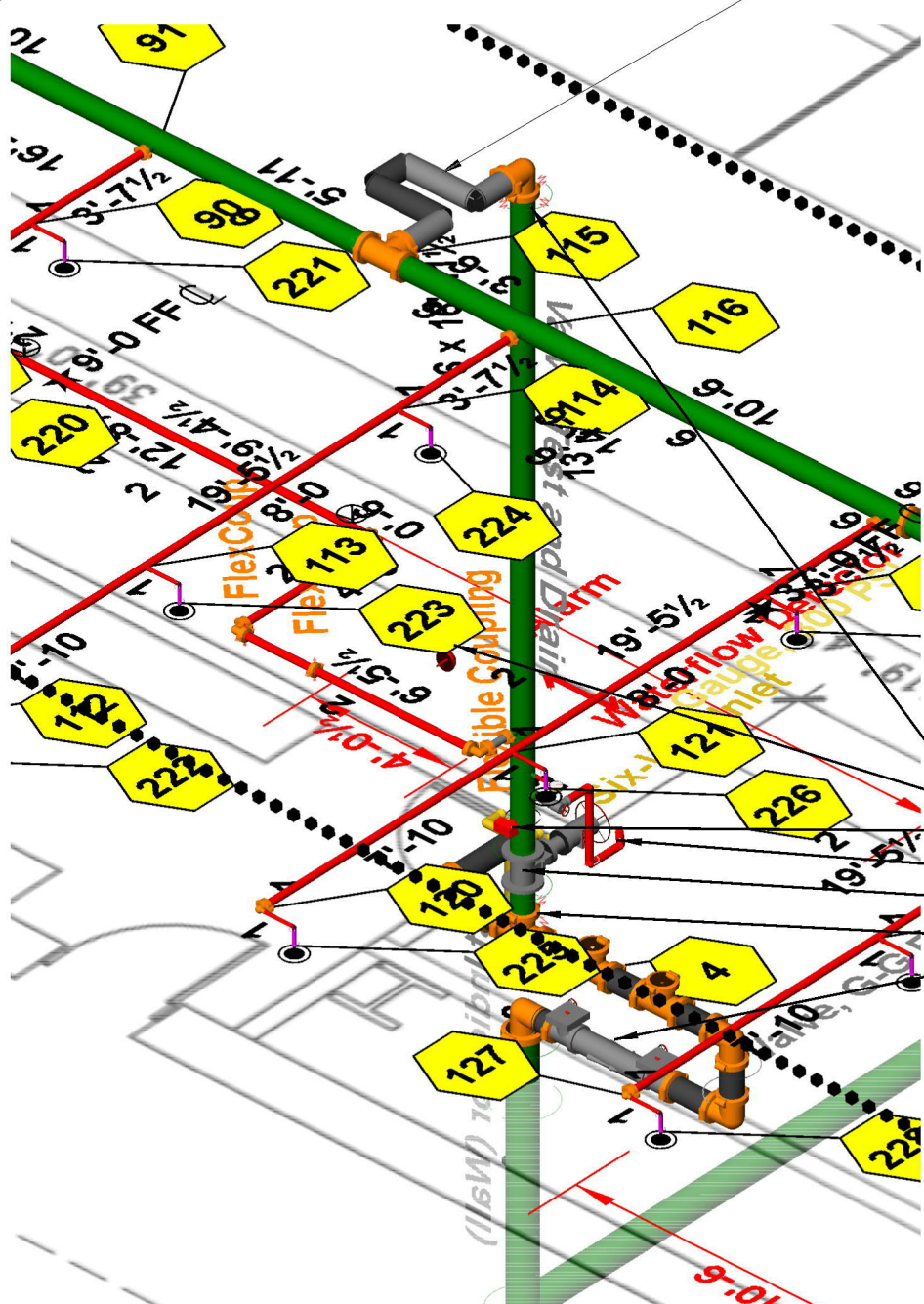
Hydraulic Information	
OCCUPANCY CLASSIFICATION	Extra Hazard Group II
DENSITY (gpm/ft²)	0.40 for 2500ft² (Actual 2513ft²)
TOTAL HOSE STREAMS	500.00
TOTAL HEADS FLOWING	32
BASE OF RISER (gpm)	1606.12
BASE OF RISER (psi)	34.966
SAFETY MARGIN (psi)	+10.298 (22.8%)



4 SECTION VIEW-REFERENCE ONLY  
1/8"= 1'-0"



3 EXISTING FIRE ALARM PANEL FOR REFERENCE  
1/8"= 1'-0"



2 SYSTEM RISER PLAN- ISO VIEW  
1/8"= 1'-0"

SUPPORT LINES ON ROOF OF PAINT BOOTH STRUCTURE USING UNISTRUT. NO LATERAL/LONGITUDINAL BRACING REQUIRED WHEN USING UNISTRUT HANGER MOUNTS.

NO SPRINKLER PROTECTION IN EXISTING BLDG. PENDING AHJ APPROVALS

1 PAINT BOOTH- FIRE SPRINKLER PLAN SHEET REFERENCE 22X34  
1/8"= 1'-0"

REV	DATE	Description
△		
△		
△		
△	11/15/20	SUBMITTAL PLAN
△	10/29/20	PERMIT SET



Designed By:  
CBPE  
Drawn By:  
CBPE  
Checked By:  
CBPE  
Approved By:  
CBPE

UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
FIRE SPRINKLER PLAN - PAINTBOOTH

Scale:  
1/8"=1'-0"  
CADD Filename:  
1012 UTA Warm Springs Paint Booth.cad  
Submit Date:  
NOV 15, 2020  
UTA Project No.:  
SGR-358  
Drawing No.:  
CBPE#1012  
Sheet No.:  
F100



Scale:		1/8"=1'-0"
CADD Filename:		
1012 UTA Warm Springs Paint Booth.cad		
Submittal Date		
NOV 15, 2020		
UTA Project No.:		
SGR-358		
Drawing No.:	Sheet No.:	
CBPE#1012	F101	



C

B

A

MECHANICAL LEGEND											
SYMBOL		ABR.		DESCRIPTION		SYMBOL		ABR.		DESCRIPTION	
GENERAL TERMINOLOGY				AIR SIDE				WET SIDE			
				DETAIL NUMBER DESIGNATION CORRESPONDING WITH GRID LOCATION						EXISTING AIR DUCT TO BE REMOVED	
				MECHANICAL EQUIPMENT DESIGNATION EQUIPMENT ITEM DESIGNATION						EXISTING AIR DUCT TO REMAIN	
				REGISTER, GRILL OR DIFFUSER DESIGNATION WITH BALANCING CFM LISTED BELOW						NEW AIR DUCT	
				GRILLE, OR LOUVER DESIGNATION WHERE BALANCING NOT REQUIRE						NEW SPIRAL DUCT	
				REVISION DESIGNATOR AND NUMBER						NEW MEDIUM PRESSURE DUCT	
				KEY NOTE DESIGNATOR AND NUMBER						BURIED OR UNDER FLOOR DUCT	
		POC		POINT OF CONNECTION						FLEXIBLE AIR DUCT	
		POR		POINT OF REMOVAL						LINED DUCT	
GC				GENERAL CONTRACTOR						VANED ELBOW	
MC				MECHANICAL CONTRACTOR						RADIUS ELBOW	
ATC				CONTROL CONTRACTOR						FLEXIBLE AIR DUCT CONNECTION	
EC				ELECTRICAL CONTRACTOR						VOLUME DAMPER	
FPC				FIRE PROTECTION CONTROL						SUPPLY AIR DIFFUSER	
NIC				NOT IN CONTRACT						RETURN AIR, FRESH AIR, AND TRANSFER AIR	
NTS				NOT TO SCALE						CEILING MOUNTED EXHAUST FAN OR EXHAUST GRILLE	
C				COMMON						RETURN OR OUTSIDE AIR DUCT UP	
NC				NORMALLY CLOSED						SUPPLY DUCT UP	
NO				NORMALLY OPEN						EXHAUST AIR INTAKE UP	
										RETURN OR OUTSIDE AIR DUCT DOWN	
										SUPPLY DUCT DOWN	
										EXHAUST DUCT DOWN	
										ROUND DUCT UP	
										ROUND DUCT DOWN	
								AP		ACCESS PANEL	
										EXISTING EQUIPMENT TO BE REMOVED	
										EXISTING EQUIPMENT TO REMAIN	
										NEW EQUIPMENT	
						RTU-1		T-STAT		WALL MOUNTED THERMOSTAT MECHANICAL EQUIPMENT CONTROLLED	
						SA				SUPPLY AIR	
						RA				RETURN AIR	
						EA				EXHAUST AIR	
						OA				OUTSIDE AIR	
						MA				MIXED AIR	
						FA				FRESH AIR	
						RF				RELIEF AIR	

GENERAL NOTES:

**G-1** MECHANICAL INFORMATION IS NOT LIMITED TO THE MECHANICAL DRAWINGS. CONTRACTOR SHALL BE RESPONSIBLE FOR INFORMATION OF THE EXISTING BUILDING AND SITE CONDITIONS, EXISTING PIPING, EXISTING ELECTRICAL, AND EXISTING SUPPORTS.

A - EACH DRAWING SHEET AND THE SPECIFICATIONS HAVE BEEN PREPARED TO SUPPLEMENT EACH OTHER AND THEY SHALL BE INTERPRETED AS AN INTEGRAL UNIT WITH ITEMS SHOWN AND NOTED ON ONE AND NOT THE OTHER BEING FURNISHED AND INSTALLED AS THOUGH SHOWN AND CALLED OUT IN ALL PLACES. ITEMS IN SPECIFICATIONS OR DRAWINGS LISTED WHICH ARE DIFFERING IN EFFICIENCY OR QUALITY SHALL BE HELD TO THE GREATEST OF: EFFICIENCY, QUALITY OR GOVERNING CODE.

B - THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR THE INSTALLATION OF THE SYSTEMS ACCORDING TO THE TRUE INTENT AND MEANING OF THE CONTRACT DOCUMENTS.

C - THE CONTRACTOR SHALL INSTALL ALL EQUIPMENT WITH PROPER SERVICE ACCESS AND CLEARANCES ACCORDING TO MANUFACTURERS RECOMMENDATIONS. THE CONTRACTOR SHALL REVIEW SUPPLIERS BID PACKAGES FOR COMPLETENESS AND COMPLIANCE TO THE SPECIFICATIONS, SCHEDULES, AND DESIGN INTENT (ALL EQUIPMENT AND METHODS). THE CONTRACTOR SHALL REMOVE AND REINSTALL CORRECTLY AT HIS OWN EXPENSE ANY EQUIPMENT NOT IN COMPLIANCE.

D - THE CONTRACTOR SHALL CONSULT MANUFACTURERS INSTALLATION INSTRUCTIONS FOR SIZES, METHODS, ACCESSORIES, AND CLEARANCES IN SPACE AVAILABLE PRIOR TO BIDDING PROJECT.

E - ANYTHING NOT CLEAR OR IN CONFLICT WILL BE EXPLAINED BY MAKING APPLICATION TO THE ENGINEER IN WRITING.

**G-2** ANY AND ALL ALTERATIONS TO THE SYSTEM SHOWN SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO CHANGES FOR APPROVAL. CONTRACTOR SHALL NOT START ANY CHANGES UNTIL NOTIFIED IN WRITING. IF CHANGES ARE MADE PRIOR TO APPROVAL CONTRACTOR SHALL TAKE ALL RESPONSIBILITY FOR THE CHANGES MADE AND ALL COSTS RELATING TO FAILURE OR REPLACEMENT OF ALTERATIONS.

**G-3** CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS AND LOCATIONS.

**G-4** THE WORKING DRAWINGS ARE DIAGRAMMATIC. THEY DO NOT SHOW EVERY OFFSET, BEND, OR ELBOW NECESSARY FOR THE COMPLETE INSTALLATION IN THE SPACE PROVIDED. ALL LOCATIONS FOR MECHANICAL EQUIPMENT SHALL BE FIELD VERIFIED AND COORDINATED WITH ALL DRAWINGS. THE CONTRACTOR SHALL PROVIDE OR COORDINATE WITH THE GENERAL CONTRACTOR PROVISIONS FOR BLOCKOUTS OR CORE DRILLS THROUGH STRUCTURE.

**G-5** THE INSTRUCTION TO "PROVIDE" ALSO INCLUDES INSTALLATION.

**G-6** MECHANICAL CONTRACTOR SHALL PROVIDE AND INSTALL SMOKE AND FIRE DAMPERS AS REQUIRED BY LOCAL CODES AND AUTHORITIES.

**G-7** SHEET METAL DUCT SIZES SHOWN ON DRAWINGS ARE FREE AREA DIMENSIONS.

**G-8** PROVIDE AND INSTALL BALANCING DAMPERS IN ALL SUPPLY AND EXHAUST AIR BRANCH DUCTS. BALANCE TO CFM SHOWN ON PLAN.

**G-9** SEE ARCHITECTURAL REFLECTED CEILING PLAN FOR EXACT LOCATION OF DIFFUSERS AND GRILLES.

**G-10** PROVIDE TURNING VANES IN ALL ELBOWS OF RECTANGULAR DUCT.

**G-11** THE CONTRACTOR SHALL ASSUME ALL RESPONSIBILITY IN HANDLING AND DISPOSING OF REFRIGERANTS, OILS, ETC. ALL SUCH MATERIALS SHALL BE HANDLED, DISPOSED, AND USED IN ACCORDANCE WITH ALL LOCAL, STATE, AND FEDERAL LAWS.

**G-12** THE MECHANICAL CONTRACTOR SHALL VERIFY MOTOR VOLTAGES WITH THE ELECTRICAL DRAWING BEFORE ORDERING MOTORIZED EQUIPMENT AND CONTROLS.

**G-13** C.F.M. LISTED IS ACTUAL AIR.

**G-14** SUPPLIERS SHALL REVIEW ALL DRAWINGS AND THE SPECIFICATIONS PRIOR TO SUBMITTING PRICES TO THE CONTRACTOR. ALL QUESTIONS AND DISCREPANCIES SHALL BE BROUGHT TO THE ENGINEERS ATTENTION PRIOR TO BIDDING.

**G-15** CONTRACTOR SHALL THOROUGHLY REVIEW AND SIGN SUBMITTALS FOR COMPLETENESS AND COMPLIANCE TO THE SPECIFICATIONS PRIOR TO ENGINEERS REVIEW. SUPPLIERS SHALL HIGHLIGHT OR MARK ALL INFORMATION REQUIRED TO SHOW COMPLIANCE TO THE SPECIFICATIONS. ALL REQUESTED EXCEPTIONS TO THE SPECIFICATIONS, OR SCHEDULES SHALL BE CLEARLY NOTED AND EXPLAINED. SUBMITTAL REVIEW AND ACCEPTANCE IS FOR DESIGN CONCEPT ONLY, AND DOES NOT AT ANY TIME RELIEVE THE CONTRACTOR OF RESPONSIBILITY TO MEET SPECIFICATIONS, CAPACITIES, OR DESIGN INTENT.

**G-16** ALL MECHANICAL SHALL BE INSTALLED AND CONFORM TO THE 2018 EDITION OF THE IMC AND IPC WITH UTAH ANNOTATIONS AND LOCAL AUTHORITY REQUIREMENTS.

**G-17** THIS CONTRACTOR SHALL BE RESPONSIBLE FOR THE DRAINING DOWN AND RE-FILLING OF ALL SYSTEMS NECESSARY TO COMPLETE THE WORK OUTLINED BY THIS PROJECT. THIS INCLUDES PROVIDING THE REQUIRED CHEMICAL TREATMENT WHEN RE-FILLING THE SYSTEM.

**G-18** THIS CONTRACTOR SHALL CONTRACT WITH A DESIGN BUILD ELECTRICAL CONTRACTOR FOR THE DESIGN AND CONSTRUCTION OF THE ELECTRICAL PORTION OF THIS PROJECT. ELECTRICAL INSTALLATION AND DESIGN SHALL BE PER 2017 NEC.

**G-19** ALL PIPING, MATERIALS, ETC. SHALL BE NEW AND DOMESTIC MADE UNLESS SPECIFICALLY AUTHORIZED IN WRITING PRIOR TO BID.

**G-20** PROVIDE SPRINKLER MODIFICATIONS PER PERFORMANCE SPECIFICATION THROUGH DESIGN BUILD NICET LEVEL 4 CERTIFIED FIRE SPRINKLER CONTRACTOR.

12/7/20

PERMIT SET

REV

DATE

Description

architecture • planning • design services

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P:(801) 961-7070  
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Submitted By: \_\_\_\_\_

UTAH TRANSIT AUTHORITY

Approved By: \_\_\_\_\_

Designed By:  
**B. LASH**

Drawn By:  
**STAFF**

Checked By:  
**B. LASH**

Approved By:  
**B. LASH**

UTAH TRANSIT AUTHORITY  
FRONT RUNNER PAINT BOOTH  
WARM SPRINGS SERVICE CENTER  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
  
MECHANICAL LEGEND AND GENERAL NOTES

Scale:  
**1/8"=1'-0"**

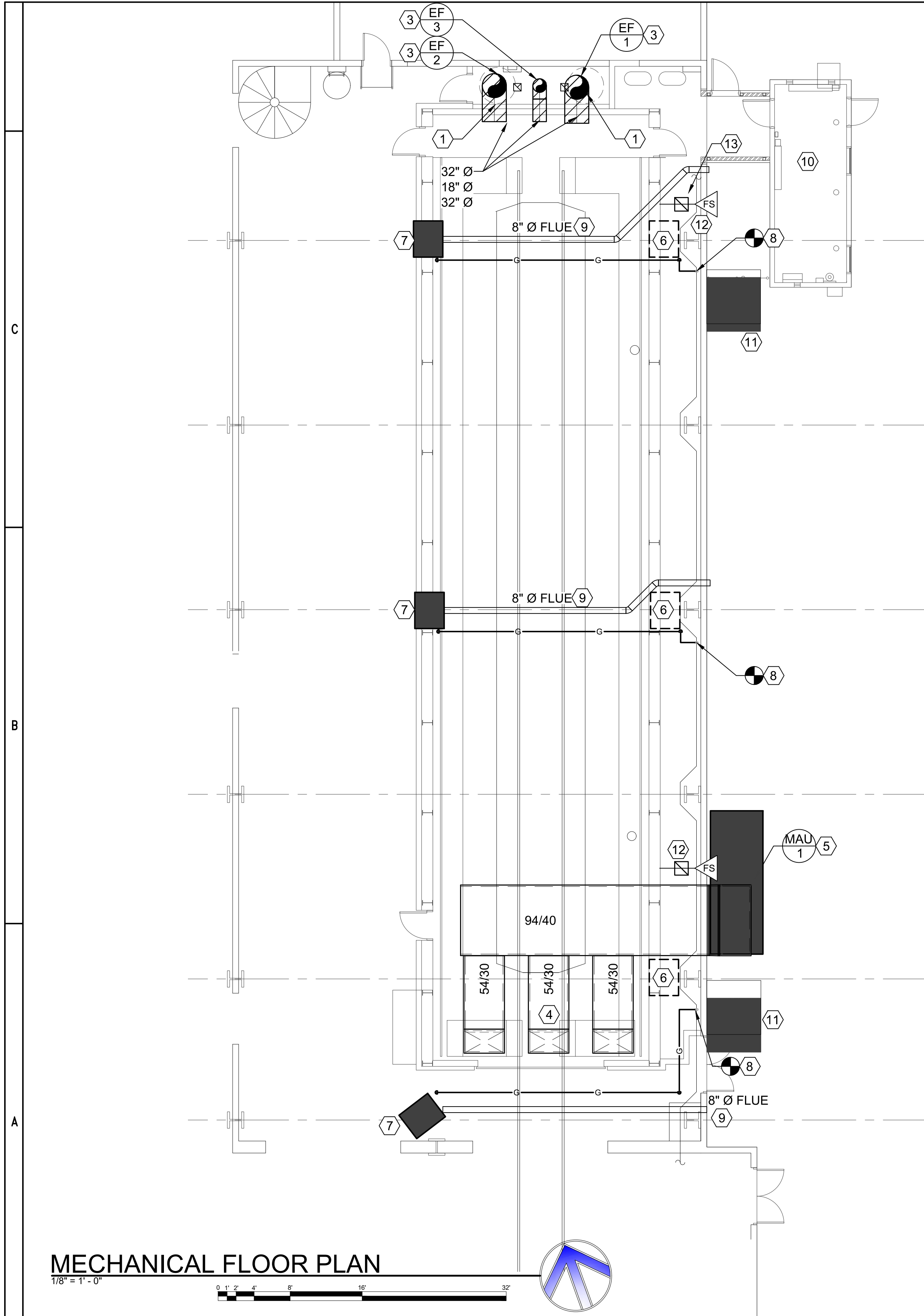
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**M20091 WARM SPRINGS PAINT BOOTH.DWG**

Submittal Date  
**12/7/20**

UTA Contract No.:  
**SGR-358**

Drawing No.:  
**MG001**



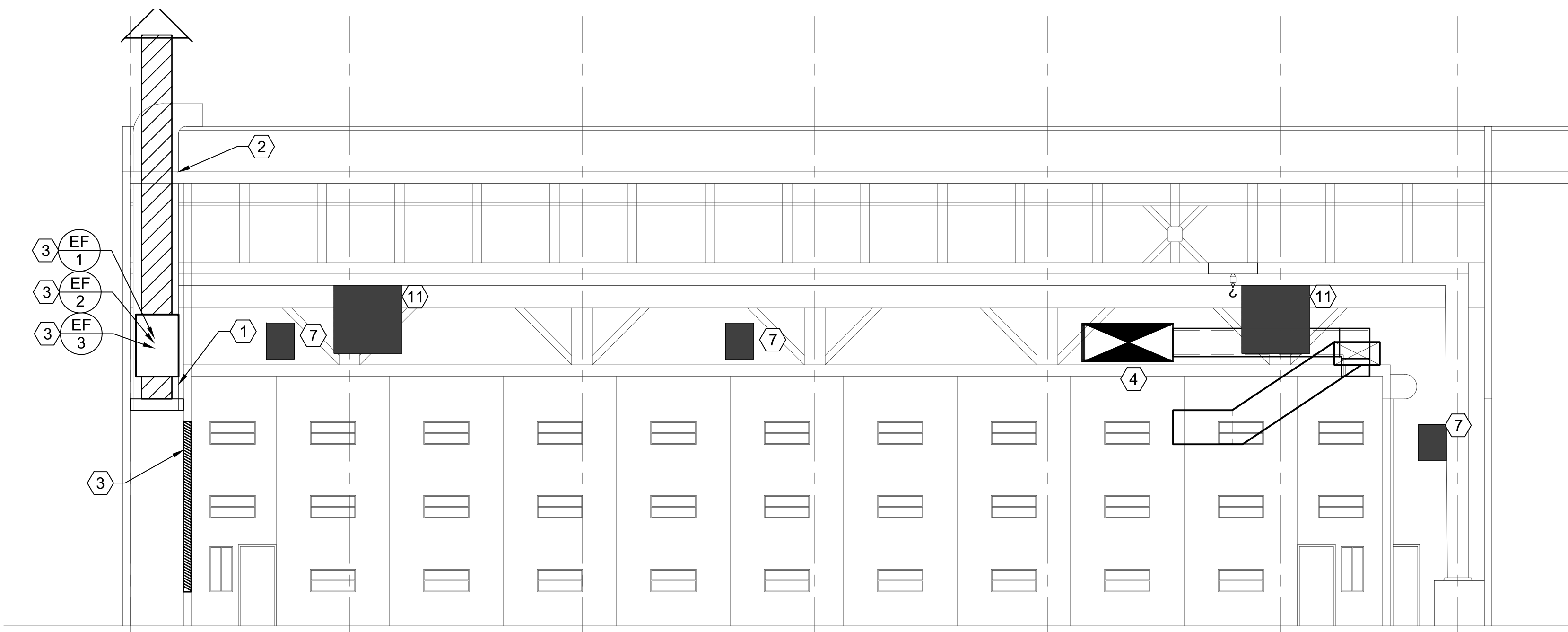


SHEET NOTES:

- EXHAUST FANS TO BE CONNECTED TO SPIRAL DUCT WITH CLEAN OUT DOOR TOWARDS THE BOTTOM WHERE THEY CAN BE EASILY ACCESSED. ROUTE UP THROUGH THE ROOF AND TERMINATE WITH MANUFACTURE RECOMMENDED WEATHER CAP AND SCREEN. COORDINATE HEIGHT OF THE STACKS WITH PAINT BOOTH MANUFACTURER AND CODE REQUIREMENTS.
- CAULK JOINT BETWEEN ROOF FLANGE COLLAR AND DUCT.
- EXHAUST AIR LOUVERS AND FILTERS. SHALL BE LAID OUT AND DESIGNED BY PAINT BOOTH MANUFACTURER USING BEST PRACTICES AND INDUSTRY STANDARDS. FILTERS SHALL BE STANDARD SIZE. EXTEND VENTS ABOVE THE ROOF AND TERMINATE WITH MANUFACTURER APPROVED WEATHER CAP AND SCREEN. PROVIDE GUY WIRES FOR STACK OVER 48" ABOVE THE ROOF.
- MAKE UP AIR LOUVERS AND FILTERS SHALL COME AS PART OF THE PAINT BOOTH PACKAGE. SHALL BE LAID OUT AND DESIGNED BY PAINT BOOTH MANUFACTURER USING BEST PRACTICES AND INDUSTRY STANDARDS. FILTERS SHALL BE STANDARD SIZE.
- PROVIDE GROUND MOUNTED VERTICAL MAKEUP AIR UNIT. DUCT LAYOUT AND DESIGN SHALL BE PERFORMED BY THE PAINT BOOTH MANUFACTURER. SHALL BE NATURAL GAS HEAT. ROUTE DUCT INTO THE PAINT BOOTH PER INDUSTRY STANDARDS AND BEST PRACTICES. COORDINATE LOCATION AND HEIGHTS WITH EXISTING CRANE ABOVE THE PAINT BOOTH. COORDINATE WITH ARCHITECT FOR BOLLARDS OR OTHER PROTECTION FOR THE EQUIPMENT. MAINTAIN MANUFACTURER RECOMMENDED CLEARANCES.
- REMOVE AND RELOCATE EXISTING GAS FIRED UNIT HEATER FROM EXISTING LOCATION TO THIS THIS APPROXIMATE LOCATION. REMOVE ASSOCIATED HANGERS AND FLU PIPING. PATCH EXTERIOR WALL WEATHER TIGHT. REMOVE EXISTING GAS PIPING BACK TO MAIN AND ROUTE NEW PIPING PER PLANS.
- RELOCATE EXISTING GAS FIRED UNIT HEATER TO THIS THIS APPROXIMATE LOCATION. ROUTE NEW FLUE AND GAS PIPING PER PLANS.
- PROVIDE NEW NATURAL GAS PIPING FROM EXISTING MAIN TO NEW PLACEMENT OF EXISTING GAS FIRED UNIT HEATER. CONNECT PER DETAIL.
- PROVIDE FLUE PER MANUFACTURERS RECOMMENDATION IN THIS APPROXIMATE LOCATION. FIELD VERIFY.
- PROVIDE HEATING AND COOLING FOR THE MIXING BOOTH AS OUTLINED IN THE PAINT BOOTH SPECIFICATION.
- RELOCATE THE EXISTING SWAMP COOLERS ON THE OUTSIDE OF THE BUILDING TO BE ABOVE THE NEW PAINT BOOTH. EXTEND THE EXISTING WATER LINE UP. FIELD VERIFY. REUSE EXISTING WALL SUPPORTS.
- PROVIDE 18X18 OPENING IN THE TOP OF THE 1 HOUR RATED ASSEMBLY IN THIS APPROXIMATE LOCATION TO ALLOW AIR CIRCULATION IN THE INTERSTITIAL SPACE BETWEEN WALL AND BOOTH. PROVIDE FIRE/SMOKE DAMPER ON OPENING. FIELD VERIFY.
- EXISTING EMERGENCY SHOWER AND EYEWASH IN THIS APPROXIMATE LOCATION SHALL REMAIN.

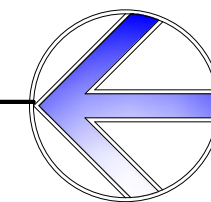
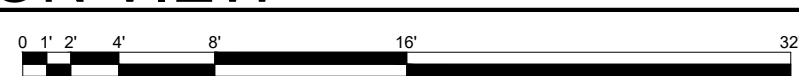
GENERAL NOTES:

- THE INTENT IS TO HAVE THESE DOCUMENTS PROVIDE ENOUGH INFORMATION SO THAT THE PAINT BOOTH CAN BE COMPETITIVELY BID. THE MAKEUP AIR UNIT, EXHAUST FANS, CONTROL PANEL, ETC SHALL BE SUPPLIED BY THE PAINT BOOTH SUPPLIER AND SHALL INTEGRATE INTO THE SYSTEM AND MEET ALL LOCAL AND NATIONAL CODES. THE BOOTH SHALL MAINTAIN 100 FPM AIRFLOW DOWN THE TUNNEL AND BE CONFIGURED AS A CROSS DRAFT LAYOUT.



WEST SECTION VIEW

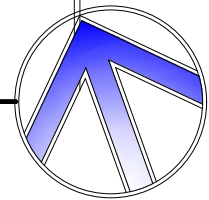
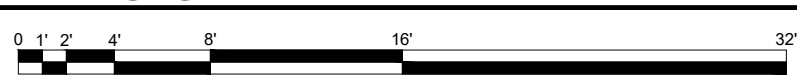
1/8" = 1' - 0"



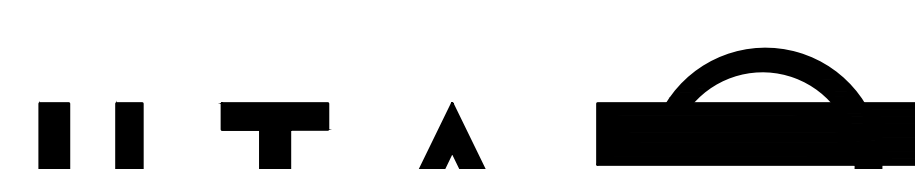


**WHW**  
**ENGINEERING LLC.**  
PROFESSIONAL MECHANICAL ENGINEERING  
8619 Sandy Parkway Suite 101  
SANDY, UTAH 84070  
(801) 465-4021 FAX 465-8536  
EMAIL: excellence@whw-engineering.com

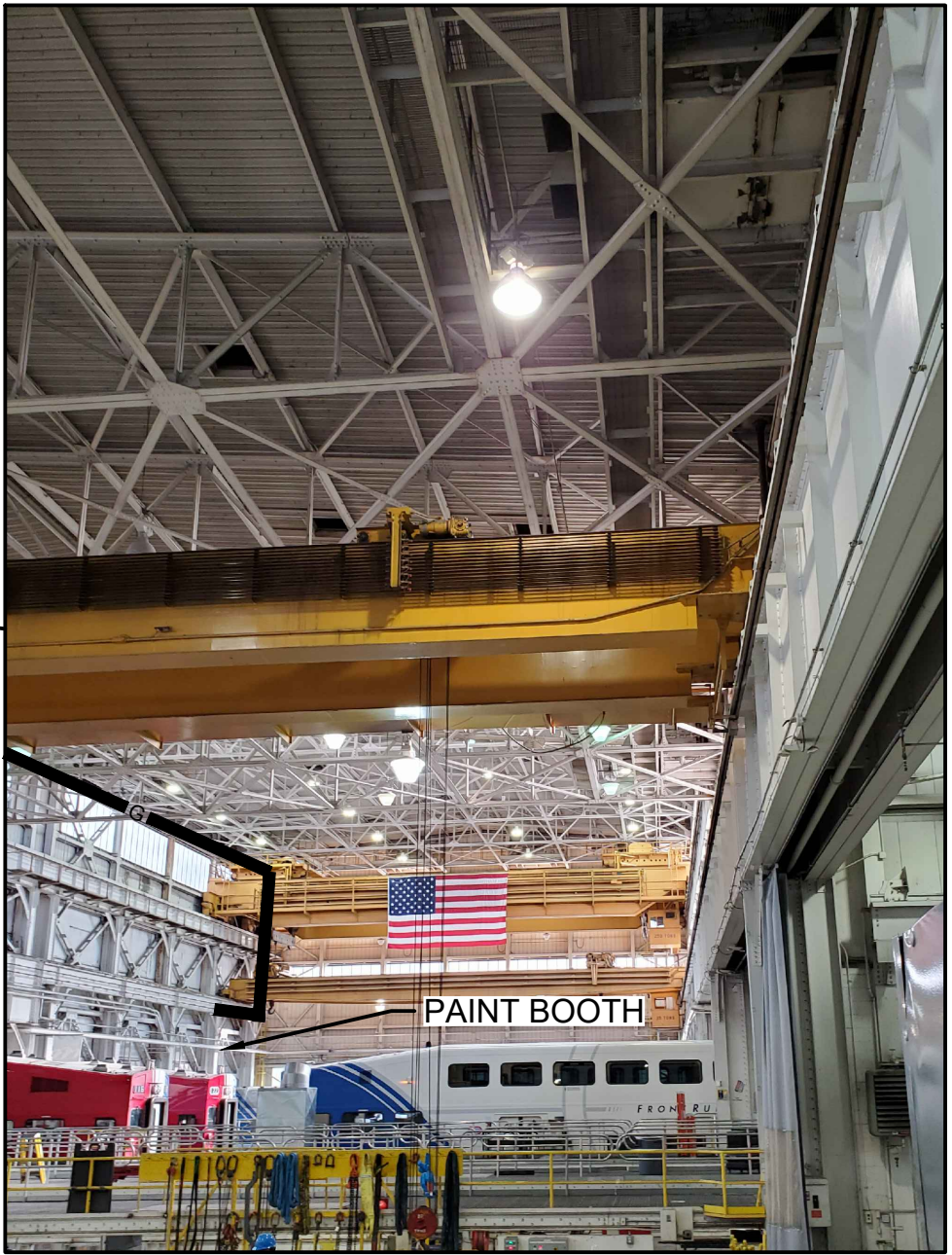
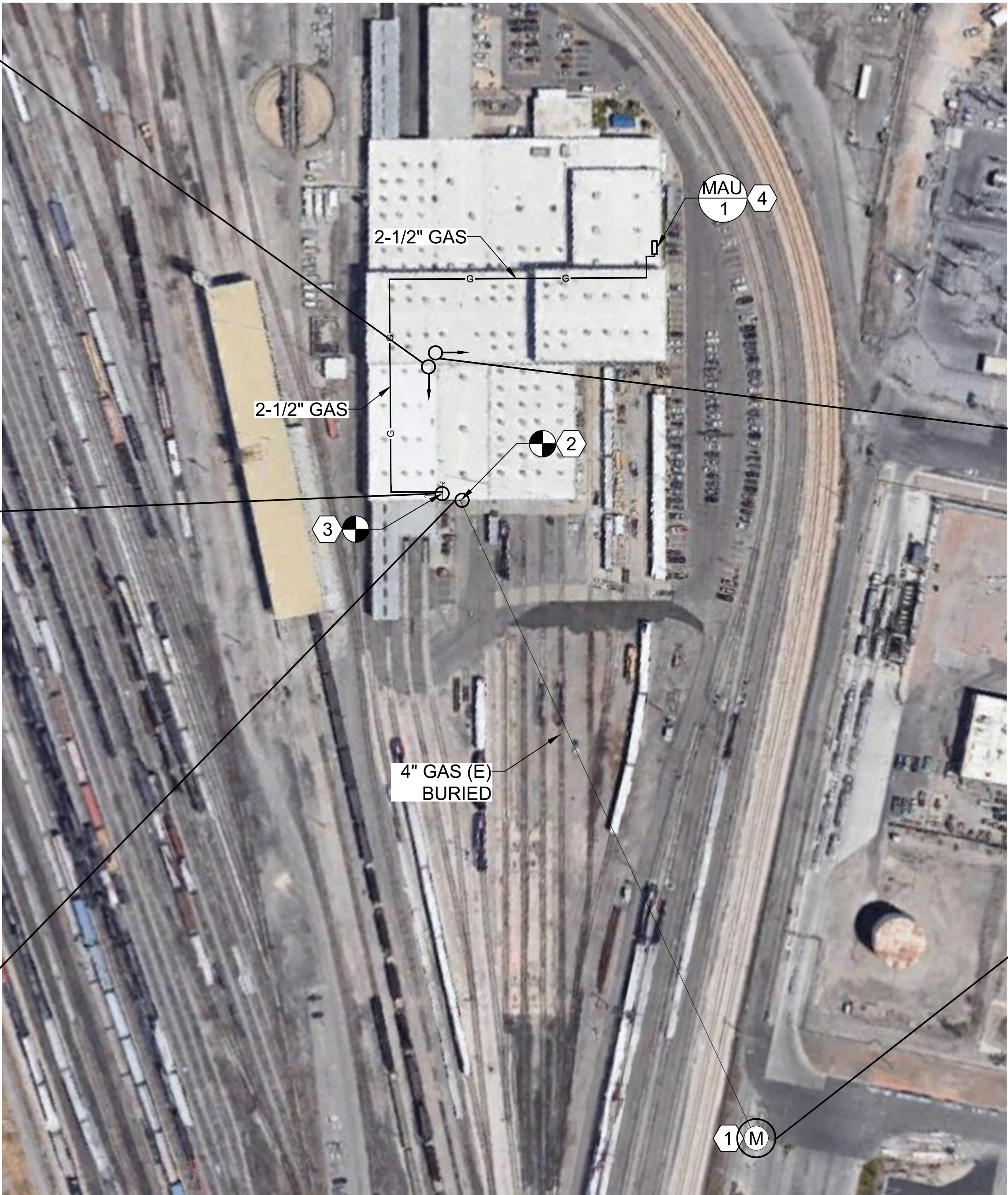
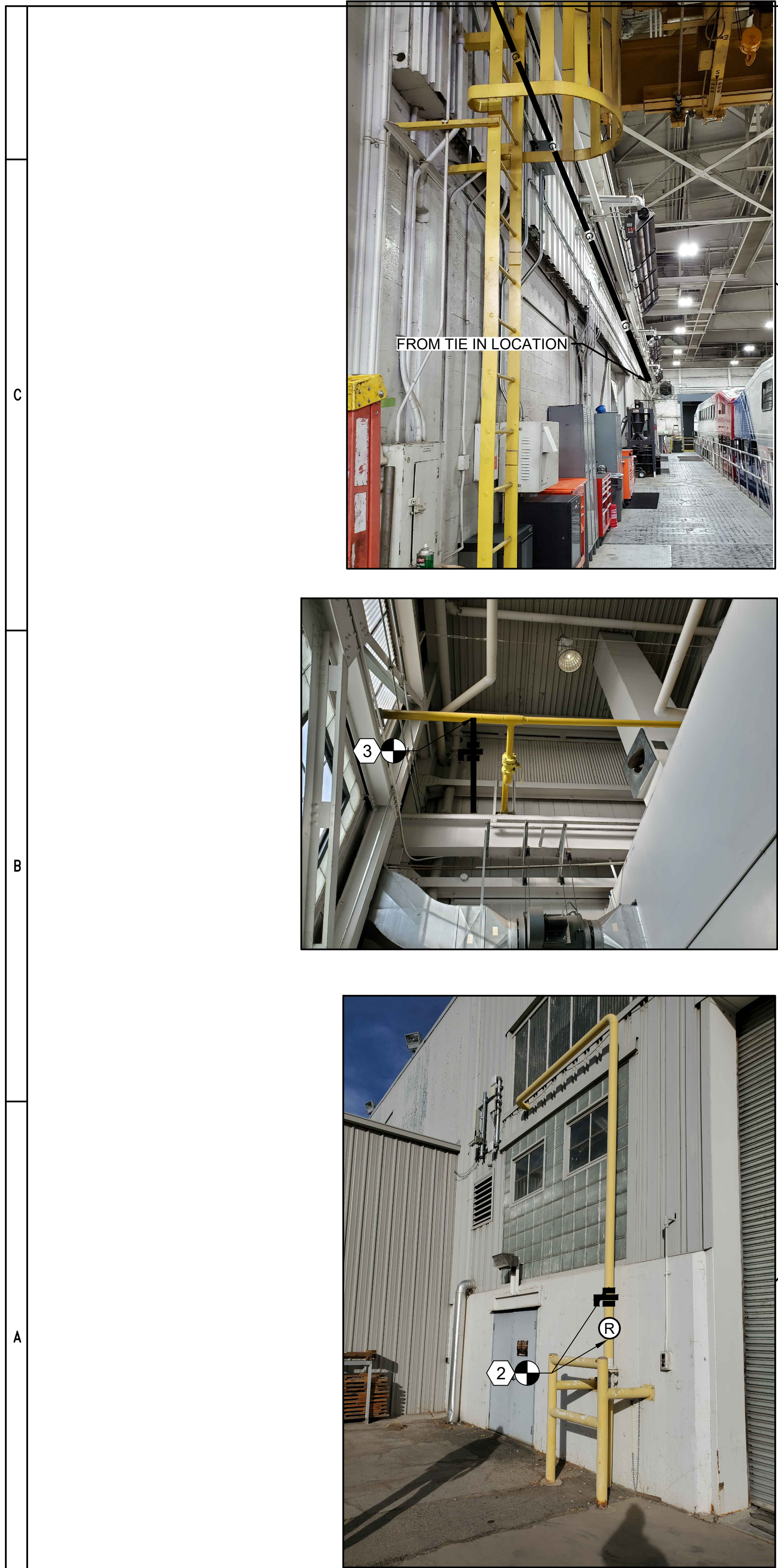
MECHANICAL FLOOR PLAN

1/8" = 1' - 0"



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<div>△</div>			<div></div>	<div><div>architecture • planning • design services</div><div>255 Crossroad Square Salt Lake City, UT 84115 P:(801) 961-7070 F:(801) 961-7373</div></div>	<div><div>UTAH TRANSIT AUTHORITY</div></div>	<div>Designed By: B. LASH</div>		<div>UTAH TRANSIT AUTHORITY FRONT RUNNER PAINT BOOTH WARM SPRINGS SERVICE CENTER  900 NORTH 500 WEST SALT LAKE CITY, UT 84116  MECHANICAL FLOOR PLANS</div>		<div>Scale: 1/8"=1'-0"</div>	
<div>△</div>						<div>CADD Filename: M20091 WARM SPRINGS PAINT BOOTH.DWG</div>					
<div>△</div>						<div>Submittal Date 12/7/20</div>					
<div>△</div>						<div>UTA Contract No.: SGR-358</div>					
<div>0</div>	12/7/20	PERMIT SET				<div>Submitted By: _____</div>				<div>Approved By: _____</div>	
REV	DATE	Description									





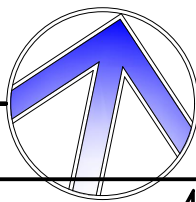
- SHEET NOTES:**
- COORDINATE WITH DOMINION ENERGY TO REPLACE EXISTING NATURAL GAS METER IN THIS APPROXIMATE LOCATION WITH A 10 PSIG NATURAL GAS METER. FIELD VERIFY. CONTRACTOR SHALL PICK UP THIS COST TO REPLACE THE METER WITH A WELDED SET. THIS HAS BEEN QUOTED BY DOMINION AS \$22,259.
  - PROVIDE 10 PSIG TO 5 PSIG REGULATOR ON EXISTING NATURAL GAS PIPING RISER IN THIS APPROXIMATE LOCATION PER DETAIL. FIELD VERIFY.
  - TIE INTO THE EXISTING 3" GAS LINE INSIDE THE BUILDING IN THIS APPROXIMATE LOCATION.
  - ROUTE NEW 5 PSIG NATURAL GAS LINE HIGH TO THE NEW MAU. TIE INTO NEW MAU IN THIS APPROXIMATE LOCATION. PROVIDE REQUIRED REGULATORS, SHUT OFF VALVES, SEDIMENT TRAPS, ETC.

- GENERAL NOTE**
- ROUTE ALL NATURAL GAS PIPING HIGH THROUGH EXISTING BAYS. COORDINATE WITH OWNER AND FIELD VERIFY BEST ROUTING TO AVOID EXISTING STRUCTURE, CRANES, ETC.
  - ALL EXPOSED NATURAL GAS PIPING AND VALVES EXCEPT COMPONENTS, WITH FACTORY-APPLIED PAINT OR PROTECTIVE COATING SHALL BE PAINTED WITH YELLOW UL PROTECTIVE PAINT.

MECHANICAL GAS PLAN

1/128" = 1' - 0"

0 8' 16' 32' 64' 128' 256'



**WHW**  
**ENGINEERING LLC.**  
PROFESSIONAL MECHANICAL ENGINEERING  
8619 Sandy Parkway Suite 101  
SANDY, UTAH 84070  
(801)465-4021, FAX 468-8536  
EMAIL: excellence@whw-engineering.com

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0	12/7/20	PERMIT SET
REV	DATE	Description



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**GROUP**  
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Submitted By: \_\_\_\_\_ Approved By: \_\_\_\_\_

Designed By:  
**B. LASH**  
Drawn By:  
**STAFF**  
Checked By:  
**B. LASH**  
Approved By:  
**B. LASH**

UTAH TRANSIT AUTHORITY  
**FRONT RUNNER PAINT BOOTH**  
**WARM SPRINGS SERVICE CENTER**  
900 NORTH 500 WEST SALT LAKE CITY, UT 84116  
**MECHANICAL NATURAL GAS PLANS**

Scale:  
**1/8"=1'-0"**  
CADD Filename:  
**M20091 WARM SPRINGS PAINT BOOTH.DWG**  
Submittal Date  
**12/7/20**  
UTA Contract No.:  
**SGR-358**  
Drawing No.: \_\_\_\_\_ Sheet No.:  
**ME102**











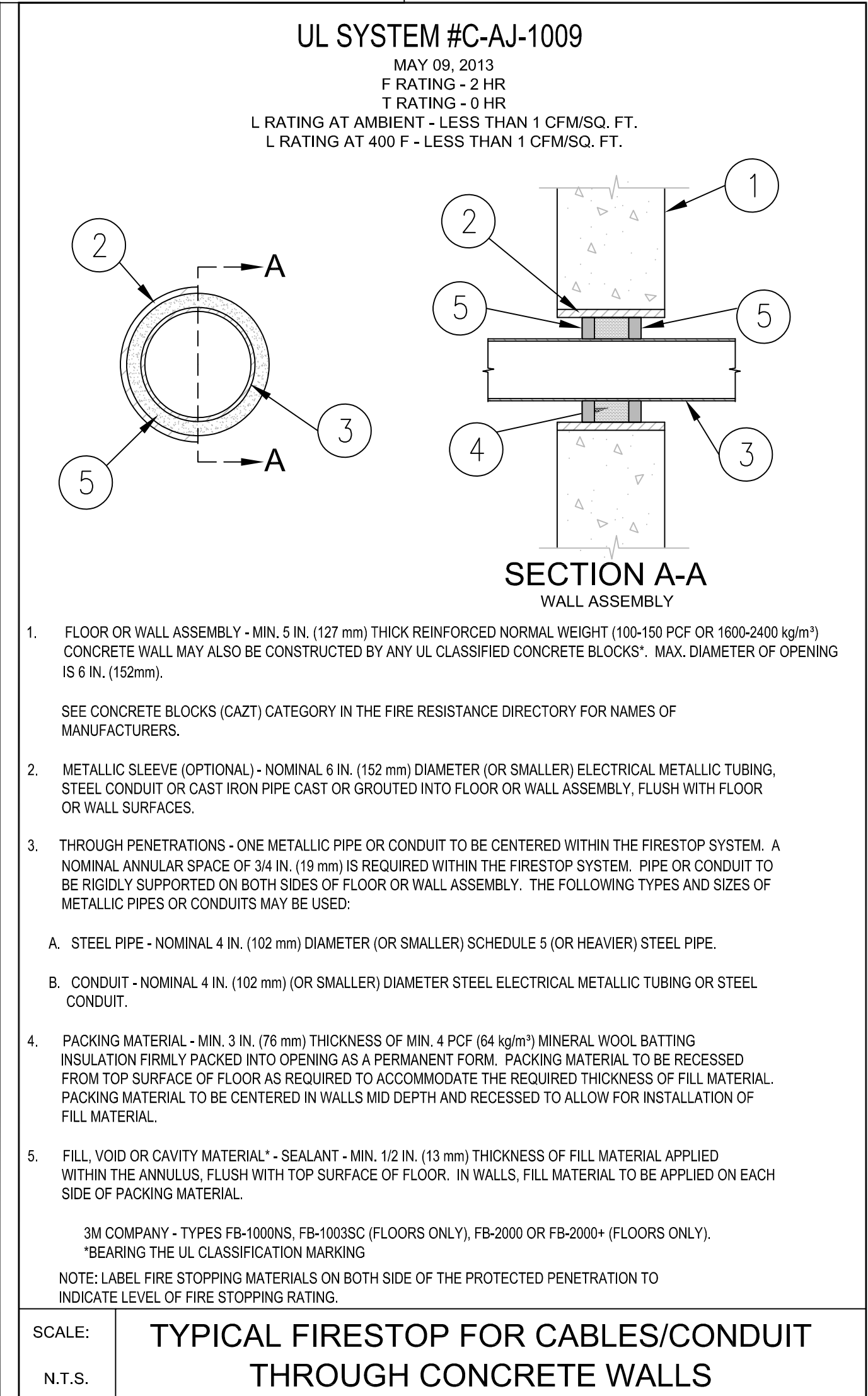
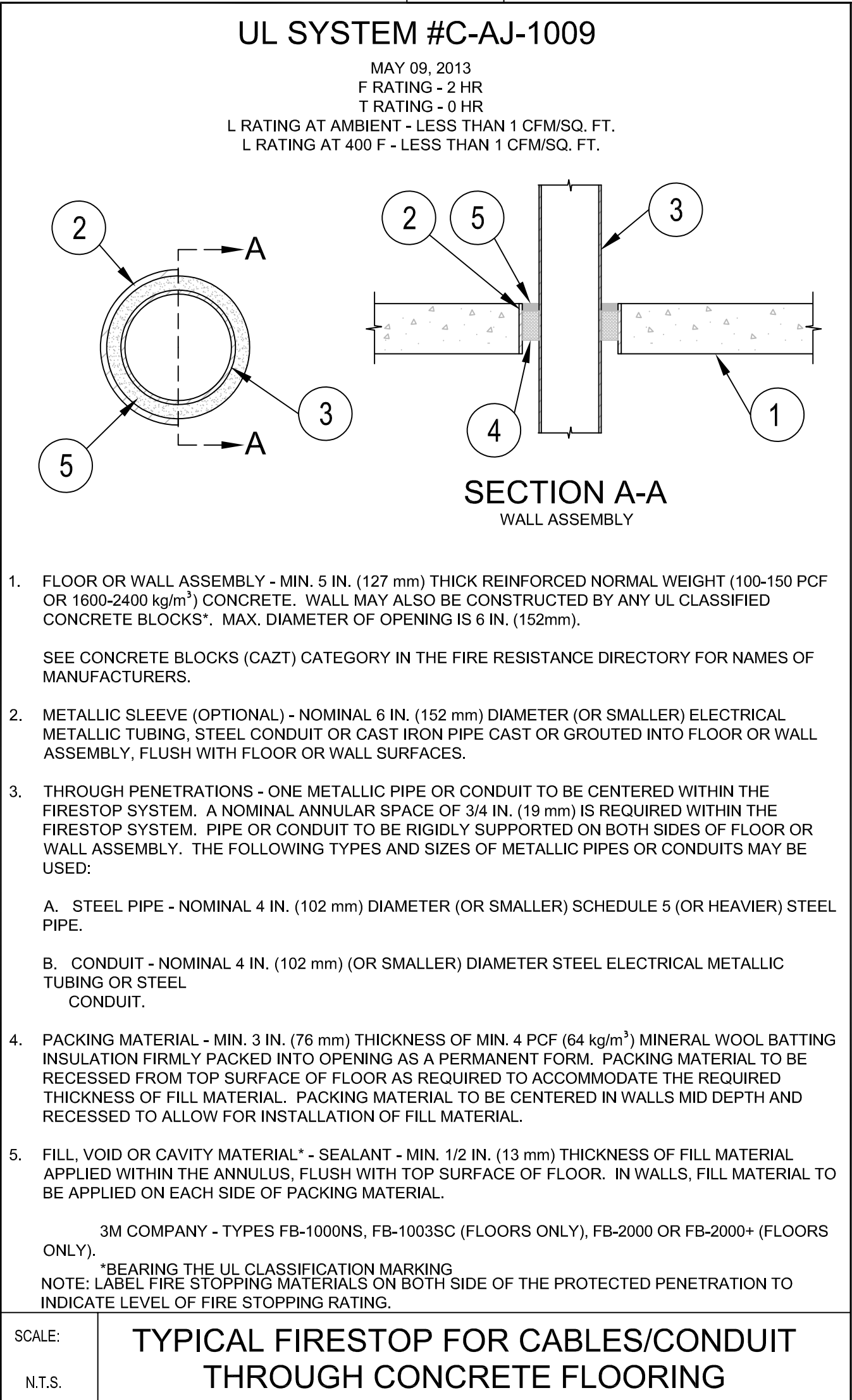
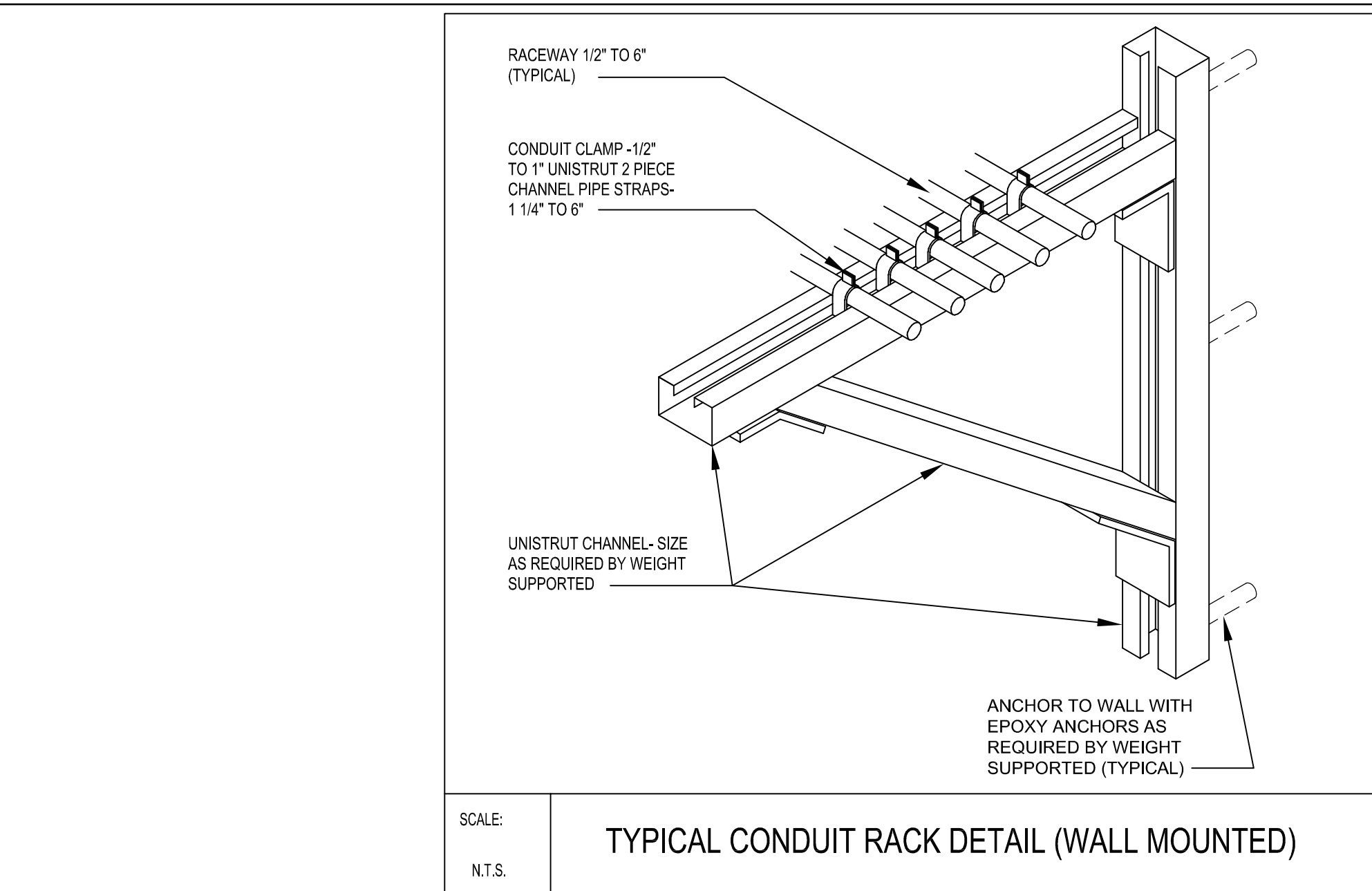
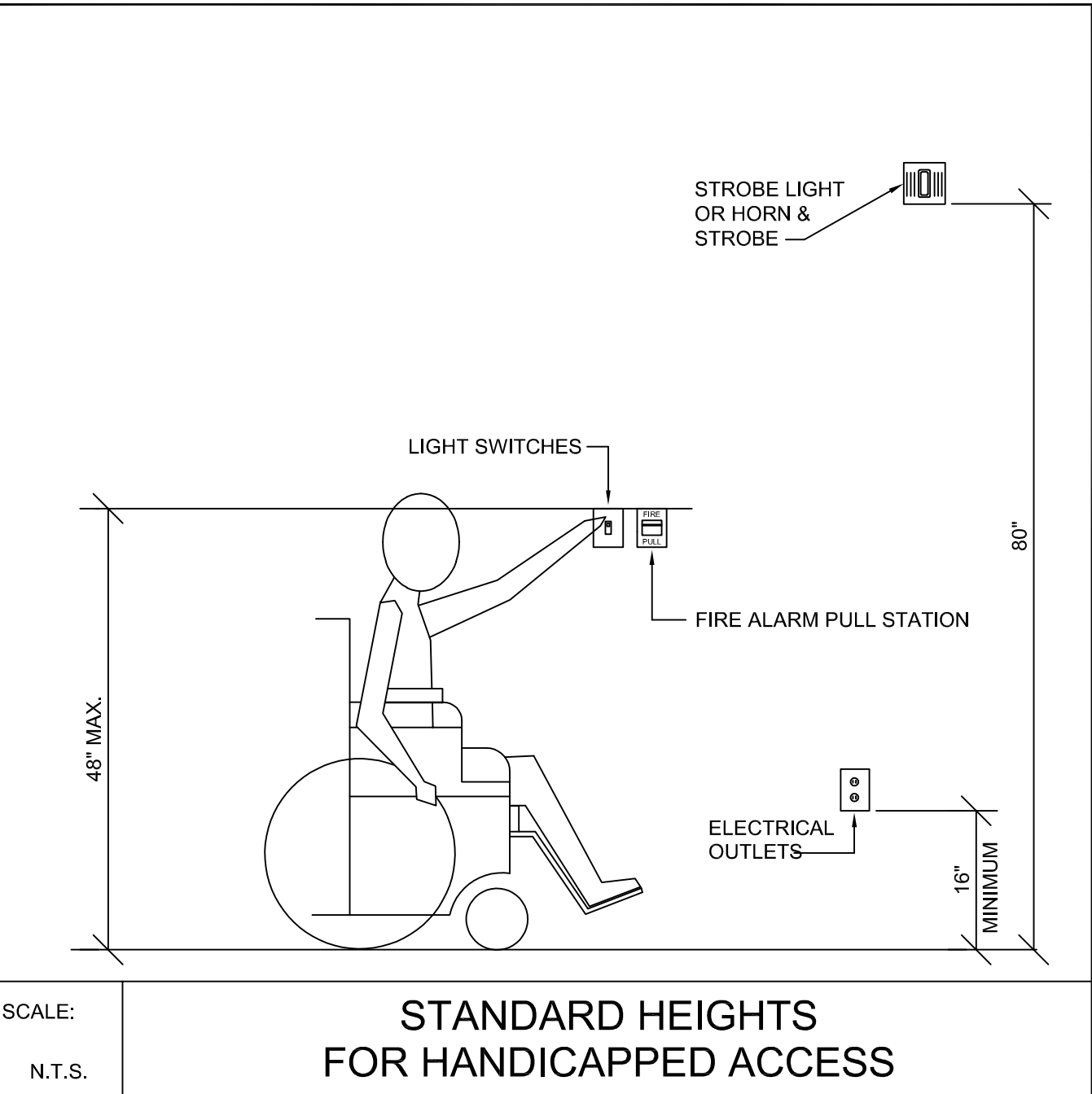
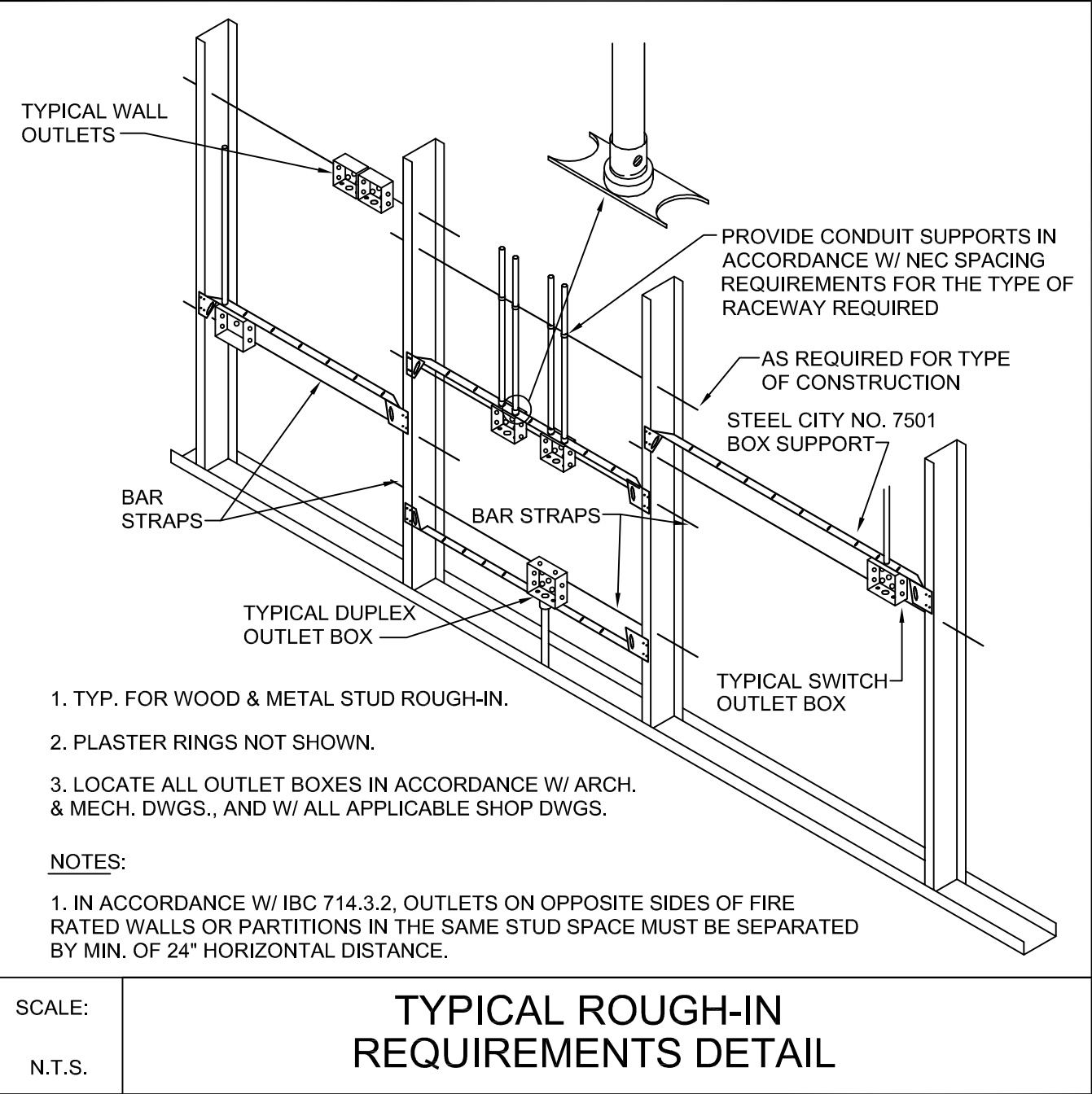
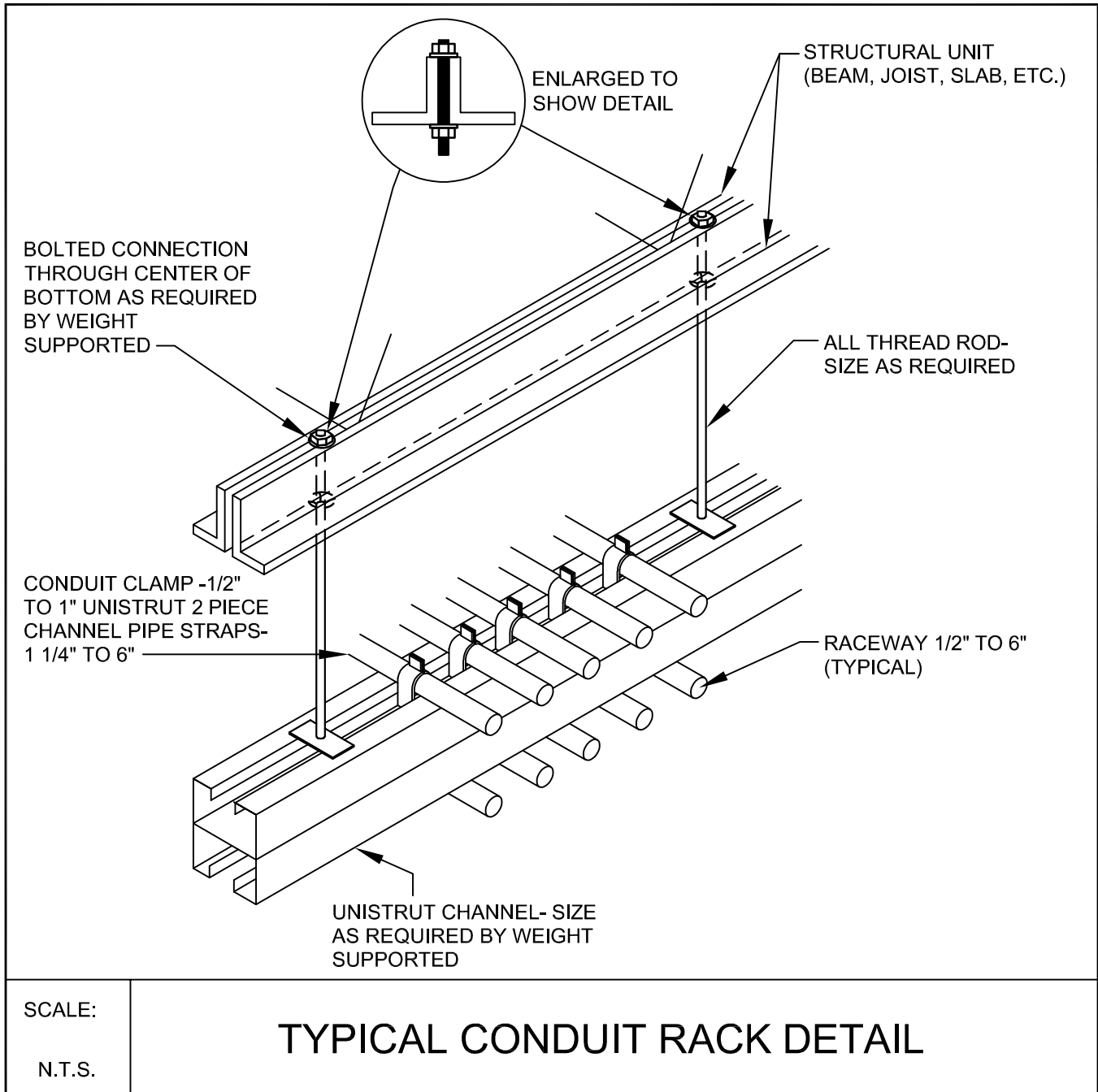
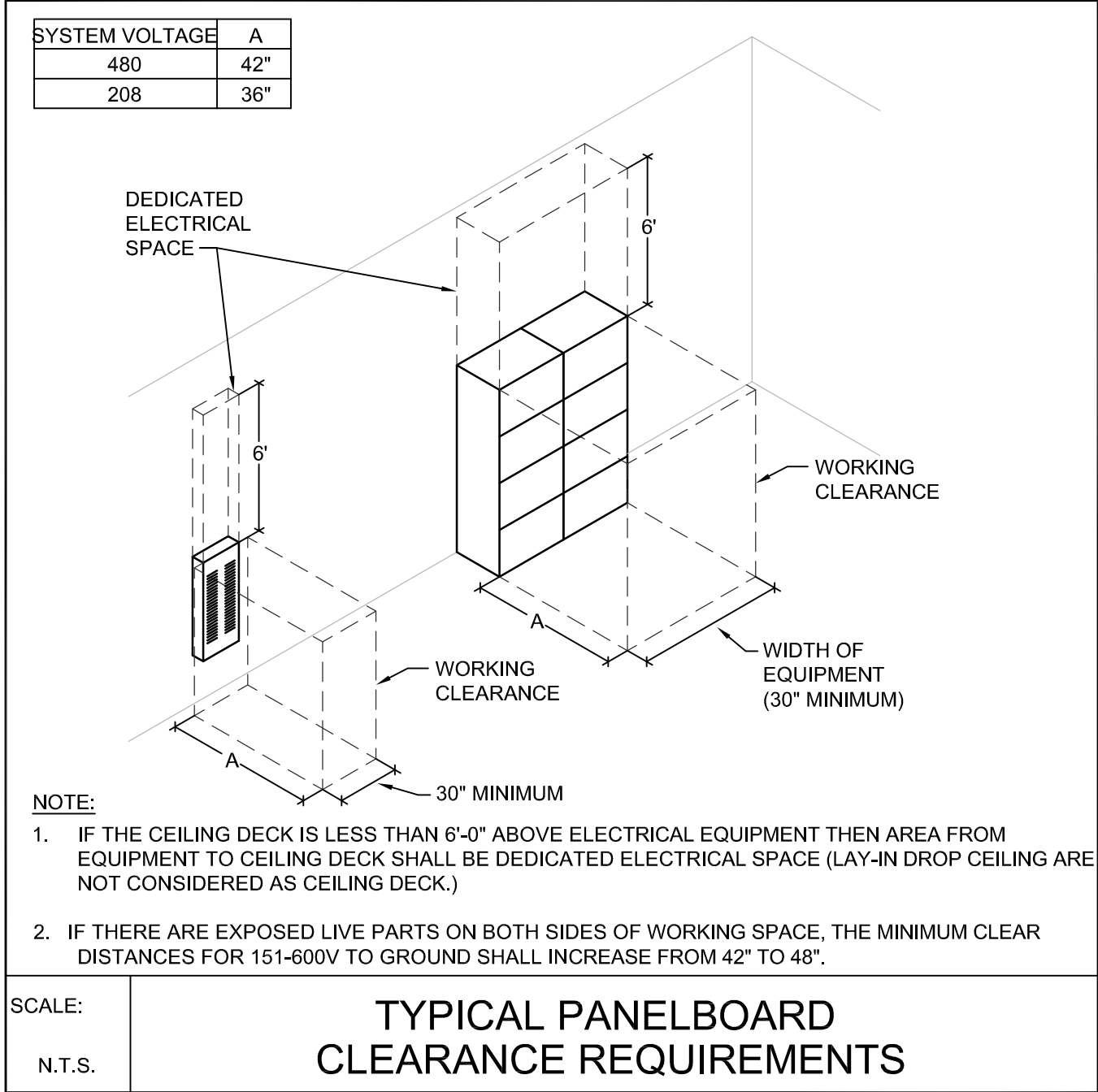
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B

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ECE

E.C.E. LLC  
Electrical Consulting Engineers  
939 So. West Temple  
Salt Lake City, Utah 84101  
Telephone (801) 521-8007  
Email: Admin@eceonline.com  
ECE Project No. 5618



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△	12/07/20	BID SET
REV	DATE	Description



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Submitted By: \_\_\_\_\_

UTA

UTAH TRANSIT AUTHORITY

Approved By: \_\_\_\_\_

Designed By:  
B. LEWIS

Drawn By:  
B. LEWIS

Checked By:  
A. MATINKHAH

Approved By:  
A. MATINKHAH

UTAH TRANSIT AUTHORITY

NEW PAINT BOOTH AT  
WARM SPRINGS SERVICE CENTER

ADDRESS SALT LAKE CITY UT 84116

ELECTRICAL DETAILS

Scale:

CADD Filename:  
EE002.DWG

Submittal Date  
OCTOBER 5, 2020

UTA Contract No.:  
SGR-358

Drawing No.:  
Sheet No.:  
EE002



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Telephone (801) 521-8007  
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ECE Project No. 5618

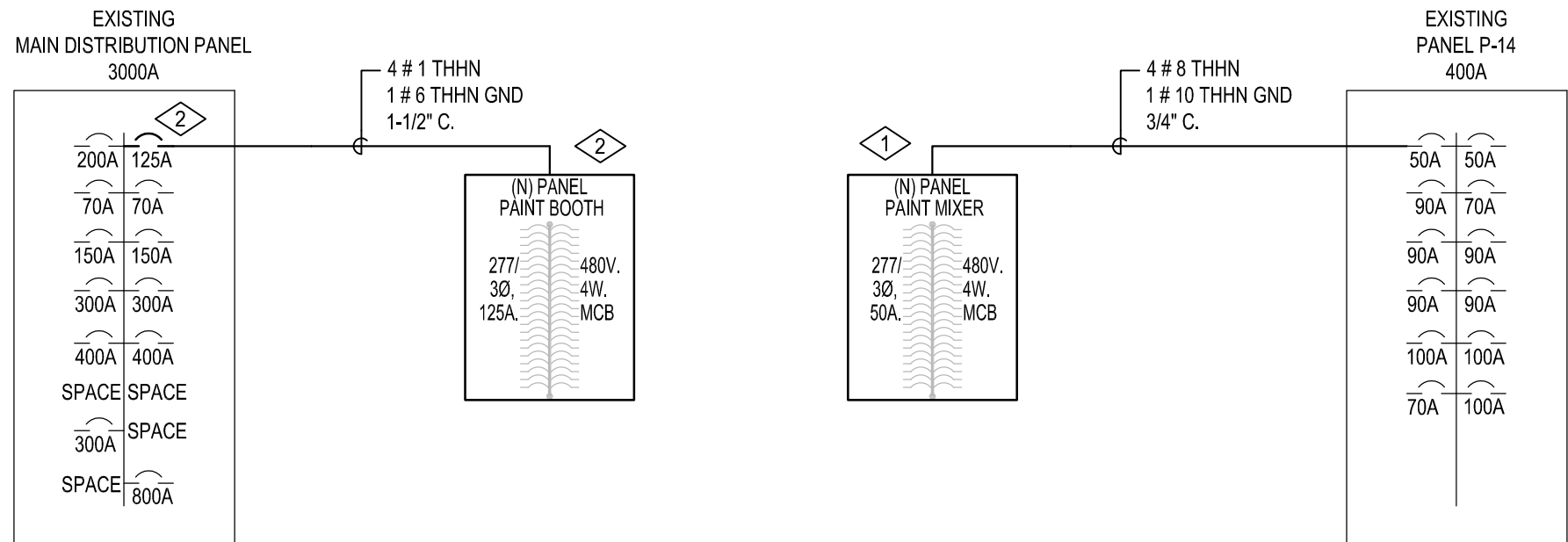
Existing Panel: P-14											
Location:Electrical Mounting:Surface Enclosure:Type1				Volts: 277/480 Phase: 3 Wires: 4				A.I.C. 22K Mains Type:MLO Mains Rating:400A			
CKT	Circuit Description	Load	Class.	Poles	Trip	Phase	Trip	Poles	Class.	Load	Circuit Description
1	(N) PAINT MIXING PANEL	3500	SF	3	50	A	50	3	AC	3500	(E) HVAC RTU-1
3	--	3500	SF	--	--	B	--	--	AC	3500	--
5	--	3500	SF	--	--	C	--	--	AC	3500	--
7	(E) PANEL A	5500	SM	3	90	A	70	3	O	5000	(E) PANEL A
9	--	5500	SM	--	--	B	--	--	O	5000	--
11	--	5500	SM	--	--	C	--	--	O	5000	--
13	(E) PANEL A TRACK 6W	5500	SM	3	90	A	90	3	O	5500	(E) WELDING RECEP
15	--	5500	SM	--	--	B	--	--	O	5500	--
17	--	5500	SM	--	--	C	--	--	O	5500	--
19	(E) PANEL G	5500	SM	3	90	A	90	3	O	5500	(E) WELDING RECEP
21	--	5500	SM	--	--	B	--	--	O	5500	--
23	--	5500	SM	--	--	C	--	--	O	5500	--
25	(E) CRANE TRACK 7,8	6000	SM	3	100	A	100	3	SM	5500	(E) CRANE TRACK 5,6
27	--	6000	SM	--	--	B	--	--	SM	5500	--
29	--	6000	SM	--	--	C	--	--	SM	5500	--
31	(E) WELDING RECEP	5000	SF	3	70	A	100	3	SM	5500	(E) MACTON JACK
33	--	5000	SF	--	--	B	--	--	SM	5500	--
35	--	5000	SF	--	--	C	--	--	SM	5500	--
				AMPS		KVA		VA			
				A		A		A			
				B		B		B			
				C		C		C			
				222.0		222.0		222.0			
				221.9		TOTAL		184.5			
		Concld.	Dmnd.			Concld.	Dmnd.			Dmnd.	
		Load Factor	Load			Load Factor	Load			Load	
		10500	100%			10500	0			0	
		0	100%			0	0%			0	
		0	100%			0	0%			0	
		0	100%			0	0%			0	
		0	100%			0	0%			0	
		0	100%			0	0%			0	
		0	125%			0	100%			0	
		0	0%			0	100%			0	
		0	0%			0	100%			0	
		0	0%			0	100%			0	
		0	0%			0	100%			0	
		0	0%			0	100%			0	
		0	0%			0	100%			0	
		48000	80%			28800	100%			25500	
Notes:											

Existing Panel:MDP											
Location:Electrical Mounting:Wall Enclosure:Type 1				Volts: 277/480		A.I.C.		45K FULLY RATED			
				Phase:3		Main Lug:--					
				Wires:4		Main Breaker:3000A					
CKT	Circuit Description	Load	Class.	Poles	Trip	Phase	Trip	Poles	Class.	Load	Circuit Description
1	(E) COMPRESSOR	160.0		3	200						
2	*(N) PAINT BOOTH PANEL	100.0		3	125						
3	(E) PANEL LD	56.0		3	70						
4	(E) CONTROL ROOM POWER	56.0		3	70						
5	(E) TURN TABLE	120.0		3	150						
6	(E) SHORE POWER	120.0		3	150						
7	(E) WAYSIDE POWER TRACK #16	240.0		3	300						
8	(E) WAYSIDE POWER TRACK #17	240.0		3	300						
9	(E) BASEMENT TRANSFORMER	240.0		3	400						
10	(E) PANEL WB1	240.0		3	400						
11	SPACE	0.0									
12	SPACE	0.0									
13	(E) WAYSIDE POWER TRACK #15	240.0		3	300						
14	SPACE	0.0									
15	SPACE	0.0									
16	(E) BASEMENT DISCONNECTS	400.0		3	800						
Notes: *PROVIDE NEW CIRCUIT BREAKER SIZED AS INDICATED. SAME BRAND & A.I.C. RATING AS EXISTING CIRCUIT BREAKERS											
										Total Conn. Load VA:	1838018
										Total Est. Demand VA:	1838018
										Total Conn. AMPS:	2212
										Total Est. Demand AMPS:	2212

Existing Panel: P-15										A.I.C.10K				
Location:Electrical Mounting:Surface Enclosure:Type1					Volts: 120/208 Phase: 3 Wires: 4					Main Lug:-- Main Breaker:90A				
CKT	Circuit Description	Load	Class.	Poles	Trip	Phase	Trip	Poles	Class.	Load	Circuit Description	CKT		
1	(E) EXHAUST FAN CTRL	200	EF	1	20	A	20	1	O	720	(E) EAST WALL OUTLET	2		
3	(E) EAST WALL OUTLET	720	O	1	20	B	20	1	O	200	(E) CRANE CONTROL	4		
5	(E) EAST WALL OUTLET	720	O	1	20	C	20	1	O	540	(E) EAST WALL OUTLET	6		
7	(E) UNIT HEATER NORTH	1000	EH	1	20	A	45	2	O	2200	(E) DRILL PRESS	8		
9	(E) UNIT HEATER CENTER	1000	EH	1	20	B	--	--	O	2200	--	10		
11	(E) UNIT HEATER SOUTH	1000	EH	1	20	C	20	1	O	1200	(N) NORTH WALL/ROOF	12		
VA					KVA					VA				
1200					A 34.3					A 2920				
1720					B 34.3					B 2400				
1720					C 28.8					C 1740				
4640					TOTAL 32.5					TOTAL 11.7				
Load Class.		Conc'd.	Dmnd. Factor	Dmnd. Load	Load Class.		Conc'd.	Dmnd. Factor	Dmnd. Load					
HVAC Cool (AC)		0	100%	0	Kitchen (K)		0	0%	0					
HVAC Heat (HH)		0	100%	0	E. Range<3.5 kW (R1)		0	0%	0					
Electric Heat (EH)		3000	100%	3000	E. Range<3.5kW (R2)		0	0%	0	Panel Totals				
Exhaust Fans (EF)		200	100%	200	Electric Dryer (ED)		0	0%	0	Total Conn. Demand KVA				
Water Heating (WH)		0	100%	0	Appliance (A)		0	0%	0	11.7				
Lighting (L)		0	125%	0	Elevator (E)		0	100%	0	Total Conn. AMPS				
Lighting Dwelling Unit (LD)		0	0%	0	Boiler (B)		0	100%	0	32				
Lighting Hospital (LH)		0	0%	0	Small Motor (SM)		0	100%	0	Total Est. Demand KVA				
Lighting Motel (LM)		0	0%	0	Snow Melt (SN)		0	100%	0	11.7				
Lighting Warehouse (LW)		0	0%	0	Miscellaneous (M)		0	100%	0					
Outlets (O)		8500	100%	8500	Sub Panel Loads (SP)		0	100%	0					
Notes:														

#### REFERENCE NOTES:

- NEW PAINT MIXING ROOM PANEL PROVIDED BY PAINT MIXING MANUFACTURER. TIE NEW PANEL TO EXISTING 50A CIRCUIT BREAKER IN EXISTING PANEL P-14. SEE PANEL SCHEDULE FOR MORE INFORMATION.
- NEW PAINT BOOTH PANEL PROVIDED BY PAINT BOOTH MANUFACTURER. TIE NEW PANEL TO NEW 125A, 3-POLE CIRCUIT BREAKER IN EXISTING MDP. SEE PANEL SCHEDULE FOR MORE INFORMATION.



POWER SINGLE LINE DIAGRAM  
SCALE: NTS

#### MECHANICAL EQUIPMENT SCHEDULE

NAME OF MECHANICAL EQUIPMENT	MAKE-UP AIR UNIT	EXHAUST FAN	EXHAUST FAN
EQUIPMENT NO.	MAU-1	EF-1,2	EF-3
RATING/WATTS	40 HP	10 HP	2 HP
VOLTAGE	480	480	480
PHASE	3	3	3
AMPS	49.8	13.4	3.3
WIRE SIZE	3 #6	3 #12	3 #12
GROUND WIRE	1 #10	1 #12	1 #12
CONDUIT SIZE	3/4"	3/4"	3/4"
FUSE DISC. SW.	100	30	30
TYPE RKI FUSES	80	25	8
NON-FUSE SW.	--	--	--
NOTES	2,3	1,3	1,3

#### NOTES:

- STARTER IS FURNISHED WITH THE UNIT. PROVIDE SITE DISCONNECT.
- UNIT FURNISHED WITH VFD. PROVIDE SITE FUSED DISCONNECT WITH ELECTRICAL INTERLOCK (FACTORY INSTALLED).
- TIE-INTO PAINT BOOTH CONTROL PANEL.



12/07/20 BID SET

REV DATE Description

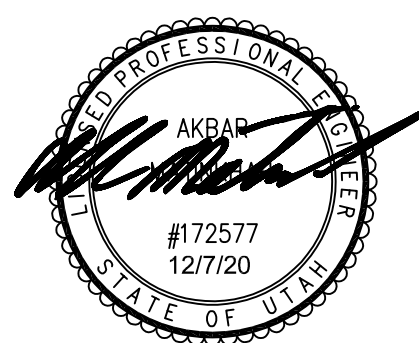
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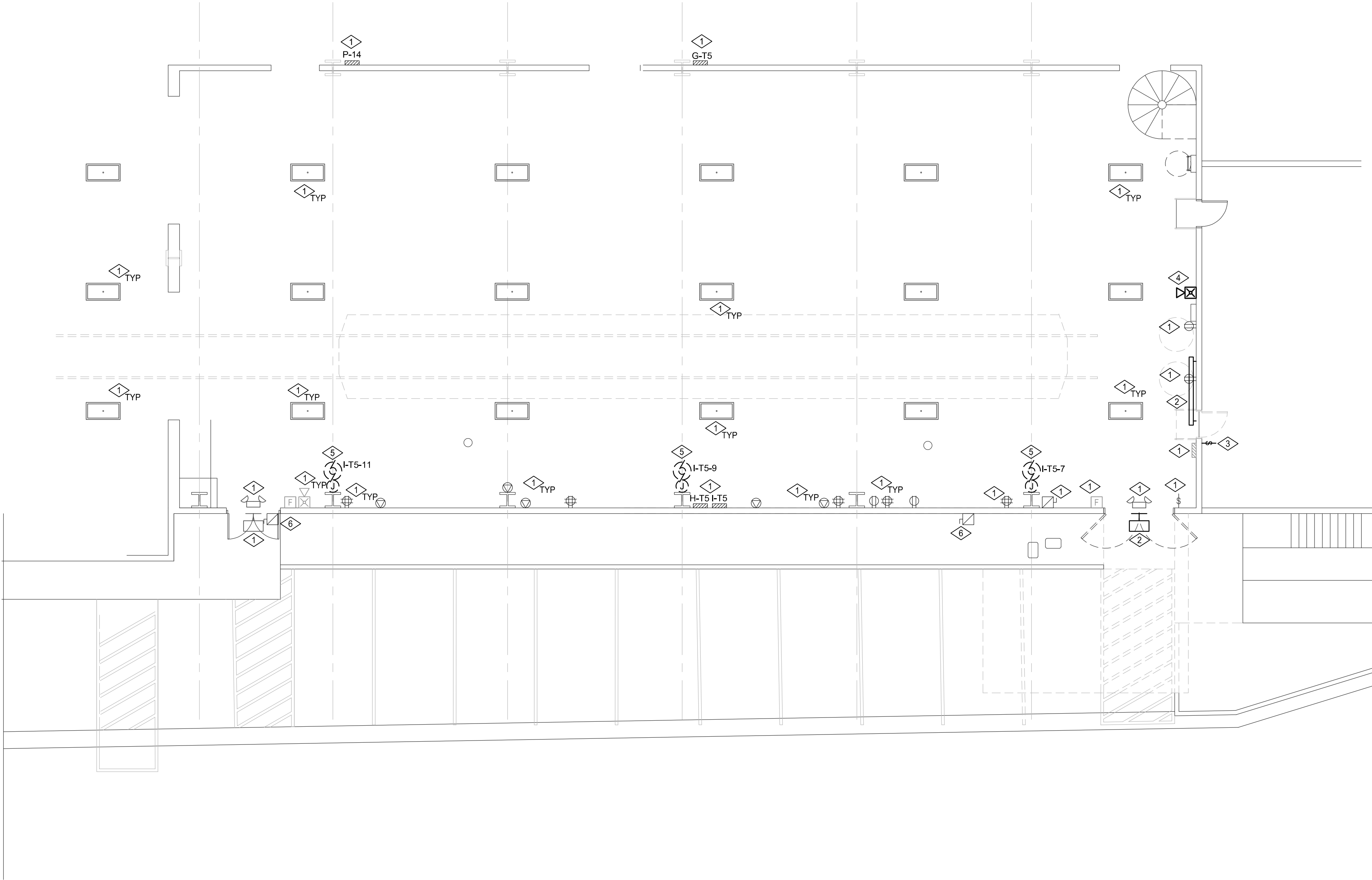


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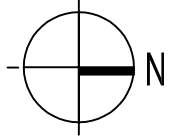
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- REFERENCE NOTES:**
1. EXISTING PANEL, OUTLET, FIRE ALARM DEVICE, ETC. SHALL REMAIN. MAINTAIN CIRCUIT INTEGRITY.
  2. EXISTING WALL MOUNTED LIGHT FIXTURE TO BE RELOCATED TO THE APPROXIMATE NEW LOCATION SHOWN ON EE102. EXTEND ASSOCIATED CONDUIT AND CONDUCTORS TO THE NEW LOCATION.
  3. REMOVE EXISTING LIGHT SWITCH. MAINTAIN CIRCUIT INTEGRITY TO EXISTING LIGHTS TO REMAIN IN ROOM.
  4. EXISTING FIRE ALARM DEVICE TO BE RELOCATED TO THE APPROXIMATE NEW LOCATION SHOWN ON EE101. EXTEND EXISTING CONDUIT AND CONDUCTORS IN CLASS "A" LOOP TO NEW LOCATION.
  5. EXISTING GAS FIRED UNIT HEATER TO BE RELOCATED. EXTEND EXISTING CONDUIT, CONDUCTORS, ETC. TO NEW LOCATION SHOWN ON SHEET EE101.
  6. REFER TO SHEET EE101 FOR MORE INFORMATION FOR THE POWER TO RELOCATED SWAMP COOLERS.

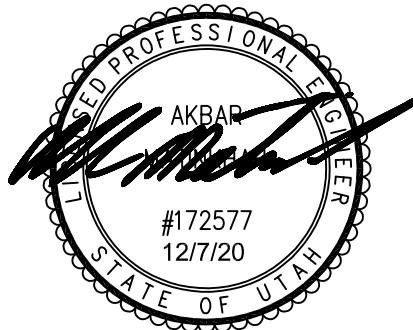


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DEMOLITION FLOOR PLAN - ELECTRICAL  
SCALE: 1/8" = 1'-0"

1		
2		
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5		
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UTAH TRANSIT AUTHORITY  
NEW PAINT BOOTH AT  
WARM SPRINGS SERVICE CENTER  
ADDRESS SALT LAKE CITY UT 84116  
DEMOLITION FLOOR PLAN – ELECTRICAL

Scale:	1/8" = 1'-0"
CADD Filename:	ED101.DWG
Submittal Date	OCTOBER 5, 2020
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Drawing No.:	Sheet No.:
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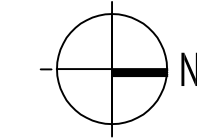
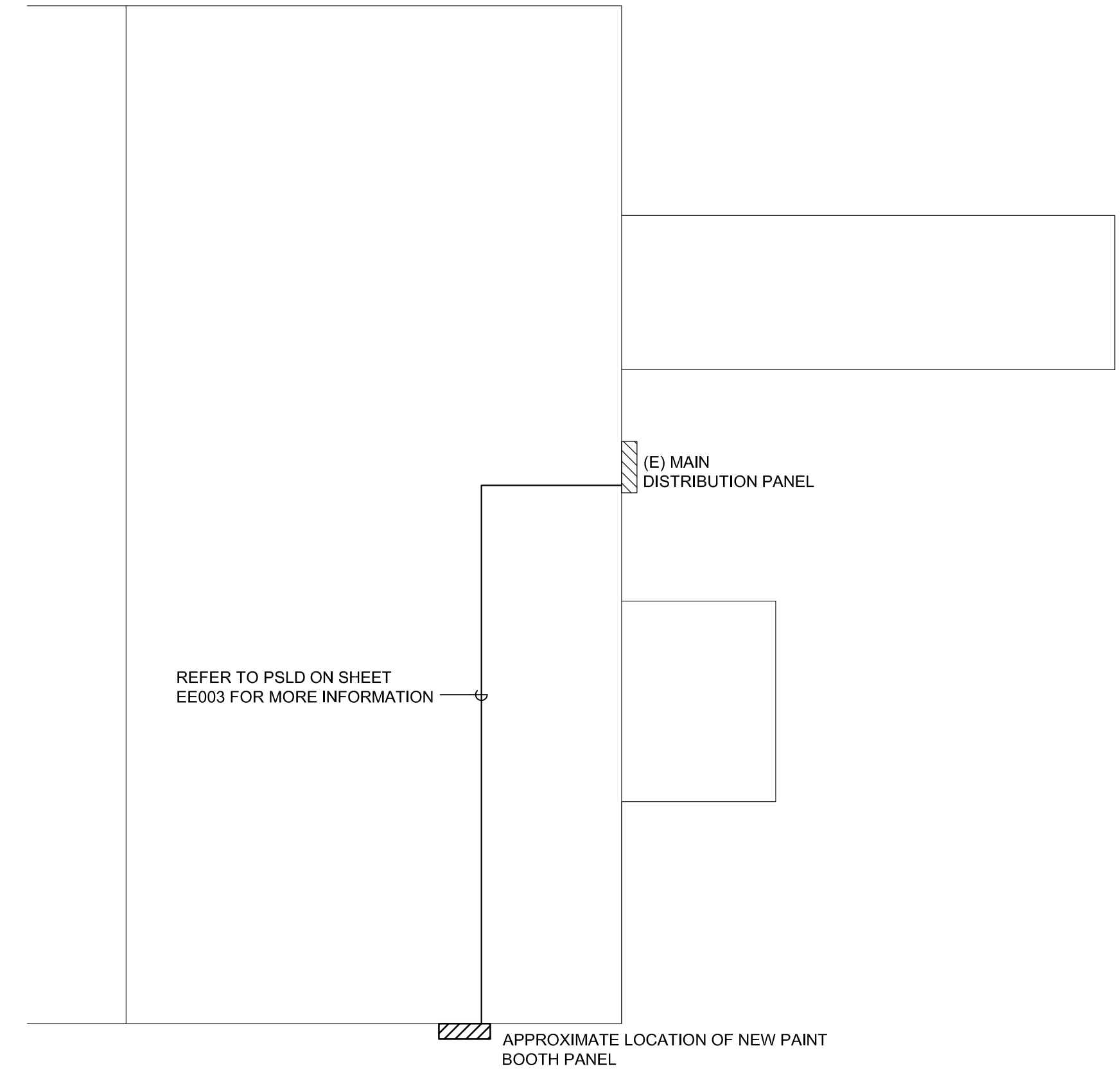
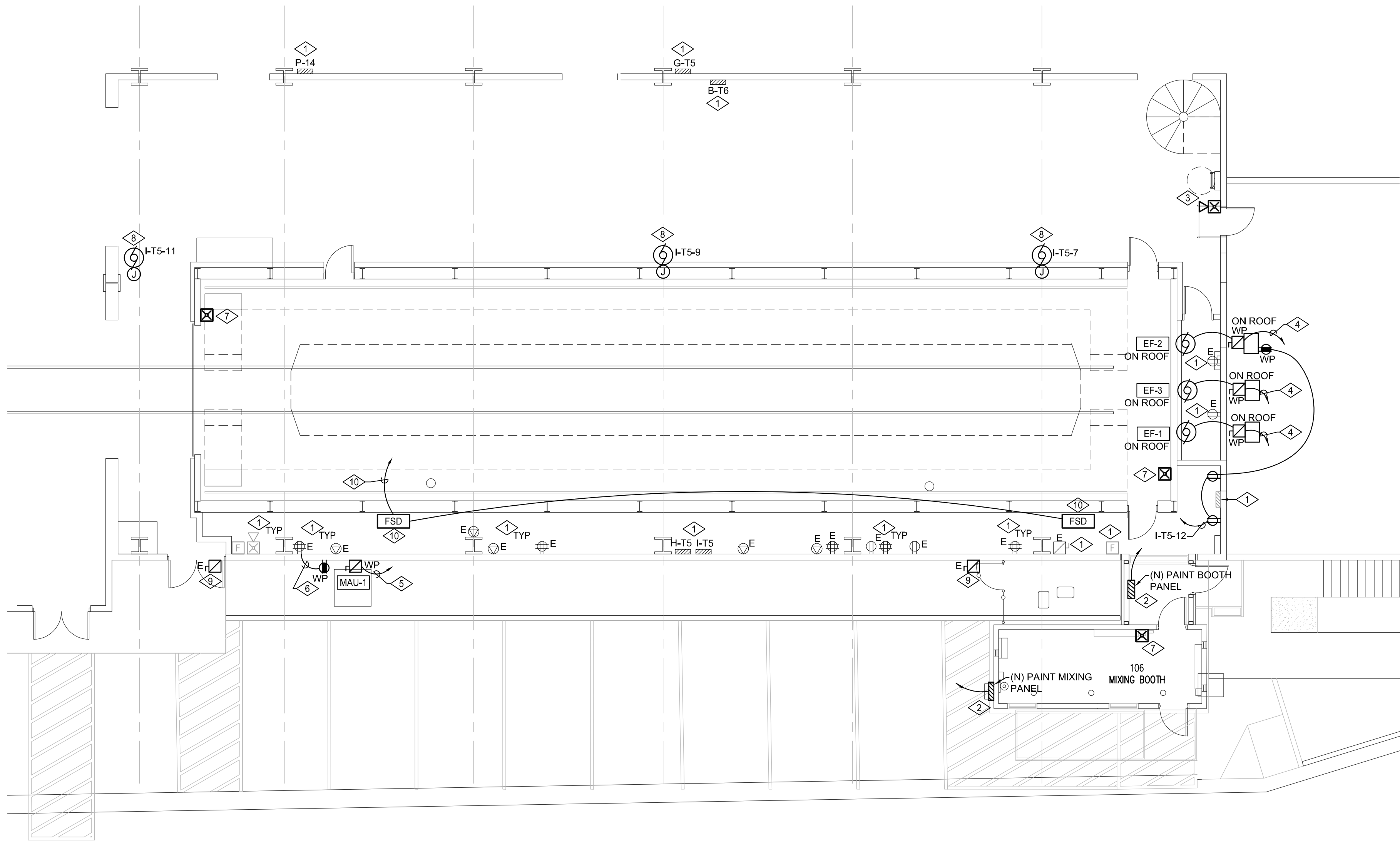
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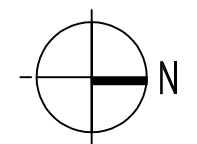
1. EXISTING PANEL, OUTLET, FIRE ALARM DEVICE, ETC. SHALL REMAIN. MAINTAIN CIRCUIT INTEGRITY.
2. NEW PAINT BOOTH PANEL BY OTHERS. REFER TO PSLD ON SHEET EE003 FOR MORE INFORMATION.
3. RELOCATE EXISTING FIRE ALARM DEVICE TO APPROXIMATE NEW LOCATION SHOWN. EXTEND CONDUIT AND CONDUCTORS TO NEW LOCATION.
4. TIE DISCONNECT TO STARTER IN PAINT BOOTH CONTROL PANEL. COORDINATE WORK WITH MECHANICAL.
5. ELECTRIC INTERLOCK CABLE TO RUN IN A SEPARATE CONDUIT TO VFD.
6. TIE NEW WEATHER PROOF OUTLET TO EXISTING OUTLET.
7. FURNISH AND INSTALL A NEW EXPLOSION PROOF FIRE ALARM DEVICE. TIE THE NEW DEVICES TO EXISTING CLASS "A" LOOP. IN AND OUT CABLES TO DEVICES MUST RUN IN SEPARATE CONDUIT. NEW DEVICES MUST BE COMPATIBLE WITH EXISTING SILENT KNIGHT FIRE ALARM CONTROL PANEL.
8. APPROXIMATE NEW LOCATION FOR GAS FIRED UNIT HEATERS. EXTEND EXISTING ASSOCIATED CONDUIT, CONDUCTORS, J-BOXES, ETC., TO NEW LOCATION. COORDINATE WITH MECHANICAL CONTRACTOR FOR EXACT HEIGHT AND LOCATION.
9. EXISTING SWAMP COOLERS TO BE RELOCATED APPROXIMATELY 10 FEET ABOVE CURRENT LOCATION. DISCONNECT CAN REMAIN AT CURRENT LOCATION, EXTEND CONDUIT, CONDUCTORS, J-BOXES, ETC. TO NEW LOCATION. COORDINATE WITH MECHANICAL CONTRACTOR FOR EXACT HEIGHT AND LOCATION.
10. TIE THE FIRE SMOKE DAMPERS TO THE NEAREST 120V EMERGENCY POWER CIRCUIT WITH AVAILABLE CAPACITY. TIE CIRCUIT THROUGH FIRE ALARM CONTROL PANEL. DAMPERS ARE TO CLOSE WHEN FIRE ALARM SYSTEM IN ON ALARM. COORDINATE WITH MECHANICAL CONTRACTOR FOR EXACT LOCATION.

SPECIAL NOTES:

1. ELECTRICAL EQUIPMENT SIZES AND LOCATIONS BASED ON MANUFACTURER RECOMMENDATIONS. VERIFY WORK WITH MANUFACTURER FOR EXACT REQUIREMENTS.
2. ALL NEW CONDUIT TO BE INSTALLED AGAINST THE DECK. DO NOT PENDANT HANG CONDUIT FROM DECK.



SITE PLAN - POWER  
SCALE: 1" = 40'



FLOOR PLAN - POWER  
SCALE: 1/8" = 1'-0"



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ADDRESS SALT LAKE CITY UT 84116  
NEW FLOOR PLAN - POWER

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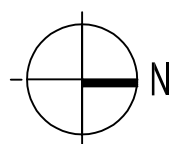
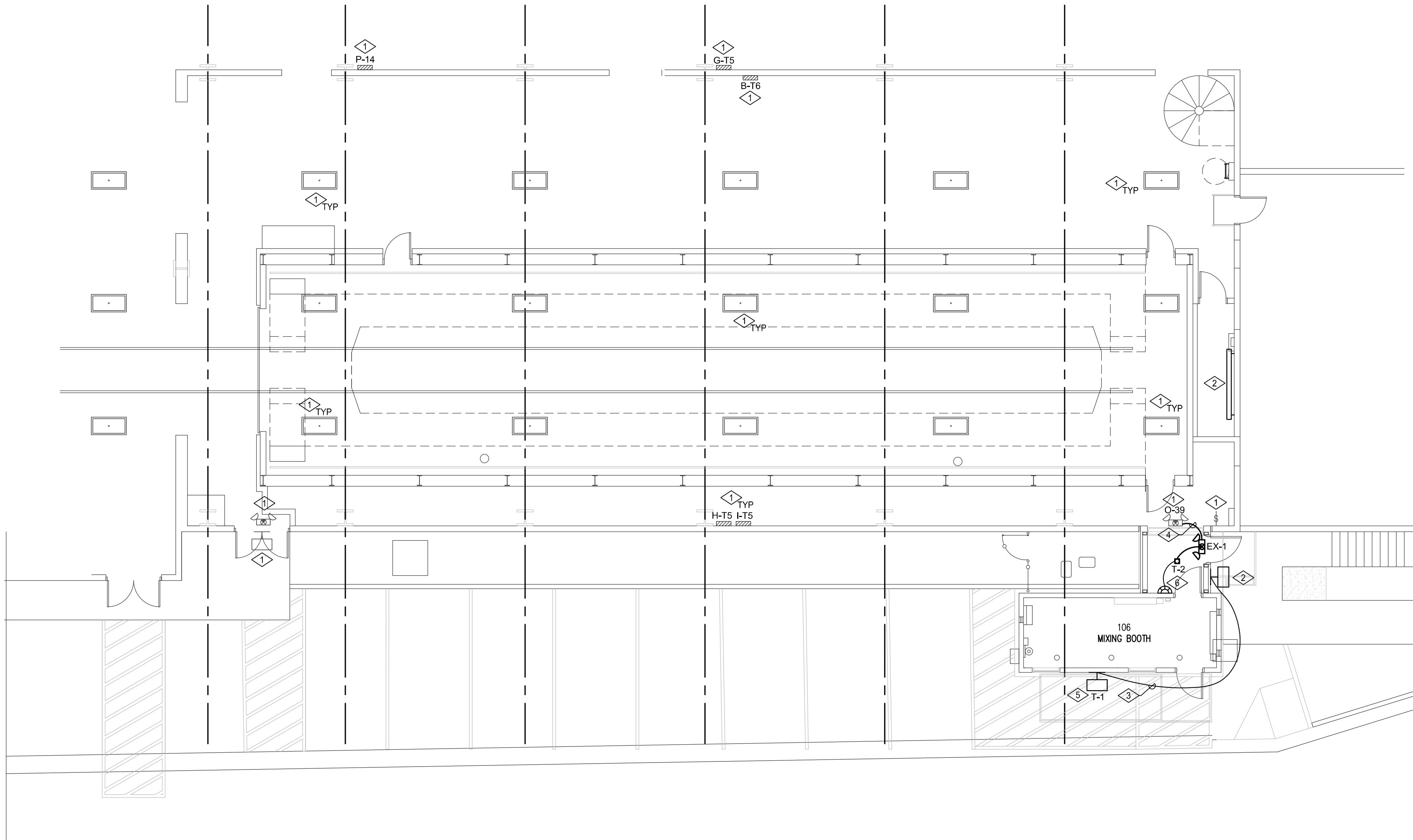
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REFERENCE NOTES:

1. EXISTING PANELS, LIGHT FIXTURES, ETC, ARE TO REMAIN. MAINTAIN CIRCUIT INTEGRITY.
2. RELOCATE EXISTING WALL MOUNTED FIXTURE TO APPROXIMATE NEW LOCATION SHOWN. EXTEND ASSOCIATED CONDUIT AND CONDUCTORS TO NEW LOCATION. COORDINATE WITH ARCHITECT FOR EXACT LOCATION AND HEIGHT.
3. TIE NEW T-1 LIGHT FIXTURE TO EXISTING LIGHTING CIRCUIT ASSOCIATED WITH THE RELOCATED LIGHT.
4. TIE NEW EXIT SIGN / EMERGENCY LIGHT TO EXISTING EMERGENCY LIGHTING CIRCUIT AS SHOWN.
5. COORDINATE EXACT HEIGHT AND LOCATION OF EXTERIOR LIGHT FIXTURE WITH ARCHITECT.
6. FURNISH AND INSTALL A WALL MOUNTED DUAL TECHNOLOGY MOTION SENSOR WITH AUTO-OFF SWITCH IN APPROXIMATE LOCATION SHOWN, TO CONTROL CAN LIGHT FIXTURE. WATTSTOPPER TYPE DW-100 OR APPROVED EQUAL.



FLOOR PLAN - LIGHTING

SCALE: 1/8" = 1'-0"

**ECE**

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